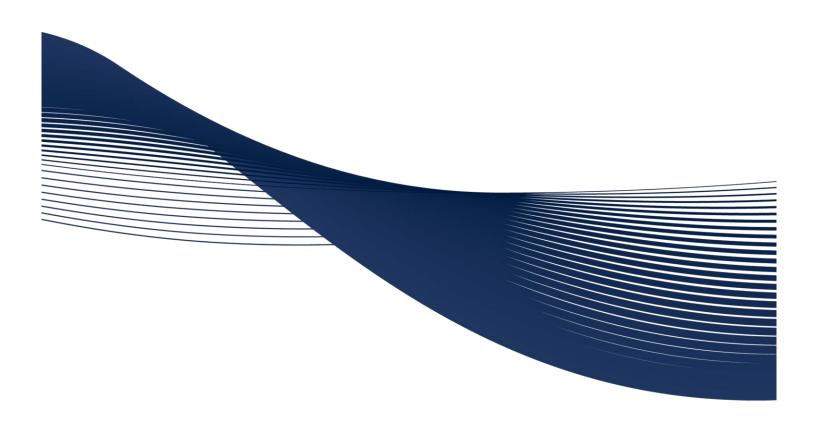
CITY PARK (OLD BARBER) HOMES INC.

REVISED TRAFFIC IMPACT AND PARKING STUDY

5155 Mississauga Road, City of Mississauga

Project No.: UD16-0022





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October 23, 2017 Project No. UD16-0022

Giancarlo Tedesco, E.I.T., C.E.T. Traffic Planning Technologist City of Mississauga 300 City Centre Drive Mississauga, ON L5B 3C1

Dear Mr. Tedesco:

Re: Response Letter to City of Mississauga Comments dated June 8, 2017

5155 Mississauga Road Proposed Residential Development

City of Mississauga

Cole Engineering Group Ltd. (COLE) is pleased to provide this letter to the City of Mississauga (the "City"), Planning and Building Department, Development and Design Division, in response to City Staff Comments, issued via the Application Status Report dated June 8, 2017. The transportation comments were drafted by Giancarlo Tedesco in May 11, 2017.

The subject property is located on the northeast side of the intersection of Barbertown Road and Mississauga Road, municipally known as 5155 Mississauga Road.

We have reviewed the June 8, 2017 the Application Status Report. This letter provides City comments reiterated below in **bold**, and our response to said comments in *italics*. COLE has also attached a Revised Traffic Impact Study (TIS), which includes an updated Site Plan dated October 19, 2017.

1. March 28, 2017 Recirculation: The plans have revised visitor parking, shifting the 4th space required to just within the site entrance. A minimum access clear throat length of 8.0m is required. Notwithstanding other comments related to access, the visitor parking space located near the site entrance will not be permitted. The required 4 visitor spaces should be identified in suitable location(s). A detailed turning movement diagram will be required to demonstrate how the most easterly visitor parking space will function to the satisfaction of this department. Additional provisions to aid in the safety and operation of these features may be required.

As per the comments, the Site Plan has been modified by the Site Plan team. The visitor parking spaces have been relocated to the most easterly side of the roadway. The detailed turning movement diagram for the visitor parking #4 is illustrated in Figure 7-3. As per the AutoTurn Diagram a typical 5m (TAC-1999) vehicle, can enter / exit parking stall with a four (4) point turn. AutoTurn analysis has been completed for compact car as well.

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Compact cars are classified by volume in North America under the United States Environmental Protection Agency (EPA). As such, a sample compact car (Toyota Auris) was utilized for modelling and analysis purposes. The sample car has a length of 4.245m and a total width of 1.76m. This sample car was used to demonstrate parking maneuvers for the visitor parking space #4. Other examples of possible compact cars include:

- Toyota Corolla
- Honda Civic
- Chevrolet Cruze
- Hyundai Elantra
- Ford Focus

Based on the passenger vehicle template it is determined that 4.245m passenger vehicle can enter / exit the parking space with ease.

2. The proposed Mississauga Road access is to be relocated to Barbertown Road. The applicant is advised that the access to Mississauga Road will be restricted in accordance with the Scenic Route Policy and Official Plan. Access shall be designed for the safe and efficient flow of goods and traffic, further minimizing conflicts with active transportation modes. To ensure this operation, the applicant is to revise the site plan drawing to illustrate access from Barbertown Road only.

The site access for the proposed development is proposed to be a full movement access, via Mississauga Road (west side of the development).

The proposed access is expected to have negligible effect on the operations of Mississauga Road due to low site traffic volume. As seen in **Section 5.2** of the revised TIPS, the addition of site traffic onto Mississauga Road will have no effect on the operations of Mississauga Road.

If the site access were to be provided along Barbertown Road (south of the proposed development), the traffic operations at the site access may be poor due to no dedicated turning lanes along Barbertown Road. With the close proximity to the intersection of Barbertown Road and Mississauga Road, the queue length of vehicles waiting to turn left, eastbound, into the proposed development, may impact intersection operations. Mississauga Road currently has a two (2)-way centre left-turn lane, which provides a safe refuge and queuing area for left turns along Mississauga Road, and will maintain an acceptable LOS along the roadway.

The proposed site access is located approximately 60m away from the intersection of Barbertown Road and Mississauga Road, and with an existing left-turn lane, the site traffic will have no impact on the intersection.

Closing the current site access onto Barbertown Road, and not providing access for the future proposed development on Barbertown Road, would increase the safety of the roadway, neighbourhood and the park which is located directly south of the proposed development, and minimize traffic infiltration into the neighbourhood.



In addition, the four (4) detached dwellings along Barbertown Road will be access from Barbertown Road so that does remove some of the traffic generation from the main site access on Mississauga Road.

3. March 28, 2017 Recirculation: The plans shall be revised to identify an appropriate "clear throat" at the internal intersection, whereas the parking spaces shown will not be permitted to intersect the radii curbing.

As per the comments, the Site Plan has been modified by the Site Plan team. Please see Figure 1-2 in the report for the most updated Site Plan.

4. This department is in receipt of a Traffic Impact Study, prepared by Cole Engineering Group Itd. (dated June 2016 and updated March 23, 2017). Detailed comments and conditions will be provided prior to the Recommendation Meeting. - Additionally, the study shall recognize the following existing uses and provide an opinion on their impacts to the network and study: - The Portico a community church, located at 1814 Barbertown Road. - Sunday Service: (Sundays: 8:40am; 10:10am; and 11:40am); - Students @ Portico: (Tuesdays: 7:00pm - 9:00pm) - Mid Week Ministry Program: (Wednesdays: 7:00pm - 8:30pm) - Facility Rentals: 1,000 seat auditorium; 200 seat chapel; 450 seat gymnasium - ADM Milling, located at 1770 Barbertown Road. - Tankers: 6 in, 6 out (12 two-way) trips *daily* - Wheat delivery: 6 in, 6 out (12 two way trips) *daily* - The consultant is to review and provide an opinion on the queuing potential associated with the at grade rail crossing North of the site.

We have recognized the above existing uses. In our opinion, these developments will not have any effect on the road network and study as they are existing developments. Any traffic that is generated from these developments that coincide with the peak periods of the proposed development was captured in the turning movement count data collection and subsequently included in our existing traffic analysis.

As for the peak periods of these developments, they do not coincide with the proposed developments peak periods. Since these developments do not have corresponding peak periods, the surrounding road network will be able to accommodate the associated traffic. While one (1) development is experiencing high site generated traffic, the other will be experiencing negligible traffic (off-peak).

We have also reviewed the at grade rail crossing, which is located approximately 560m north of the proposed development, and we are of the opinion that the proposed development will have no effect on the crossing.

The opinion was made after reviewing both study area intersections (Mississauga Road and Barbertown Road, and Mississauga Road and Site Access) in the future (2021) total traffic horizon year. All southbound movements at both intersections were reviewed for their 95th percentile queue length in both the weekday AM and PM peak period. It was determined that the queue lengths would not have any effect on the crossing, as the maximum observed 95th percentile queue length for the southbound movements was 1.0m, which was for the southbound left turn at the intersection of Mississauga Road and Barbertown Road.



5. March 28, 2017 Recirculation: The plans identify a structure described as "relocated brick shed (bike storage)". A response should be supplied which provides clarification of how this achieves the requirement for 1 outdoor and 3 indoor spaces. RE: 4 Barber House Units The applicant is required to provide short term (outdoor) and long term (indoor) bicycle storage facilities for the purpose of a minimum 1 outdoor and 3 indoor parking spaces (in a close vicinity to the main access). The Site Plan shall be revised to identify these locations and to specify the facility detail(s), including quantity of spaces proposed for each.

Short-term bicycle parking is typically provided in close proximity to entrances via high tensile strength racks. Long term bicycle parking is intended for use over several hours or overnight and the residents will also likely use their garages for bicycle storage and parking on a long term basis.

The proposed development will provide a brick shed, to be located to the east of the Barber House that will serve as a secure storage facility for both residents and visitors wishing to cycle to and from the site. The brick shed will serve as both short term and long term bicycle parking, with an area of 6.54m², and exclusively will be used by visitors and residents. Based on the City's comments, dated June 8, 2017, **Table 1** below summarizes the required bicycle parking spaces for the proposed development, based on short-term and long-term parking.

Table 1 Required Bicycle Parking Requirements

Table = medamea = reference arming medame emerce					
Type of Bicycle Parking	Rate	# Parking Required			
Short-Term (outdoor) (four (4)units) two (2)- storey back-to-back within the converted Old Barber House	0.08/unit	1 space			
Long-Term (indoor) (four (4) units) two (2)- storey back-to-back within the converted Old Barber House	0.70/unit	3 spaces			

The proposed development will provide a total of one (1) bicycle parking spaces for visitors (short-term) and three (3) bicycle parking spaces for residents (long-term) as summarized in the above table.



6. Waste Collection Comments: For the townhouses: The Region of Peel will provide curbside collection of garbage, recyclable materials, household organics and yard waste subject to the following conditions: The current proposed plan will not be permitted for waste collection, as the back-up distance is too large. Each dwelling unit within a development must have its own identifiable collection point. Waste Collection Vehicle Access Route Comments: The turning radius from the centre line must be a minimum of 13 metres on all turns. In those situations where a waste collection vehicle must reverse, then the maximum straight back-up distance is 15 metres. The internal road layouts should be designed to permit continuous collection without reversing. Where the requirement for continuous collection cannot be met, a cul-desac or a ?T?-turnaround will be permitted in accordance with the specifications shown in Appendix 2 and 3 of the WCDSM (Waste Collection Design Standards Manual), respectively. Curbside Collection Comments: The waste set out location is to be as close as possible to the travelled portion of the roadway, directly adjacent to the private property of the unit occupier/owner, directly accessible to the waste collection vehicle and free of obstructions (i.e. parked cars). For the 4 back-to-back units (Block 4): The use of common collection areas for single-detached dwellings, semi-detached dwellings, townhouses and multi-residential complexes up to six dwelling units is not permitted. The set out area along the curb, adjacent to the driveway must be at least 3 square metres per unit in order to provide sufficient space for the placement of two carts: maximum 1 large garbage or recycling cart (360 litres) and 1 organics cart (100 litres), overflow waste (i.e. additional bags), yard waste receptacles and bulky items. Each unit within a development must have its own identifiable waste collection point (distinct set out area along the curb or the sod that cannot be shared with neighbouring units) as approved by Public Works Commissioner or Delegate. The collection point must be located along the curb, adjacent to the driveway, and must be directly accessible to the waste collection vehicle and free of obstruction such as parked cars.

As per the City comments, the Site Plan has been revised. The minimum turning radius from the centerline 13m is provided.

Based on the discussion with the Regional staff, given the constraints of the site, the Region allowed exception to the backup distance requirement with following conditions:

- Developer will provide space for storage of waste carts for units 17-20, on the day of collection, carts to be set out close to curb for collection;
- Waste collection vehicle driver will backup for approximately 15m for automated collection from some units, and for the remaining units (units 11-16) she / he will manoeuver the carts manually for collection;
- Common collection pad will not be allowed.



Giancarlo Tedesco City of Mississauga Page 6 October 23, 2017

Yours sincerely,

COLE ENGINEERING GROUP LTD.

Kim Nystrom Senior Project Manager

Transportation Planning and Traffic

Transportation

SC:KN/jay

Sevim Coskun, C.E.T. Transportation Analyst Transportation Planning and Traffic Transportation





October 23, 2017 Reference No. UD16-0022

Mr. Evan Perlman Glen Scharr & Associates Inc. c/o City Park (Old Barber) Homes Inc. 700 – 10 Kingsbridge Garden Circle Mississauga, ON L5R 3K6

Dear Mr. Perlman:

Re: Revised Traffic Impact and Parking Study

5155 Mississauga Road City of Mississauga

Cole Engineering Group Ltd. (COLE) is pleased to submit this Revised Traffic Impact and Parking Study for the above noted development. This report documents our findings and conclusions regarding the traffic impact and parking requirements in support of an application for Official Plan and Zoning By-law Amendment for the proposed residential development located at 5155 Mississauga Road, in the City of Mississauga (the "City").

The conceptual development plan for the site will consist of 25 residences including: five (5) freehold detached single family units, 16 townhouse dwellings, and four (4) two (2)-storey back-to-back units within the converted Old Barber House. Access to the proposed development will be provided via a proposed full movement driveway to Mississauga Road. A total 44 parking spaces including 32 parking spaces for common element detached / townhouse dwelling units, seven (7) spaces for common element condo / two (2)-storey townhouse visitors, and five (5) parking spaces for visitors are provided.

This Revised Traffic Impact and Parking Study concludes that the development will have minimal impact to the study intersections. Further, the proposed parking supply was found to be adequate to accommodate the parking demands of the proposed development.

Should you have any questions, please do not hesitate to contact the undersigned.

Yours truly,

COLE ENGINEERING GROUP LTD.

Kim Nystrom Principal Traffic and Planning

Transportation

SC:KN/jay

Sevim Coskun, C.E.T. Transportation Analyst Traffic and Planning Transportation



PREPARED BY:

COLE ENGINEERING GROUP LTD.

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CHECKED BY:

COLE ENGINEERING GROUP LTD.

Kim Nystrom Principal

Traffic and Planning

Transportation

Issues and Revisions Registry

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Identification	Date	Description of issued and/or revision			
Draft Report	March 2016	For internal review			
Draft Report	March 2016	For Client review			
Final Report	April 2016	For Client submission			
Final Report	May 2016	For Client review			
Final Report	June 2016	For Client review			
Draft Report	March 2017	For Client review			
Final Report	October 2017	For submission			



Statement of Conditions

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1 Introduction

Cole Engineering Group Ltd. (COLE) was retained by City Park (Old Barber) Homes Inc. (the "Owner") to undertake a Traffic Impact and Parking Study in support of an application for Official Plan and Zoning Bylaw Amendment for the proposed residential development located at 5155 Mississauga Road, in the City of Mississauga (the "City"), in the Region of Peel (the "Region").

The subject property is located in the northeast quadrant of the Mississauga Road and Barbertown Road intersection, with single family houses to the east, Mississauga Road to the west, vacant land to the north and Barbertown Road to the south. **Figure 1-1** illustrates the site location.

The proposed use will consist of 24 residences including four (4) freehold detached single family units, 16 townhouse dwellings, and four (4) two (2)-storey back-to-back units within the converted Old Barber House. Access to the development will be provided via a proposed full moves driveway to Mississauga Road. The proposed development will provide a total 44 parking spaces including 32 parking spaces for common element detached / townhouse dwelling units, seven (7) spaces for common element condo / two (2)-storey townhouse visitors, and five (5) parking spaces for visitors are provided. **Figure 1-2** illustrates the proposed Site Plan.

The purpose of this study is to:

- Establish existing traffic conditions for the study area through the operational assessment of the study area intersections;
- Determine future traffic conditions for a post build out, five (5)-year traffic horizon;
- Identify study area capacity constraints (if any) in the future background conditions and recommend roadway improvements, if required, to provide better roadway operations;
- Forecast and estimate the traffic associated with the proposed development using the information published in the *Trip Generation Manual*, 9th Edition, by the Institute of Transportation Engineers (ITE);
- Undertake future total traffic analysis to determine the impact of the proposed development on the surrounding road network and recommend any improvements which may be required to accommodate the proposed development;
- Identify mitigation measures to address any deficiencies at each horizon year (existing, future background and future total traffic) for key study area intersections; and,
- Review the adequacy of the proposed parking supply with regards to the future demands of the site.

1.1 Study Approach

The study's methodology and analysis reflects the City of Mississauga's Traffic Impact Study Guidelines.

Weekday morning and afternoon peak period traffic volume counts were undertaken by Accu-Traffic Inc. on February 10, 2016, on behalf of COLE for following intersection:

• Mississauga Road and Barbertown Road (Unsignalized).

Future background traffic volumes for the 2021 horizon year, excluding the additional traffic volumes generated by the proposed development, consist of the following components: traffic growth from outside the study area, and traffic from active developments in the vicinity of the subject development.

Site traffic was derived using information contained in the *Trip Generation*, 9th Edition, published by the Institute of Transportation Engineers (ITE), and distributed based on the existing traffic pattern.

Future total traffic conditions were determined by the summation of the estimated traffic volumes generated by the site and the background traffic volumes for the 2021 full build-out horizon.

1.1.1 Assessment of Unsignalized Intersection Operations

The unsignalized intersection operational analysis in this report was also conducted using *Synchro 9.0* software, which employs the Highway Capacity Manual (HCM 2000) methodology. All parameters for the unsignalized intersection analysis were based on the Synchro default values. Synchro results for the unsignalized intersections are provided in HCM format.

2 Existing Traffic Conditions

2.1 Existing Road Networks

The existing road network, lane configurations and existing traffic control for the study area is shown in **Figure 2-1**. The details are described as follows:

- **Mississauga Road:** is a north-south major collector (scenic) road consisting of two (2) through travel lanes with a continuous left turn lane in the center with dedicated a bike lane on both sides of the road. The road is under the jurisdiction of the City. The roadway maintains a posted speed limit of 50km/h in the vicinity of the subject site.
- Barbertown Road: is an east-west local road consisting of two (2) through travel lanes, and is under jurisdiction of the City. The roadway maintains an unposted speed limit of 50km/h in the vicinity of the subject site. It is a dead end road with a sidewalk on the south side.
- Rothesay Court: is an east-west and south-north local road consisting of two (2) through travel lanes, and is under jurisdiction of the City. The roadway maintains an unposted speed limit of 50km/h in the vicinity of the subject site. It is a dead end road with a cul-de-sac. A sidewalk is located on the south side of the road.

2.2 Existing Traffic Volumes

Existing traffic volumes were collected by Accu-Traffic Inc. on February 10, 2016 during the morning (7:00 a.m. to 9:00 a.m.) and afternoon (4:00 p.m. to 6:00 p.m.) peak periods. Detailed existing traffic data is provided in **Appendix A**.

2.3 Existing Traffic Assessment

The study area intersections were analyzed under existing traffic conditions using *Synchro 9.0* software. Volume / capacity (v/c) ratios for overall intersection operations, through movements, or shared through / turning movements greater than 0.85, and v/c ratios for exclusive movements greater than 0.95 have been shown in bold. The existing traffic volumes are illustrated in **Figure 2-2**. The results are summarized in **Table 2.1** below, and the detailed calculations are shown in **Appendix B**.

Table 2.1 Existing Traffic Analysis

Intersection	Key Movement	AM Peak Hour	PM Peak Hour
		LOS (v/c)	LOS (v/c)
	EB left-through-right	B (0.05)	B (0.02)
Mississauga Road / Barbertown Road (Unsignalized)	WB left-through-right	B (0.07)	C (0.08)
	NB Left	A (<0.01)	A (0.01)
	NB through-right	A (0.29)	A (0.41)
	SB left	A (0.01)	A (0.03)
	SB through-right	A (0.40)	A (0.33)

The existing analysis reveals that the unsignalized intersection of Mississauga Road / Barbertown Road is operating with good levels of service (LOS) and v/c ratios during the a.m. and p.m. peak hours.

3 Future Background Traffic Conditions

3.1 Background Traffic Growth

Based on the discussion with the City staff, a 1.0% growth rate per annum was applied to all traffic directions during the a.m. and p.m. peak hours, along Mississauga Road.

3.2 Background Developments

As discussed with the City staff, COLE was advised that there are three (3) active background development applications in the vicinity of the study area. The background developments included in the assessment are summarized in **Table 3.1** on the following page. The detailed calculations are shown in **Appendix C.**

Table 3.1 Background Developments Trip Generation

Lond Hee	11::+ / CEA	Downwaters	AM	peak I	Hour	PM	Peak H	lour
Land Use	Unit / GFA	Parameters	In	Out	Total	In	Out	Total
		Gross Trips	104	63	167	199	216	415
5276 Mississauga Road	444 0005;3	Gross Trip Rate (trips / unit)	0.92	0.57	1.49	1.78	1.93	3.71
Commercial/Retail Plaza	111,900ft ²	Bypass Trip (33%)			1	69	69	138
(LUC 820)		New Trips				130	147	277
		New Trip Rate				1.16	1.32	2.48
5276 Mississauga Road		Gross Trips	4	13	17	8	5	13
Residential Development (LUC 210)	10 Units	Gross Trip Rate (trips / unit)	0.40	1.30	1.70	0.80	0.50	1.30
5175 Mississauga Road		Gross Trips	5	15	20	12	7	19
Residential Development (LUC 210)	15 Units	Gross Trip Rate (trips / unit)	0.33	1.00	1.33	0.80	0.47	1.27
Total 113 91 204 150 15					159	309		

Based on the foregoing, the proposed background developments are expected to generate 204 two (2)-way, and 309 two (2)-way trips during the roadway a.m. and p.m. peak hours, respectively. The background development site traffic volumes are illustrated in **Figure 3-1**.

The following two (2) developments are currently existing in the surrounding area, and are currently fully functioning:

- The Portico Community Church (1814 Barbertown Road); and,
- ADM Milling (1770 Barbertown Road).

The above existing land uses have been acknowledged. In our opinion, these developments will not have any additional impact on the road network and study, as they are existing developments. Any traffic that is generated from these developments that coincides with the peak periods of the proposed development, is captured in the turning movement count data collected, and subsequently included in our existing traffic analysis.

As for the peak periods of these developments, they do not coincide with the proposed development's peak periods. Since these developments do not have corresponding peak periods, the surrounding road network will be able to accommodate the associated traffic. While one (1) development is experiencing high site generated traffic, the other will be experiencing negligible traffic (off-peak).

3.3 Future Background (2021) Assessment

The future background (2021) traffic (sum of the existing, general background growth and active background developments) was analyzed using *Synchro 9.0* software. The background developments included in the assessment and results are summarized in **Table 3.2** below with v/c ratios for overall intersection operations, through movements, or shared through / turning movements greater than 0.85; and v/c ratios for exclusive movements greater than 0.95 have been shown in bold. The detailed calculations are shown in **Appendix D**. The future background traffic volumes are illustrated in **Figure 3-2** below.

Table 3.2	Future Backgrou	und (2021)	Traffic Analy	/sis
-----------	-----------------	------------	---------------	------

Intersection	Koy Moyomont	AM Peak Hour	PM Peak Hour
intersection	Key Movement	LOS (v/c)	LOS (v/c)
	EB left-through-right	C (0.06)	C (0.03)
Mississauga Road / Barbertown Road (Unsignalized)	WB left-through-right	C (0.09)	C (0.11)
	NB Left	A (<0.01)	A (0.01)
	NB through-right	A (0.36)	A (0.53)
	SB left	A (0.02)	B (0.04)
	SB through-right	A (0.50)	A (0.43)

The future background analysis reveals that the unsignalized intersection of Mississauga Road / Barbertown Road is operating with good LOS and v/c ratios during the a.m. and p.m. peak hours.

4 Site Traffic

The proposed site will consist of 24 residences including four (4) freehold detached units, 16 townhouse dwellings, and four (4) two (2)-storey back-to-back units within the converted Old Barber House on the subject property, for a total of 24 dwelling units. The existing historic Barber House will remain in place and is proposed for two-storey townhouses.

4.1 Site Generated Traffic

Trip generation forecasts are based on information contained in the *Trip Generation Manual, 9th Edition* published by the Institute of Transportation Engineers (ITE). The ITE Land Use Code (LUC) 230 (Residential Condominium / Townhouse) was selected to estimate residential townhouse trips, while LUC 210 was used for the Detached Single Family Units, for the weekday a.m., and p.m. peak hours. The equation was used instead of the average rate calculation since the equations generate higher traffic volume, which is considered more conservative. The results are summarized in **Table 4.1** in the following page.

AM peak Hour **PM Peak Hour** Unit / **Land Use Parameters GFA** In Out Total In Out Total **Gross Trips** 3 10 13 4 2 6 Single –Family Detached **Gross Trip** Housing 4 Units Rate (trips / 0.75 2.50 3.25 1.0 0.50 1.50 (LUC 210) unit) 5 Residential Condominium **Gross Trips** 2 12 14 11 16 **Common Element Gross Trip** 20 Units Townhouse Rate (trips / 0.10 0.60 0.70 0.55 0.25 0.80 (LUC 230) unit) Total 5 22 27 15 7 22

Table 4.1 Trip Generation for Proposed Development

The proposed development is expected to generate 27 two (2)-way (five (5) inbound and 22 outbound) trips during the a.m. peak hour and 22 two (2)-way (15 inbound and seven (7) outbound) trips during the p.m. peak hour. The site traffic volumes associated with the proposed development are illustrated in **Figure 4-1** and detailed calculations are provided in **Appendix E**.

4.2 Trip Distribution

Trip distribution is based on the existing traffic patterns and the available road network. The resulting distribution is summarized in **Table 4.2** below. The site traffic was assigned to the road network as per the trip distribution noted below.

Direction (to /	Street	AM Peak Hour		PM Peak Hour	
from)	Street	In	Out	In	Out
North	Mississauga Road	58%	41%	45%	55%
South	Mississauga Road	42%	59%	55%	45%
	Total	al 100%			

Table 4.2 Trip Distribution Based on Existing Traffic Pattern

4.3 Trip Assignment

The proposed site development traffic volumes noted in **Section 4.1** were assigned to the study area intersections based on the trip distribution presented in **Table 4.2**. The site traffic was assigned to the proposed site access on Mississauga Road.

5 Future Total Traffic Conditions

5.1 Future Total (2021) Assessment

The future (2021) total traffic (sum of the future (2021) background traffic and site traffic) was analyzed using *Synchro 9.0* software. The results are summarized in **Table 5.1** below with v/c ratios for overall intersection operations, through movements, or shared through / turning movements greater than 0.85; and v/c ratios for exclusive movements greater than 0.95 have been shown in bold. The future total traffic volumes are illustrated in **Figure 5-1**, respectively, and detailed calculations are provided in **Appendix F**.

Table 5.1 Future Total (2021) Traffic Analysis

Intersection	Key Movement	AM Peak Hour	PM Peak Hour
		LOS (v/c)	LOS (v/c)
	EB left-through-right	C (0.06)	C (0.03)
	WB left-through-right	C (0.12)	C (0.12)
Mississauga Road	NB Left	A (0.00)	A (0.01)
Barbertown Road (Unsignalized)	NB through-right	- (0.36)	- (0.53)
(Onsignanzea)	SB left	A (0.02)	B (0.04)
	SB through-right	- (0.50)	- (0.43)
	WB left-right	B (0.03)	C (0.02)
Mississauga Road /	NB through-right	- (0.36)	- (0.53)
Proposed Site Access (Unsignalized)	SB left	A (0.00)	A (0.01)
(Onsignanzea)	SB through	- (0.51)	- (0.45)

The future total analysis reveals that the unsignalized intersection of Mississauga Road / Barbertown Road will operate with good LOS and v/c ratio during the a.m. and p.m. peak hours. In addition, the proposed Site Access / Mississauga Road intersection will operate with good LOS and v/c ratios during the road peak hours.

5.2 Proposed Site Access

The site access for the proposed development is proposed to be a full movement access, via Mississauga Road (west side of the development).

The proposed access is expected to have negligible effect on the operations of Mississauga Road due to low site traffic volume. As discussed in **Section 5.1**, the addition of site traffic onto Mississauga Road will have no effect on the operations of Mississauga Road.

If the site access were to be provided along Barbertown Road (south of the proposed development), the traffic operations at the site access may be poor, due to no dedicated turning lanes along Barbertown Road. With the close proximity to the intersection of Barbertown Road and Mississauga Road, the queue length of vehicles waiting to turn left, eastbound, into the proposed development, may impact intersection operations. Mississauga Road currently has a two-way centre left-turn lane, which provides a safe refuge and queuing area for left turns along Mississauga Road, and will maintain an acceptable LOS along the roadway.

The proposed site access is located approximately 60m away from the intersection of Barbertown Road and Mississauga Road, and with an existing left-turn lane, the site traffic will have no impact on the intersection.

As well, closing the current site access onto Barbertown Road, and not providing access for the future proposed development on Barbertown Road, would increase the safety of the roadway, neighbourhood and the park which is located directly south of the proposed development, and minimize traffic infiltration into the neighbourhood.

6 Parking Supply Evaluation

The Site Plan proposes a total 44 parking spaces including 32 parking spaces for common element townhouse dwelling units, seven (7) spaces for common element condo / two (2)-storey townhouse visitors, and five (5) parking spaces for visitors are provided. To determine the adequacy of the proposed parking supply to service the proposed residential development, the existing in force City's Zoning By-law No. 0225-2007 Part III was reviewed and examined.

6.1 City of Mississauga Zoning By-law No.: 0225-2007

The City Zoning By-law No. 0225-2007 Part III has been reviewed. The parking rates are as follows and the detailed calculations are summarized in **Table 6.1** and **Table 6.2** below.

Table 6.1 Parking Requirements for Freehold Townhouses

Land Use	Unit/GFA	Parking Rate	Required Parking Spaces	Proposed Parking Spaces	Difference
Freehold SFU	4 Units	3.25 spaces / unit	13	16	+3

Based on the current City's Zoning By-law, the proposed development is required to provide 3.25 parking spaces per unit including visitors, for the freehold detached single family units, and is being exceeded by the proposed development.

Table 6.2 Parking Requirements for Rest of the Development

Land Use	Unit/GFA	Parking Rate	Required Parking Spaces	Proposed Parking Spaces	Difference
Common Elements Townhouse	16 Units	2 spaces / unit	32	32	-
Condominium (Two (2)-storey back-to- back within the converted Old Barber House)	4 Units	1.5 spaces / unit	6	7	+1
Residential Visitors	20 Units	0.25 spaces / unit	5	5	-
	Total		43	44	+1

Based on the current City's Zoning By-law, the proposed development is required to provide 43 parking spaces (32 parking spaces for common element townhouses, five (5) parking spaces for residential visitors and six (6) spaces for two (2)-storey townhouse residents). Therefore, the proposed parking supply will meet the requirement with 44 parking spaces.

6.1.1 Public Transit

The site is located within an area serviced by the Mississauga Transit (Miway) bus services with at least 12 bus stops within a 500m walking radius. **Figure 6-1** illustrates 500m walking distance bus stops. The transit routes include:

- Route 9 Rathburn-Millers Grove: operates along Rathburn Road to Creditview Road, continues
 to Eglinton Avenue and turns to Mississauga Road. The Route 9 ends at Meadowvale Town Centre
 Bus Terminal. In the vicinity of the site, this route operates with an average headway of 30 minutes
 during the weekday a.m. and p.m. peak hour, respectively. Figure 6-2 illustrates Route 9 road
 network;
- Route 44 Mississauga Road: operates along Mississauga Road. In the vicinity of the site, this route operates with an average headway of 18 to 19 minutes during the weekday a.m. and p.m. peak hour, respectively. Figure 6-3 illustrates Route 44 road network;
- Route 35 / Route 35A Eglinton Road: operates along Eglinton Avenue. Service starts at Islington Subway Station and it ends at Ninth Line. In the vicinity of the site, this route operates with an average headway of seven (7) minutes during the weekday a.m. and p.m. peak hour, respectively. Figure 6-4 illustrates Route 35 road network;
- Route 34 Credit Valley: operates along Eglinton Avenue. Service starts at Square One Bus Terminal and it ends at Erin Mills Town Centre Bus Terminal. In the vicinity of the site, this route operates with an average headway of 23 minutes during the weekday a.m. and p.m. peak hour, respectively. Figure 6-5 illustrates Route 34 road network; and,
- The Streetville GO Train Station is approximately a 13 minute walk from the subject development property, travelling north along Queen Street to Old Station Road.

The above bus services provide a reliable, cost effective alternative mode of traveling through the comprehensive and continually growing transit network system. This further validates the accessibility of the subject site for non-automotive users.

7 Site Circulation Assessment

The inbound and outbound movements were reviewed based on the latest Site Plan, dated October 19, 2017. The internal vehicular circulation for emergency and servicing has been reviewed. The *AutoTURN* analysis for fire and garbage trucks entering and exiting the site is illustrated in **Figure 7-1** and **Figure 7-2**. The garbage truck dimensions were based on the standards set out in the Region of Peel's Waste Collection Design Standards Manual, for a 11.93m long Fully-Automated Waste Collection Vehicle.

In addition, based on the discussion with the Regional staff, given the constraints of the site, the Region allowed exception to the backup distance requirement with following conditions:

- Developer will provide space for storage of waste carts for units 17-20, on the day of collection, carts to be set out close to curb for collection;
- Waste collection vehicle driver will backup for approximately 15m for automated collection from some units, and for the remaining units (units 11-16) she / he will manoeuver the carts manually for collection; and,
- Common collection pad will not be allowed.

From the analysis, there is adequate manoeuvring area on-site to enable garbage and other similar service trucks to exit the site in a forward manner without adversely affecting the adjacent public roadways. Corresponding with the Region Peel included in **Appendix G**.

In addition, *AutoTURN* analysis for passenger vehicles illustrated in **Figure 7-3**. As per the *AutoTurn* Diagram a typical 5m (TAC-1999) vehicle, can enter / exit the parking stall with a four (4) point turn, however, AutoTurn analysis completed it for compact car as well.

Compact cars are classified by volume in North America under the United States Environmental Protection Agency (EPA). As such, a sample compact car (Toyota Auris) was utilized for modelling and analysis purposes. The sample car has a length of 4.245m and a total width of 1.76m. This sample car was used to demonstrate parking maneuvers for the visitor parking space #4. Other examples of possible compact cars include:

- Toyota Corolla;
- Honda Civic;
- Chevrolet Cruze;
- Hyundai Elantra; and,
- · Ford Focus.

Based on the analysis it is determined that 4.245m passenger vehicle can enter / exit the parking space with ease.

8 Transportation Demand Management Plan

Transportation Demand Management (TDM) Plan refers to a variety of strategies to reduce congestion, minimize the number of single-occupant vehicles, encourage non-auto modes of travel, and reduce vehicle dependency to create a sustainable transportation system. In short, a TDM plan works to change how, when, where, and why people travel.

The TDM measures have been prepared to address the following required transportation demand management objectives:

- Provision of facilities / operations to minimize single occupant vehicle (SOV) access to the study area and encourage the use of alternate transportation modes, particularly transit; and,
- Identify the operational and financial roles and responsibilities of the landowner including program development, implementation and ongoing management and operations of the travel demand management plan / program.

The TDM education and promotion related strategies support mobility by making the public aware of the options available and actively encourages the use of alternative transportation. These marketing initiatives may take the form of general marketing to targeted outreach. TDM marketing and promotion approaches include:

- Area marketing campaigns Promoting transit services, carpooling or other TDM;
- Education Programs Campaigns to inform the public about the specifics of services; and,
- Employer Based Outreach Partnerships with employers to support TDM.

8.1 TDM Programs

8.1.1 Area Marketing Campaigns

Area marketing campaigns are premised on the objective of behavioural change. The methods of persuasion include conveying messages of:

- Personal benefit;
- Social benefit; and,
- Adhering to the "social norm".

It is recognized that individuals are more likely to change their current behaviour or continue that behaviour if it is the social norm amongst their peers.

One of the first area marketing TDM campaigns within the GTA was established by Metropolitan Toronto in 1994. The Diamond Lane Campaign was implemented in co-ordination with the expansion of the High Occupancy Lane network. The campaign included billboard, transit vehicle, radio, and newspaper ads. Transportation Management Association (TMA)'s can help facilitate TDM directives by developing promotional materials, advocate for funding, conduct transportation fairs, and many other things.

8.1.2 Education Programs and Information Services

The effectiveness of transportation systems to provide mobility to the public is only as good as the awareness of the public to the services provided. Education programs and information services (such as call-lines and internet based route planners) can supplement marketing campaigns, increasing the commuter's understanding of their mobility options.

Miway provides http://www.mississauga.ca/portal/miway, which is a map based navigator. Progressive trip planners including web based trip data, can add to the commuter's understanding of travel options by providing real time information.

A Denver, Colorado TMA (Transportation Solutions) managed a program which was designed to improve ridership. Before-and-after surveys were to gauge the impact of the efforts. The results clearly demonstrated the direct impact of the project on improved perceptions of bus transportation in the area. The results showed an increase in awareness:

- 17% increase in those who "know which bus routes to take to work / school"; and,
- 19% increase in those who feel they have "the information I need to ride the bus".

8.1.3 Walking / Cycling

The City has a long-standing commitment to an off-road trail system, having built over 250km of pathways and trails in parks and greenspaces across the City. Pathways in the neighbourhood parks provide cycling and walking friendly connections to schools, community centres and libraries. In addition, cycling safety programs were offered such as CAN-BIKE courses and community safety workshops. The Road Safety Handbook was published and includes a chapter on cycling safety.

8.1.4 Bicycle Parking

As per the City's comments, dated June 8, 2017, "the applicant is required to provide short term (outdoor) and long term (indoor) bicycle storage facilities for the purpose of a minimum rate of 0.08 outdoor and 0.70 indoor parking spaces per units (in a close vicinity to the main access)."

Short-term bicycle parking is typically provided in close proximity to entrances via high tensile strength racks. Long term bicycle parking is intended for use over several hours or overnight and the residents will also likely use their garages for bicycle storage and parking on a long term basis.

The proposed development will provide a brick shed, to be located to the east of the Barber House that will serve as a secure storage facility for both residents and visitors wishing to cycle to and from the site, thus further reducing the single occupancy vehicle (SOV) mode. The brick shed will serve as both short term and long term bicycle parking, with an area of 6.54m².

Based on the discussion with staff, **Table 8.1** below summarizes the required bicycle parking spaces for the proposed development, based on short-term and long-term parking.

Table 8.1 Required Bicycle Parking Supply

· · · · · · · · · · · · · · · · · · ·				
Type of Bicycle Parking	Rate	# Parking Required		
Short-Term (outdoor) (four (4) units) two (2)- storey back-to-back within the converted Old Barber House	0.08/unit	1 spaces		
Long-Term (indoor) (four (4) units) two (2)- storey back-to-back within the converted Old Barber House	0.70/unit	3 spaces		

The proposed development will provide a total of one (1) bicycle parking spaces for visitors (short-term) and three (3) bicycle parking spaces for residents (long-term) as summarized in **Table 8.1**.

8.1.5 Carpooling

The Smart Commute program has been established in the form of 10 transportation management associations (TMAs) across the GTHA. The Smart Commute TMAs are supported by Metrolinx to coordinate and implement TDM initiatives.

Smart Commute – Mississauga is a TMA committed to reducing traffic congestion, improving air quality and advocating for sustainable transportation within the study area. Since inception in 2001, Smart Commute has collectively reduced approximately 1.2 million single-occupant vehicle (SOV) trips, which avoided more than 61 million vehicle kilometres traveled (VKT) and prevented the emission of over 14,000 tonnes of greenhouse gases (GHG) and 88 tonnes of smog-causing air pollutants. Smart Commute works to:

- Implement employee trip reduction programs at local workplaces;
- Decrease traffic congestion, and improve air quality and health by reducing vehicle emissions;
- Improve employee productivity and morale, and reduce employee turnover;
- Advocate for improved transit service, and increased local transportation infrastructure;
- Bus-only and cycling lanes, and a wider network of subway and light rapid transit;
- Promote the benefits of transit-supportive development and smart-growth strategies;
- Encourage legislative flexibility in support of high-value, cost effective transportation strategies such as vanpools, telework, transit subsidies and shuttle services; and,
- Increase opportunities for TMA collaboration with business and government.

Smart Commute offers one-on-one support for staff to use sustainable modes of transportation and provides promotions and incentives on an on-going basis. A web based car matching tool is provided to allow carpoolers to easily identify ride matching options among a large membership base and form carpooling arrangements.

9 Conclusions

Based on our analysis, our findings and recommendations are as follows:

- The study area intersection operates at good LOS in the existing traffic conditions during a.m. and p.m. peak hours;
- The study area intersection is operating at good LOS in the future (2021) background traffic conditions during a.m. and p.m. peak hours;
- The proposed development is expected to generate 27 two (2)-way (five (5) inbound and 22 outbound) trips during the a.m. peak hour and 22 two (2)-way (15 inbound and seven (7) outbound) trips during the p.m. peak hour;
- Similar to the future 2021 background traffic analysis, the study area intersections will operate at good LOS in the future (2021) total traffic conditions during a.m. and p.m. peak hours;
- Based on the current City Zoning By-law, the proposed development is required to provide 13 parking spaces for the freehold detached units, while 16 spaces are provided. In addition, the proposed development is required to provide 43 parking spaces (32 parking spaces for common element townhouses, five (5) parking spaces for residential visitors and six (6) spaces for two (2)-storey townhouses). The proposed parking supply of 44 will successfully accommodate the parking requirements during peak operation;
- The results of the *AutoTURN* assessment indicate that there is sufficient manoeuvring space within the site for trucks to enter / egress the site with ease; and,
- The proposed development will have minimal impact on the surrounding road network.

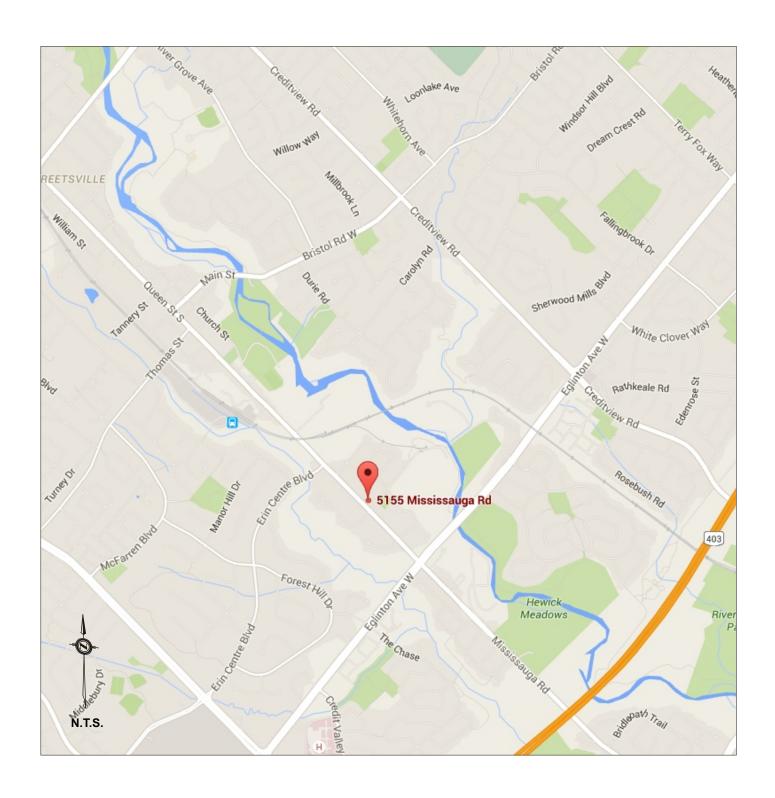


Figure 1-1 Site Location



Traffic Impact and Parking Study

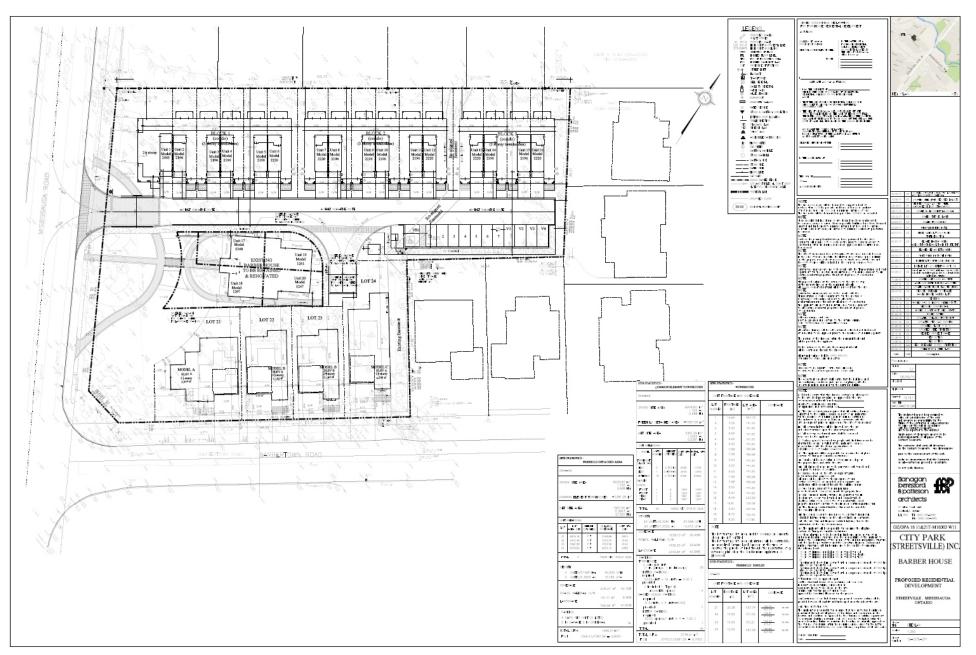


Figure 1-2 Proposed Site Plan



Traffic Impact and Parking Study



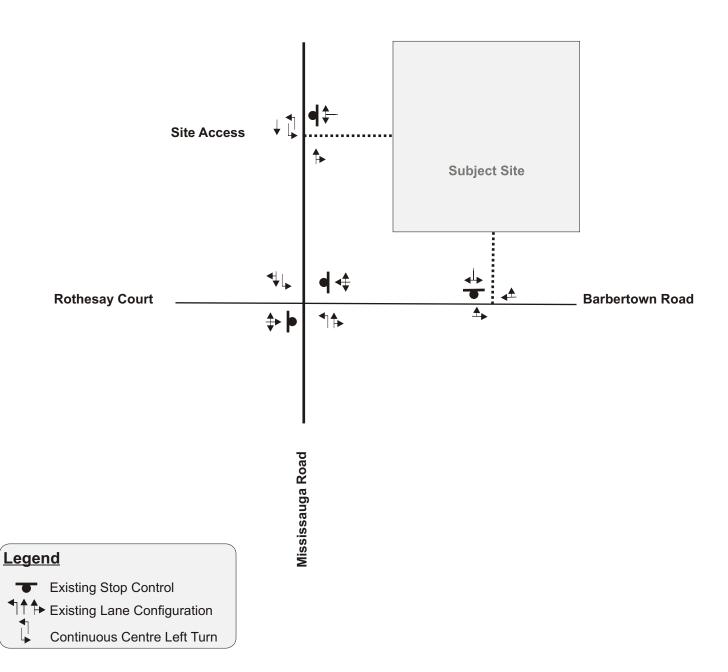
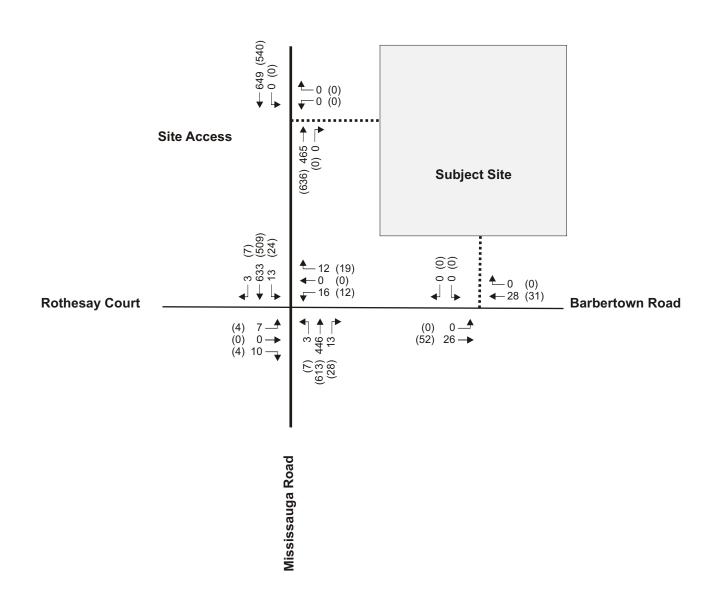


Figure 2-1 Existing Lane Configurations

Traffic Impact and Parking Study





Legend

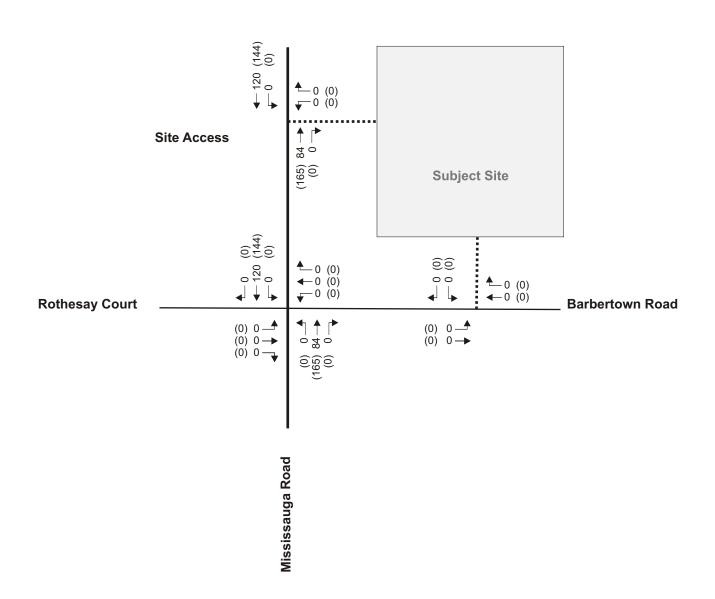
99 AM Peak Hour Volume(99) PM Peak Hour Volume

Figure 2-2 Existing Traffic Volumes



Traffic Impact and Parking Study





Legend

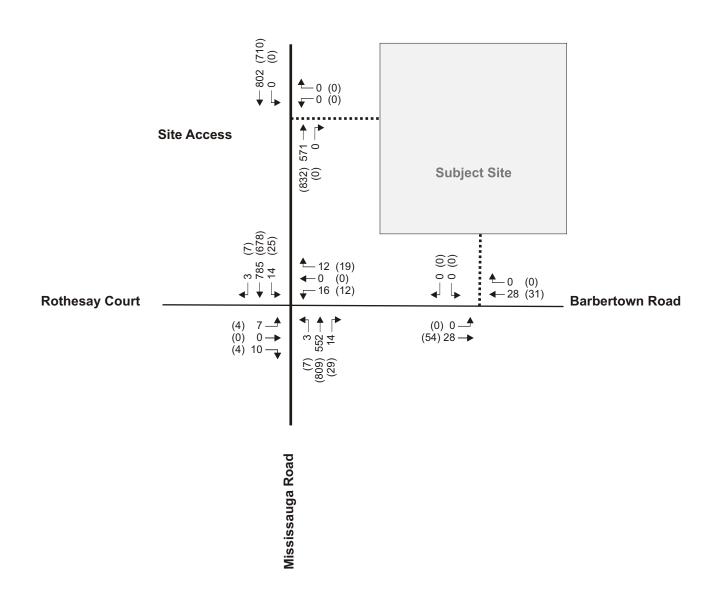
99 AM Peak Hour Volume (99) PM Peak Hour Volume

Figure 3-1 Background Development Site Traffic Volumes



Traffic Impact and Parking Study





Legend

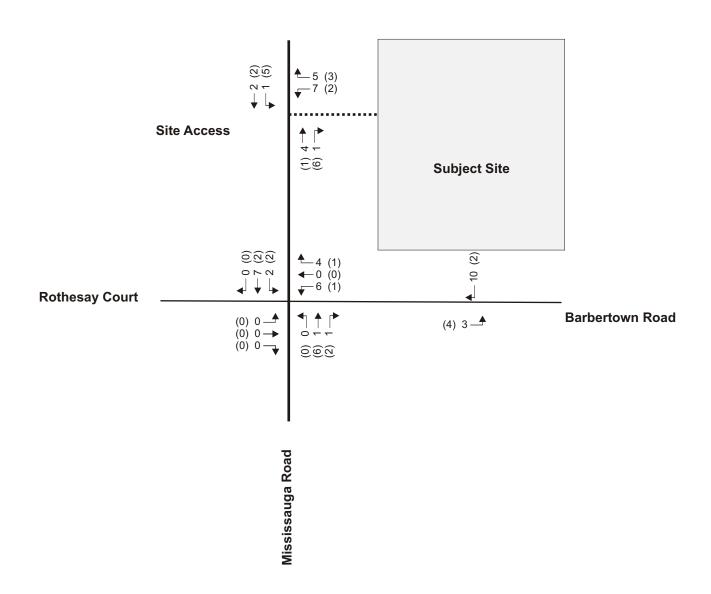
99 AM Peak Hour Volume (99) PM Peak Hour Volume

Figure 3-2 Future Background Traffic Volumes



Traffic Impact and Parking Study





Legend

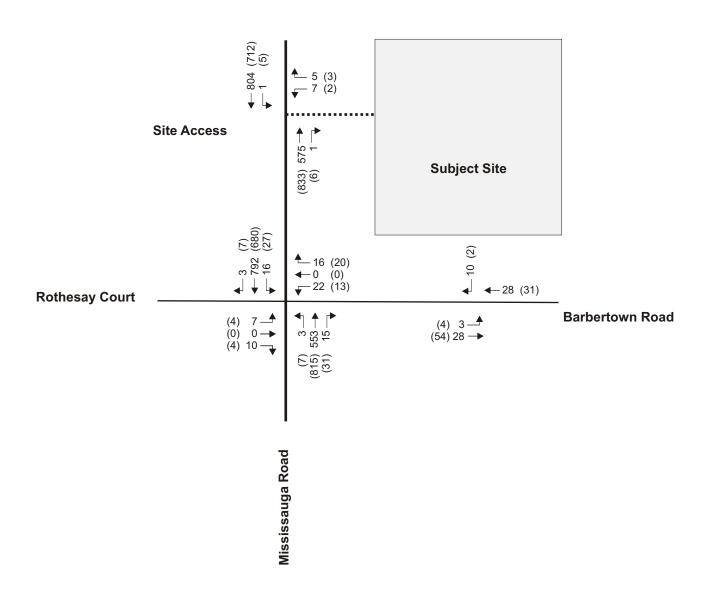
99 AM Peak Hour Volume(99) PM Peak Hour Volume

Figure 4-1 Site Traffic Volumes



Traffic Impact and Parking Study





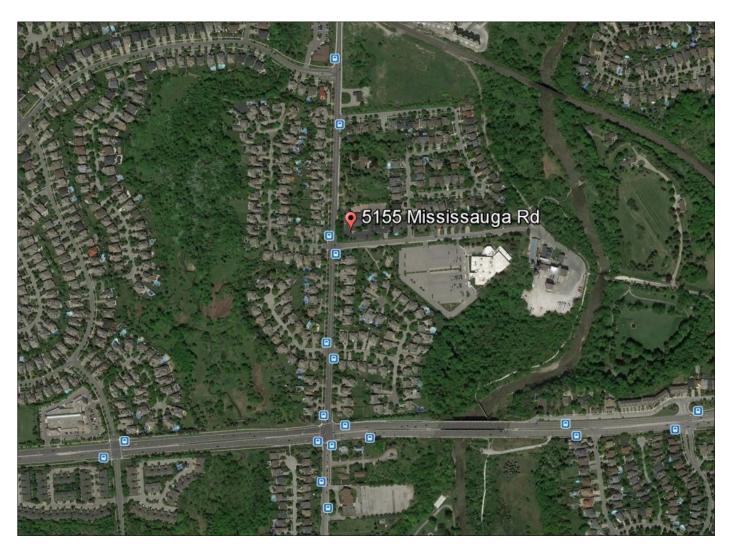
Legend

99 AM Peak Hour Volume(99) PM Peak Hour Volume

Figure 5-1 Future Total Traffic Volumes









Traffic Impact and Parking Study

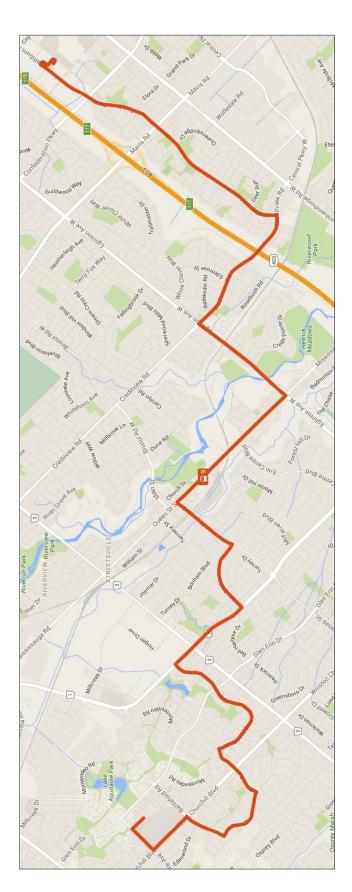




Figure 6-2 Miway - Route 9 - Ratburn / Miller Drive



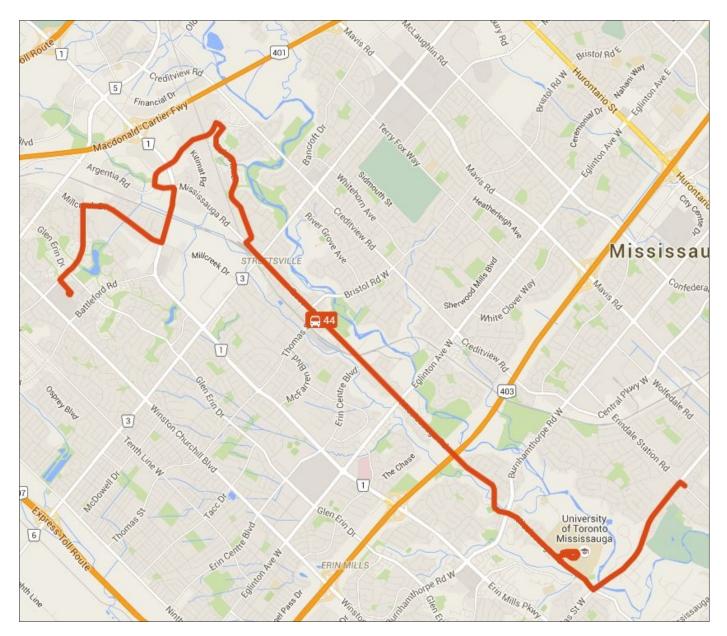


Figure 6-3 Miway - Route 44 - Mississauga Road





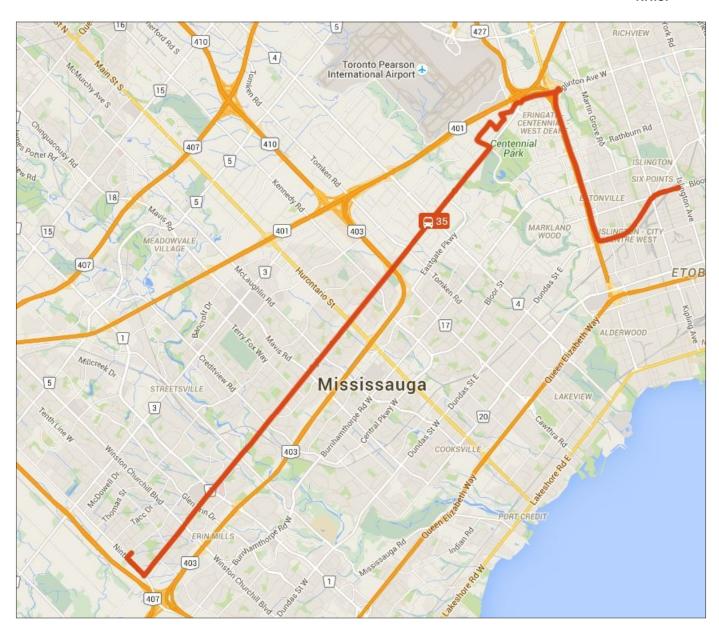


Figure 6-4 Miway - Route 35/35A - Eglinton Avenue



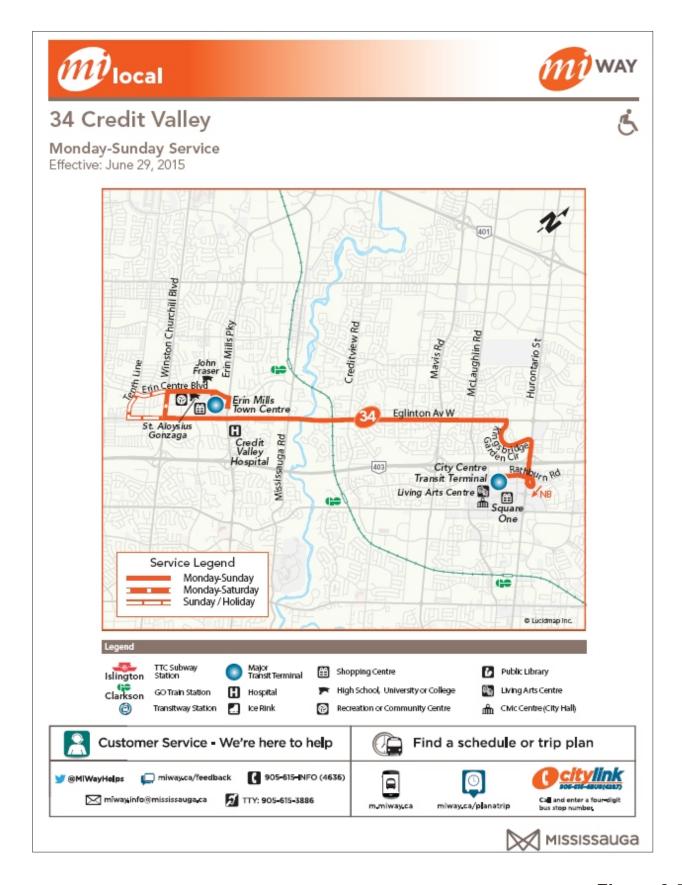
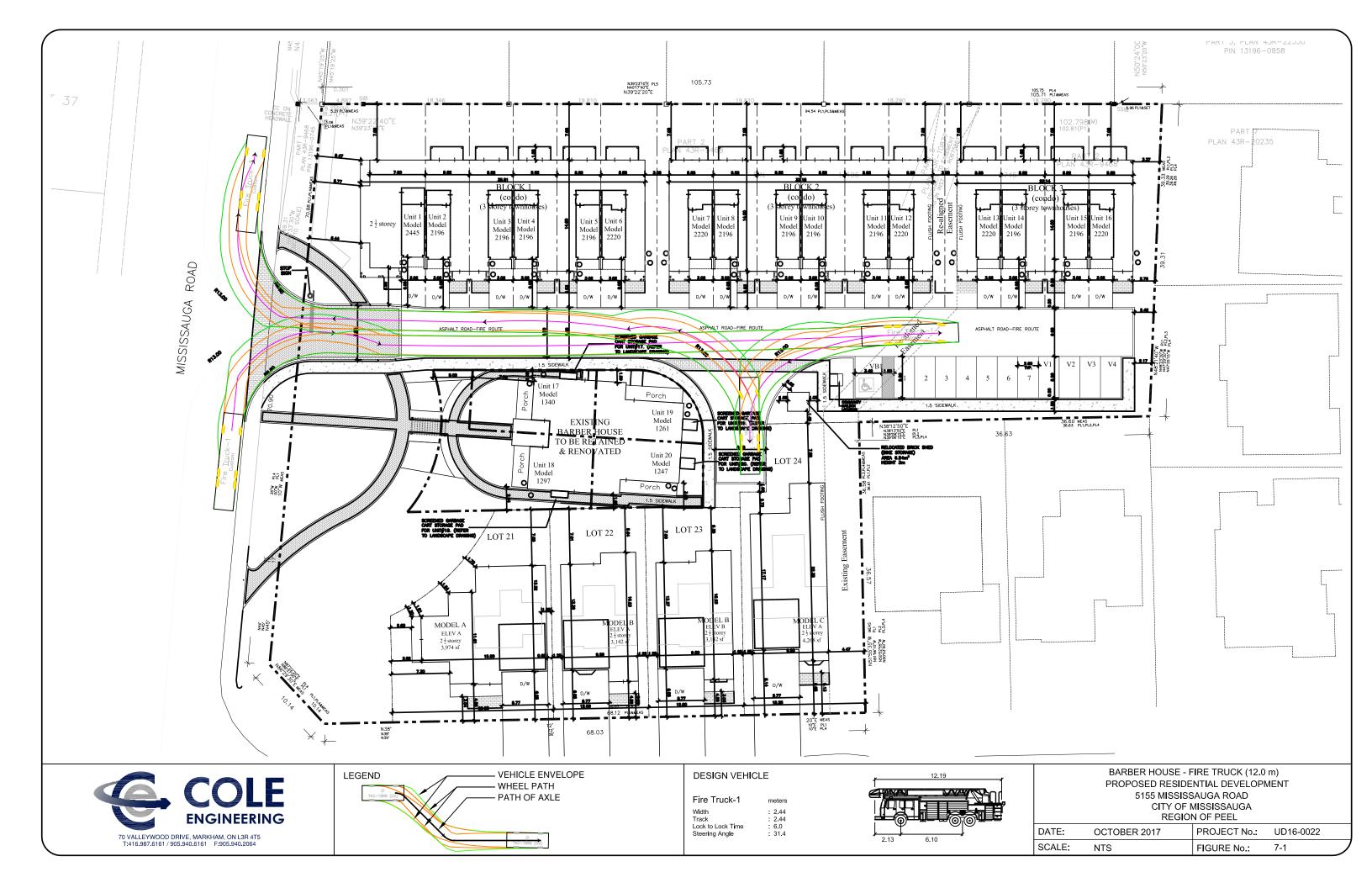
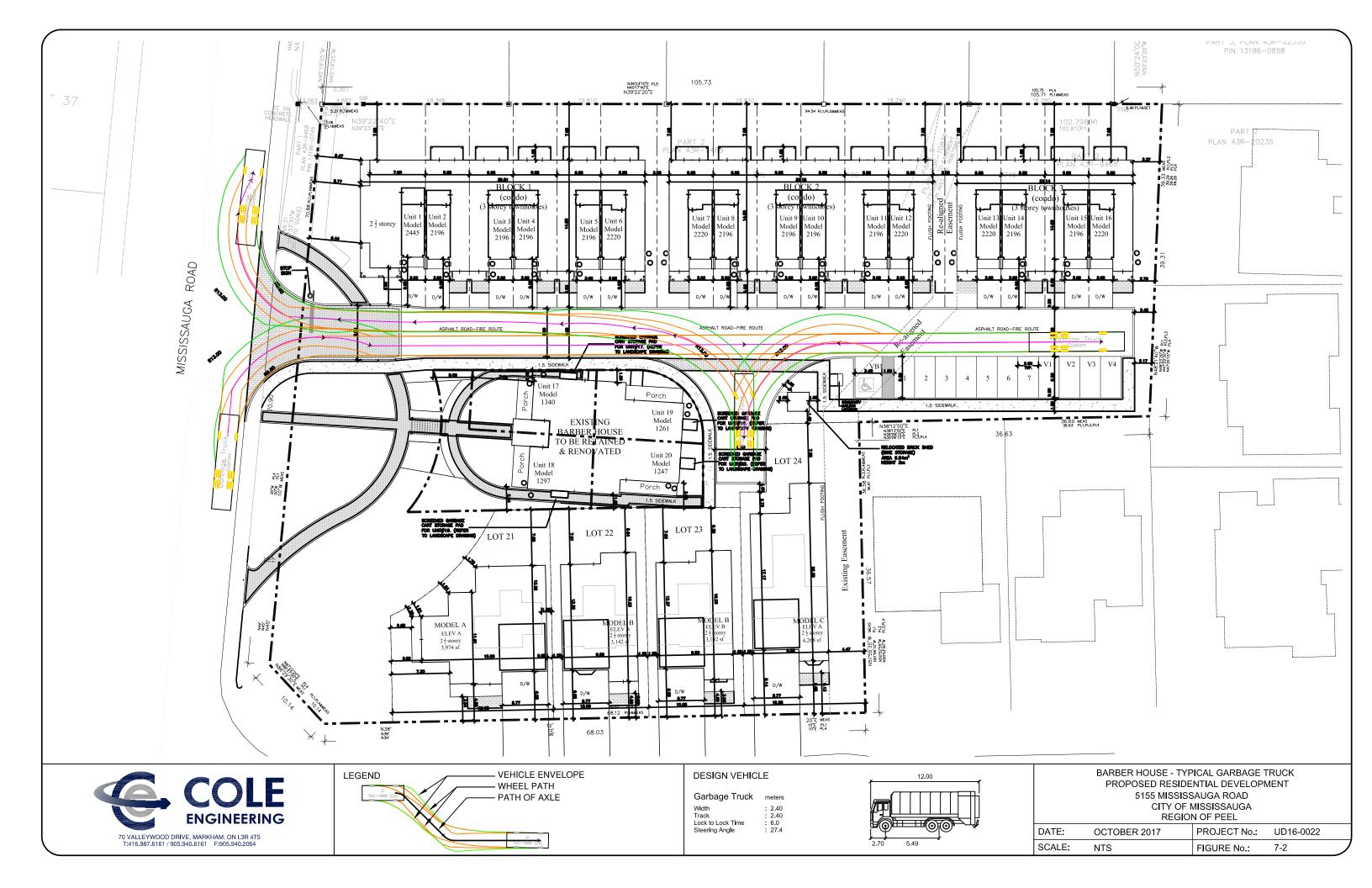
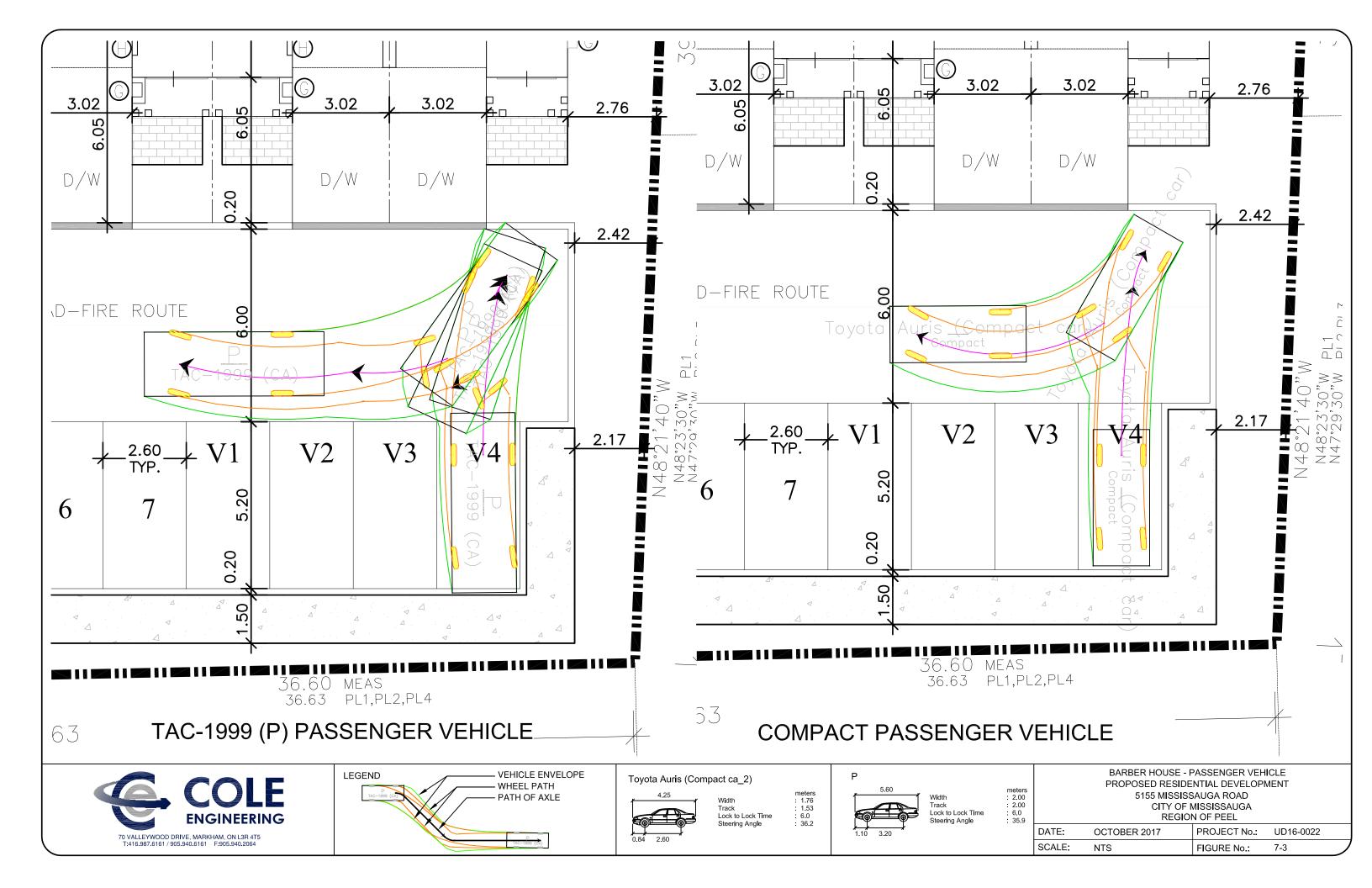


Figure 6-5 Miway - Route 34 - Credit Valley









APPENDIX A Existing Traffic Data



		4CCU-11	rattic ii	TC.	
Morning Pe	ak Diag	ram	Specified From: 7		One Hour Peak From: 8:00:00 To: 9:00:00
Site #: 16020	ssauga 600001 ssauga Rd & B eb-16	arbertown Rd	Person o	repared:	
** Non-Signalized I	ntersection	**	Major Ro	oad: Mississa	uga Rd runs N/S
North Leg Total: 1114 North Entering: 649 North Peds: 0 Peds Cross: ✓	Heavys 0 Trucks 0 Cars 3 Totals 3	4 0	27 4 618	Heavys 10 Trucks 2 Cars 453 Totals 465	East Leg Total: 54 East Entering: 28 East Peds: 1 Peds Cross: X
Heavys Trucks Cars Tota 0 0 6 6 Rott	als Ct	↓	Mississauga Rd N E		Cars Trucks Heavys Totals 11
Heavys Trucks Cars Tota 0 0 7 7 0 0 0 0 0 10 10	als \triangle		s A		ertown Rd Cars Trucks Heavys Totals
0 0 17	1	Mississauga R	^q 7	Γ ₁ ∕	20 0 6 26
Peds Cross: X West Peds: 4 West Entering: 17 West Leg Total: 23	Cars 629 Trucks 4 Heavys 26 Totals 659	Tru Hea	Cars 3 435 cks 0 1 cvys 0 10 tals 3 446	0 1 4 14	Peds Cross: ► South Peds: 0 South Entering: 462 South Leg Total: 1121
	ı	Com	ments		1

Comments



	4	ACCU-I	rattic ii	IC.											
Afternoon	Peak Dia	agram		1 Period 6:00:00 8:00:00	One Hour Peak From: 17:00:00 To: 18:00:00										
Site #: 1602 Intersection: Missi TFR File #: 1	issauga 600001 issauga Rd & B eb-16	arbertown Rd	Person c	repared:											
** Non-Signalized	North Leg Total: 1176 Heavys 0 8 0 8 Heavys 8 East Leg Total: 83														
0 0 14 14	Person prepared: Person checked: Person checked: Person checked: Person checked: Person che														
$\begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 4 & 4 \\ \hline 0 & 0 & 8 & 4 \end{bmatrix}$		Mississauga R			Cars Trucks Heavys Totals 52 0 0 52										
Peds Cross: X West Peds: 0 West Entering: 8 West Leg Total: 22	Cars 515 Trucks 1 Heavys 9 Totals 525	Tri Hea	Cars 7 604 ucks 0 1 avys 0 8 otals 7 613	0 1 0 8	Peds Cross: M South Peds: 0 South Entering: 648 South Leg Total: 1173										
		Com	ments												



Total Count Diagram

Municipality: Mississauga

Site #: 1602600001

Intersection: Mississauga Rd & Barbertown Rd

TFR File #:

Peds Cross:

Count date: 10-Feb-16 Weather conditions:

Person counted: Person prepared:

Person checked:

Major Road: Mississauga Rd runs N/S

** Non-Signalized Intersection **

North Leg Total: 3968 Heavys 0 North Entering: 2009 Trucks 0 North Peds: Cars 20

62 2 10 1864 50 Totals 20 1936 53

Heavys 50 Trucks 10 1934 Cars 1899 Totals 1959

East Leg Total: 248 East Entering: 120 East Peds: X Peds Cross:



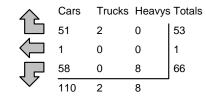




64

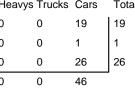
11

Mississauga Rd



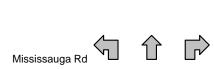
Barbertown Rd

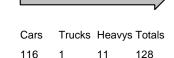
Heavys Trucks Cars Totals 0 19 19 0 0 1 1 0





Rothesay Ct





X Peds Cross: West Peds: West Entering: West Leg Total: 87

Cars 1948 Trucks 10 Heavys Totals 2028



1914 Cars 20 1829 65 Trucks 0 8 0 8 Heavys 9 59 50 Totals 20 1887 74

Peds Cross: M South Peds: South Entering: 1981 South Leg Total: 4009

Comments



Accu-Traffic Inc. Traffic Count Summary

Municipality: Mississauga Intersection: Mississauga Rd & Barbertown Rd Count Date: 10-Feb-16 North Approach Totals **South Approach Totals** North/South Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Hour Total Hour Total Total Grand Grand **Ending** Peds **Ending** Peds Approaches Thru Right Thru Right Left Total Total 7:00:00 0 0 0 0 0 0 7:00:00 0 0 0 0 0 2 429 747 4 301 8:00:00 7 420 0 8:00:00 13 318 1 13 9:00:00 633 649 0 1111 9:00:00 3 446 13 462 0 0 0 16:00:00 0 0 0 0 0 16:00:00 0 0 0 0 374 8 6 20 17:00:00 9 391 0 944 17:00:00 527 553 0 24 509 540 1188 18:00:00 648 0 18:00:00 613 28 Totals: 1936 20 2009 3990 S Totals: 1887 1981 74 **East Approach Totals West Approach Totals** East/West Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Total Hour Hour Total Total Grand Grand **Ending** Peds **Ending** Peds Approaches Right Left Thru Right Left Thru Total Total 7:00:00 0 0 0 0 7:00:00 0 0 1 15 9 25 2 7 8:00:00 1 38 8:00:00 5 1 13 4 9:00:00 16 0 12 28 1 45 9:00:00 7 0 10 17 4 16:00:00 0 0 0 16:00:00 0 0 0 0 1 1 1 0 2 17:00:00 22 0 13 35 2 42 17:00:00 0 5 7 1 3 4 8 0 18:00:00 12 19 31 39 18:00:00 0 Totals: 66 120 166 W Totals: 26 46 9 **Calculated Values for Traffic Crossing Major Street** Hours Ending: 9:00 16:00 17:00 18:00 0:00 7:00 8:00 0:00 Crossing Values: 23 0



Count	Date:	Passenger Cars - North Approach				0001													1	
		Passeng	er Cars -	North A	pproach			True	cks - Nort	h Approa	ach			He	avys - No	rth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	57	57	0	0	0	0	1	1	0	0	0	0	2	2	0	0	0	0
7:30:00	0	0	133	76	0	0	0	0	2	1	0	0	0	0	6	4	0	0	0	0
7:45:00	1	1	270	137	0	0	1	1	2	0	0	0	0	0	15	9	0	0	0	0
8:00:00	6	5	396	126	2	2	1	0	3	1	0	0	0	0	21	6	0	0	0	0
8:15:00	7	11	551	155	2	0	1	0	3	0	0	0	2	2	31	10	0	0	0	0
8:30:00	8	1	686	135	5	3	1	0	5	2	0	0	2	0	40	9	0	0	0	0
8:45:00	14	6	843	157	5	0	1	0	7	2	0	0	2	0	46	6	0	0	0	0
9:00:00	17	3	1000	157	5	0	1	0	7	0	0	0	2	0	46	0	0	0	0	0
9:15:00	17	0	1000	0	5	0	1	0	7	0	0	0	2	0	46	0	0	0	0	0
16:00:00	17	0	1000	0	5	0	1	0	7	0	0	0	2	0	46	0	0	0	0	0
16:15:00	17	0	1096	96	6	11	1	0	9	2	0	0	2	0	49	3	0	0	0	0
16:30:00	22	5	1188	92	8	2	1	0	9	0	0	0	2	0	51	2	0	0	0	0
16:45:00	23	11	1268	80	13	5	1	0	9	0	0	0	2	0	52	11	0	0	0	0
17:00:00	26	3	1364	96	13	0	1	0	9	0	0	0	2	0	54	2	0	0	0	0
17:15:00	29	3	1498	134	16	3	1	0	9	0	0	0	2	0	57	3	0	0	0	0
17:30:00	32	3	1630	132	17	1	1	0	9	0	0	0	2	0	59	2	0	0	0	0
17:45:00	39	7	1754	124	17	0	1	0	10	11	0	0	2	0	61	2	0	0	0	0
18:00:00	50	11	1864	110	20	3	1	0	10	0	0	0	2	0	62	11	0	0	1	1
18:15:00	50	0	1864	0	20	0	1	0	10	0	0	0	2	0	62	0	0	0	1	0
18:15:15	50	0	1864	0	20	0	1	0	10	0	0	0	2	0	62	0	0	0	1	0



Count	Date:	Passenger Cars - East Approach				0001														
		Passen	ger Cars	- East Ap	proach			Tru	cks - Eas	t Approa	ch			Не	eavys - Ea	ast Appro	pach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	3	3	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	4	1	1	1	2	1	0	0	0	0	0	0	0	0	0	0	0	0	1	1
7:45:00	5	1	1	0	4	2	0	0	0	0	0	0	1	1	0	0	0	0	2	1
8:00:00	13	8	1	0	9	5	0	0	0	0	0	0	2	11	0	0	0	0	2	0
8:15:00	15	2	1	0	10	1	0	0	0	0	0	0	2	0	0	0	0	0	2	0
8:30:00	19	4	1	0	12	2	0	0	0	0	1	1	3	1	0	0	0	0	2	0
8:45:00	22	3	1	0	17	5	0	0	0	0	1	0	3	0	0	0	0	0	2	0
9:00:00	28	6	1	0	20	3	0	0	0	0	1	0	3	0	0	0	0	0	3	1
9:15:00	28	0	1	0	20	0	0	0	0	0	1	0	3	0	0	0	0	0	3	0
16:00:00	28	0	1	0	20	0	0	0	0	0	1	0	4	1	0	0	0	0	3	0
16:15:00	34	6	1	0	22	2	0	0	0	0	1	0	4	0	0	0	0	0	4	1
16:30:00 16:45:00	37 42	3	1	0	27 27	5	0	0	0	0	2	1	6	0	0	0	0	0	4	0
17:00:00	42	5 5	1	0	32	0 5	0	0	0	0	2	0	7	1	0	0	0	0	5 5	0
17:00:00	47	0	1	0	42	10	0	0	0	0	2	0	7	0	0	0	0	0	6	1
17:13:00	51	4	1	0	42	0	0	0	0	0	2	0	8	1	0	0	0	0	6	0
17:45:00	55	4	1	0	48	6	0	0	0	0	2	0	8	0	0	0	0	0	6	0
18:00:00	58	3	1	0	51	3	0	0	0	0	2	0	8	0	0	0	0	0	8	2
18:15:00	58	0	1	0	51	0	0	0	0	0	2	0	8	0	0	0	0	0	8	0
18:15:15	58	0	1	0	51	0	0	0	0	0	2	0	8	0	0	0	0	0	8	0
10.10.10	- 00		'		J 01															



Count	Date:	Passenger Cars - South Approach				0001														
		Passeng	er Cars -	South A	pproach			Truc	cks - Sout	h Appro	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	45	45	3	3	0	0	1	1	0	0	0	0	7	7	0	0	0	0
7:30:00	0	0	97	52	5	2	0	0	1	0	0	0	0	0	9	2	1	1	0	0
7:45:00	1	1	180	83	8	3	0	0	2	1	0	0	0	0	14	5	2	1	1	1
8:00:00	4	3	282	102	10	2	0	0	2	0	0	0	0	0	17	3	3	1	1	0
8:15:00	4	0	390	108	12	2	0	0	2	0	0	0	0	0	20	3	4	1	1	0
8:30:00	4	0	494	104	14	2	0	0	2	0	0	0	0	0	24	4	4	0	1	0
8:45:00	6	2	609	115	17	3	0	0	3	1	0	0	0	0	27	3	6	2	1	0
9:00:00	7	1	717	108	19	2	0	0	3	0	0	0	0	0	27	0	7	1	1	0
9:15:00	7	0	717	0	19	0	0	0	3	0	0	0	0	0	27	0	7	0	1	0
16:00:00	7	0	717	0	19	0	0	0	3	0	0	0	0	0	27	0	7	0	1	0
16:15:00	10	3	824	107	23	4	0	0	6	3	0	0	0	0	35	8	8	1	1	0
16:30:00	12	2	948	124	29	6	0	0	6	0	0	0	0	0	40	5	8	0	1	0
16:45:00	12	0	1080	132	32	3	0	0	6	0	0	0	0	0	41	1	8	0	1	0
17:00:00	13	1	1225	145	37	5	0	0	7	11	0	0	0	0	42	11	9	1	1	0
17:15:00	15	2	1378	153	38	11	0	0	7	0	0	0	0	0	45	3	9	0	1	0
17:30:00	15	0	1536	158	44	6	0	0	8	1	0	0	0	0	46	1	9	0	1	0
17:45:00	17	2	1693	157	55	11	0	0	8	0	0	0	0	0	47	1	9	0	1	0
18:00:00	20	3	1829	136	65	10	0	0	8	0	0	0	0	0	50	3	9	0	1	0
18:15:00	20	0	1829	0	65	0	0	0	8	0	0	0	0	0	50	0	9	0	1	0
18:15:15	20	0	1829	0	65	0	0	0	8	0	0	0	0	0	50	0	9	0	1	0



Count	Date.	Date: 10-Feb-16 Site #: 1602600001 Passenger Cars - West Approach											1						1	
		Passen	ger Cars -	West A	pproach			Tru	cks - Wes	t Approa	ch			He	avys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Ri	ght	Le	eft	Th	ru	Ri	ght	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	2	1	0	0	1	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	3	1	1	1	3	2	0	0	0	0	0	0	0	0	0	0	0	0	1	1
7:45:00	3	0	1	0	4	1	0	0	0	0	0	0	0	0	0	0	0	0	3	2
8:00:00	6	3	1	0	7	3	0	0	0	0	0	0	0	0	0	0	0	0	4	11
8:15:00	9	3	1	0	10	3	0	0	0	0	0	0	0	0	0	0	0	0	8	4
8:30:00	11	2	1	0	13	3	0	0	0	0	0	0	0	0	0	0	0	0	8	0
8:45:00	13	2	1	0	16	3	0	0	0	0	0	0	0	0	0	0	0	0	8	0
9:00:00	13	0	1	0	17	1	0	0	0	0	0	0	0	0	0	0	0	0	8	0
9:15:00	13	0	1	0	17	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0
16:00:00	13	0	1	0	17	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0
16:15:00	13	0	1	0	18	1	0	0	0	0	0	0	0	0	0	0	0	0	8	0
16:30:00	14	11	1	0	19 21	1	0	0	0	0	0	0	0	0	0	0	0	0	9	1
16:45:00	15	1	1	0		2	0	0	0	0	0	0	0	0	0	0	0	0	9	0
17:00:00 17:15:00	15 15	0	1	0	22 22	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0
17:15:00	18	3	1	0	24	2	0	0	0	0	0	0	0	0	0	0	0	0	9	0
17:30:00	19	3 1	1	0	24	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0
18:00:00	19	0	1	0	26	2	0	0	0	0	0	0	0	0	0	0	0	0	9	0
18:15:00	19	0	1	0	26	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0
18:15:15	19	0	1	0	26	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0
10.10.10	10				20															



Accu-Traffic Inc. **Specified Period Afternoon Peak Diagram One Hour Peak** From: 17:00:00 From: 17:00:00 To: 19:00:00 To: 18:00:00 Weather conditions: Municipality: Mississauga Site #: 1620000001 Intersection: Mississauga Rd & 5155 Mississaug Person counted: TFR File #: Person prepared: Count date: 16-Dec-16 Person checked: ** Non-Signalized Intersection ** Major Road: Mississauga Rd runs N/S North Leg Total: 1045 Heavys 0 9 Heavys 8 East Leg Total: 17 North Entering: 484 Trucks 0 Trucks 3 East Entering: North Peds: Cars 468 6 474 Cars 550 East Peds: X Peds Cross: Totals 478 6 Totals 561 Peds Cross: Mississauga Rd Trucks Heavys Totals 5 0 5155 Mississauga Rd West driveway Trucks Heavys Totals Cars 0 0 8 Mississauga Rd Cars 473 Peds Cross: M Cars 546 2 548 Trucks 1 Trucks 3 0 3 South Peds: 0 8 Heavys Heavys 8 South Entering: 559 557 2 Totals 483 **Totals** South Leg Total: 1042

Comments



Total Count Diagram

Municipality: Mississauga

Site #: 1620000001

Intersection: Mississauga Rd & 5155 Mississaug

TFR File #:

Count date: 16-Dec-16 Weather conditions:

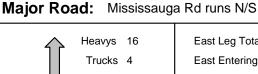
Person counted: Person prepared: Person checked:

** Non-Signalized Intersection **

North Leg Total: 1984 North Entering: 939 North Peds: Peds Cross:

Heavys 16 0 16 2 Trucks 2 0 Cars 909 12 921

Totals 927 12

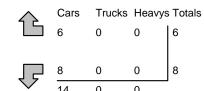


Cars 1025 Totals 1045 East Leg Total: 37 East Entering: East Peds: X

Peds Cross:







5155 Mississauga Rd West driveway



Cars 917 Trucks 2 Heavys 935

Totals

1030 Cars 1019 Trucks 4 0 4 0 16 Heavys 16 1039 Totals 11

Cars Trucks Heavys Totals 23 0 23 0

> Peds Cross: M South Peds: South Entering: 1050 South Leg Total: 1985

Comments



Accu-Traffic Inc. Traffic Count Summary

Municipality: Mississauga Intersection: Mississauga Rd & 5155 Mississau Count Date: 16-Dec-16 **North Approach Totals South Approach Totals** North/South Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Total Hour Hour Total Total Grand Grand **Ending** Peds **Ending** Peds Approaches Left Thru Right Thru Right Total Left Total 17:00:00 17:00:00 0 0 0 0 0 0 0 0 0 0 6 478 0 484 0 1043 18:00:00 0 557 2 559 18:00:00 0 19:00:00 6 449 455 946 19:00:00 0 482 491 0 Totals: 927 939 0 1989 S Totals: 1039 1050 0 **East Approach Totals West Approach Totals** East/West Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Hour Total Hour Total Total Grand Grand **Ending** Peds **Ending** Peds Approaches Right Left Thru Right Total Left Thru Total 17:00:00 17:00:00 0 0 0 0 0 0 0 0 0 18:00:00 5 0 4 3 9 18:00:00 0 0 0 0 9 0 19:00:00 3 2 5 5 19:00:00 0 0 0 0 0 Totals: 8 0 14 W Totals: 0 0 **Calculated Values for Traffic Crossing Major Street** Hours Ending: 17:00 18:00 19:00 0:00 0:00 0:00 0:00 0:00 Crossing Values:



Oount	Date.	16-Dec-	e-16 Site #: 1620000001				l						1							
		Passeng							cks - Nort						avys - No	orth Appr			Pedes	
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Ri	ght	Le	eft	Th	ru	Ri	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	123	123	0	0	0	0	0	0	0	0	0	0	3	3	0	0	0	0
17:30:00	2	2	255	132	0	0	0	0	0	0	0	0	0	0	6	3	0	0	0	0
17:45:00	3	1	381	126	0	0	0	0	1	1	0	0	0	0	8	2	0	0	0	0
18:00:00	6	3	468	87	0	0	0	0	1	0	0	0	0	0	9	1	0	0	0	0
18:15:00 18:30:00	10 10	4	577 688	109	0	0	0	0	1	0 1	0	0	0	0	12	3	0	0	0	0
18:45:00	10	0	785	111 97	0	0	0	0	2	0	0	0	0	0	13 15	2	0	0	0	0
19:00:00	12	2	909	124	0	0	0	0	2	0	0	0	0	0	16	1	0	0	0	0
19:15:00	12	0	909	0	0	0	0	0	2	0	0	0	0	0	16	0	0	0	0	0
19:15:15	12	0	909	0	0	0	0	0	2	0	0	0	0	0	16	0	0	0	0	0



Count	Date:	Passenger Cars - East Approach					ı						1						I	
		Passen	ger Cars	- East Ap	proach			Tru	cks - Eas	t Approa	ch			Н	eavys - Ea	ast Appro	pach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Ri	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	2	2	0	0	4	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	5	3	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	3	3
17:45:00	5	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0
18:00:00	5	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0
18:15:00	5	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0
18:30:00 18:45:00	6	11	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0
19:00:00	7 8	1 1	0	0	5 6	1	0	0	0	0	0	0	0	0	0	0	0	0	3	0
19:00:00	8	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0
19:15:00	8	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0



Count	I	16-Dec			162000	UUU I	I						1						Ι	
			er Cars -						ks - Sout						avys - So				Pedes	
Interval Time	Le		Th			ght	Le		Th		Rig		Le		Th			ght	South	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	156	156	1	1	0	0	1	1	0	0	0	0	2	2	0	0	0	0
17:30:00	0	0	288	132	1	0	0	0	2	1	0	0	0	0	4	2	0	0	0	0
17:45:00	0	0	418	130	2	1	0	0	3	1	0	0	0	0	6	2	0	0	0	0
18:00:00	0	0	546	128	2	0	0	0	3	0	0	0	0	0	8	2	0	0	0	0
18:15:00	0	0	669	123	6	4	0	0	4	1	0	0	0	0	10	2	0	0	0	0
18:30:00 18:45:00	0	0	798 905	129 107	8 10	2	0	0	4	0	0	0	0	0	11 14	1	0	0	0	0
19:00:00	0	0		114	11	1	0	0	4	0	0	0	0	0	16	2	0	0	0	0
19:00:00	0	0	1019 1019	0	11	0	0	0	4	0	0	0	0	0	16	0	0	0	0	0
19:15:00	0	0	1019	0	11	0	0	0	4	0	0	0	0	0	16	0	0	0	0	0



Count	Date:	Pate: 16-Dec-16 Site #: 1620000001 Passenger Cars - West Approach				0001														
		Passen	ger Cars	- West A _l	oproach			Tru	cks - Wes	t Approa	ch			Не	eavys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:15:00 19:15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



Accu-Traffic Inc. **Specified Period Afternoon Peak Diagram One Hour Peak** From: 17:00:00 From: 18:00:00 To: 19:00:00 To: 19:00:00 Weather conditions: Municipality: Mississauga Site #: 1620000002 Intersection: Barbertown Road & 5155 Mississa Person counted: TFR File #: Person prepared: 16-Dec-16 Count date: Person checked: ** Non-Signalized Intersection ** Major Road: Barbertown Road runs W/E North Leg Total: 17 Heavys 0 0 0 Heavys 0 East Leg Total: 69 0 North Entering: 0 Trucks 0 0 Trucks 0 East Entering: North Peds: Cars 0 0 0 Cars 17 East Peds: X Totals 17 Peds Cross: Totals 0 0 Peds Cross: 5155 Mississauga Rd South driveway Totals Trucks Heavys Totals Heavys Trucks Cars Cars 0 0 30 30 0 0 0 30 Barbertown Road 30 0 Heavys Trucks Cars Totals Barbertown Road 0 17 17 2 0 37 39 Trucks Heavys Totals Cars 54 2 37 0 39 X Peds Cross: West Peds: West Entering: 56 West Leg Total: 86 **Comments**



Total Count Diagram

Municipality: Mississauga

Site #: 1620000002

Intersection: Barbertown Road & 5155 Mississa

TFR File #: 1

Count date: 16-Dec-16

Weather conditions:

Person counted: Person prepared: Person checked:

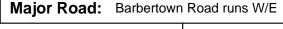
** Non-Signalized Intersection **

 North Leg Total: 34
 Heavys 0 0
 0

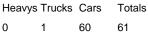
 North Entering: 8
 Trucks 0 0
 0

 North Peds: 0
 Cars 8 0
 0

 Peds Cross: ►
 Totals 8 0
 0



Heavys 0
Trucks 0
Cars 26
Totals 27



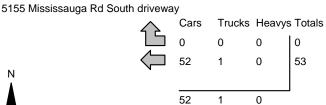




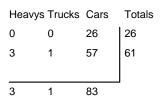
0

0

8



Barbertown Road









Barbertown Road

Cars Trucks Heavys Totals
57 1 3 61

Peds Cross: X
West Peds: 0
West Entering: 87
West Leg Total: 148

Comments



Accu-Traffic Inc. Traffic Count Summary

Municipality: Mississauga Intersection: Barbertown Road & 5155 Mississa Count Date: 16-Dec-16 **North Approach Totals South Approach Totals** North/South Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Total Hour Hour Total Total Grand Grand **Ending** Peds **Ending** Peds Approaches Thru Right Thru Right Total Left Total 17:00:00 17:00:00 0 0 0 0 0 0 0 0 0 0 0 18:00:00 0 8 0 8 18:00:00 0 0 0 0 8 0 0 19:00:00 0 0 19:00:00 0 0 0 Totals: 0 8 S Totals: 0 0 **East Approach Totals West Approach Totals** East/West Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Hour Total Hour Total Total Grand **Ending** Peds **Ending** Peds Approaches Right Right Thru Left Thru Total Left Total 17:00:00 0 17:00:00 0 0 0 0 0 0 0 0 18:00:00 18:00:00 0 23 0 23 0 54 22 0 31 9 0 19:00:00 0 30 30 0 86 19:00:00 17 39 56 0 Totals: 0 53 140 W Totals: 87 0 **Calculated Values for Traffic Crossing Major Street** 17:00 18:00 Hours Ending: 19:00 0:00 0:00 0:00 0:00 0:00 Crossing Values:



Count	Date:	ate: 16-Dec-16 Site #: 1620000002 Passenger Cars - North Approach											1							
		Passeng			1				cks - Nort						avys - No				Pedes	
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Ri	ght	Le	eft	Th	ru	Ri	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	7	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	8	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00 18:30:00	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:45:00	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:00:00	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:15:00	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:15:15	0	0	0	0	8	0	0	0	Ö	0	0	0	0	0	0	0	0	0	0	0



		Passenger Cars - East Approach						Tru	cks - Eas	t Approa	ch			He	eavys - Ea	ast Appro	oach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	Le	ft	Th	ru	Rig	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	4	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	10	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	14	4	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	22	8	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
18:15:00	0	0	26	4	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
18:30:00	0	0	32	6	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
18:45:00	0	0	40	8	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
19:00:00	0	0	52	12	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
19:15:00	0	0	52	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
19:15:15	0	0	52	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0



	Passenger Cars - South Approach						Truc	ks - Sout	h Approa	ach			He	avys - So	uth Appr	oach		Pedes	trians	
Interval	Le	eft	Th		Riç	jht	Le		Th			ght	Le		Th		Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00 18:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



		Passen	ger Cars	- West Ap	proach			Tru	cks - Wes	t Approa	ch			Не	avys - W	est Appr	oach		Pedestrians		
Interval	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	West	Cross	
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17:15:00	2	2	3	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17:30:00	3	1	10	7	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17:45:00	6	3	16	6	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	
18:00:00	9	3	20	4	0	0	0	0	1	0	0	0	0	0	1	1	0	0	0	0	
18:15:00	14	5	28	8	0	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	
18:30:00 18:45:00	20 22	6	32 43	<u>4</u> 11	0	0	0	0	1	0	0	0	0	0	2	<u>1</u> 1	0	0	0	0	
19:00:00	26	4	57	14	0	0	0	0	1	0	0	0	0	0	3	0	0	0	0	0	
19:00:00	26	0	57	0	0	0	0	0	1	0	0	0	0	0	3	0	0	0	0	0	
19:15:15	26	0	57	0	0	0	0	0	1	0	0	0	0	0	3	0	0	0	0	0	

APPENDIX B Existing Traffic Analysis

1. Micolobaaga rta	<u> </u>	oou, c	, Daib	0110111	1 1 KG							
	•	→	•	•	+	•	•	†	<i>></i>	/	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		¥	ĵ»		ň	f)	
Traffic Volume (veh/h)	7	0	10	16	0	12	3	446	13	13	633	3
Future Volume (Veh/h)	7	0	10	16	0	12	3	446	13	13	633	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	0	11	17	0	13	3	480	14	14	681	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1210	1210	682	1213	1205	487	684			494		
vC1, stage 1 conf vol	710	710		493	493							
vC2, stage 2 conf vol	499	500		720	712							
vCu, unblocked vol	1210	1210	682	1213	1205	487	684			494		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.3	4.1			4.2		
tC, 2 stage (s)	6.1	5.5		6.2	5.5							
tF (s)	3.5	4.0	3.3	3.6	4.0	3.4	2.2			2.3		
p0 queue free %	98	100	98	95	100	98	100			99		
cM capacity (veh/h)	352	366	453	340	368	569	919			1006		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	19	30	3	494	14	684						
Volume Left	8	17	3	0	14	0						
Volume Right	11	13	0	14	0	3						
cSH	404	412	919	1700	1006	1700						
Volume to Capacity	0.05	0.07	0.00	0.29	0.01	0.40						
Queue Length 95th (m)	1.2	1.9	0.1	0.0	0.3	0.0						
Control Delay (s)	14.3	14.4	8.9	0.0	8.6	0.0						
Lane LOS	В	В	Α		Α							
Approach Delay (s)	14.3	14.4	0.1		0.2							
Approach LOS	В	В										
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utiliza	ation		43.5%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									

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	•	4	†	<i>></i>	/	Ţ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>		ሻ	†
Traffic Volume (veh/h)	0	0	465	0	0	649
Future Volume (Veh/h)	0	0	465	0	0	649
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	0	500	0	0	698
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL			TWLTL
Median storage veh)			2			2
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1198	500			500	
vC1, stage 1 conf vol	500					
vC2, stage 2 conf vol	698					
vCu, unblocked vol	1198	500			500	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4	0.2				
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	420	575			1075	
			CD 1	CD 1	1070	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	0	500	0	698		
Volume Left	0	0	0	0		
Volume Right	0	0	0	0		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.00	0.29	0.00	0.41		
Queue Length 95th (m)	0.0	0.0	0.0	0.0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS	А					
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		37.5%	IC	U Level	of Service
Analysis Period (min)			15			
J						

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	٠	→	•	•	←	4	•	†	<i>></i>	\	↓	4
Movement	EBL	EBT	EBR	v WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	₽		ሻ	1>	
Traffic Volume (veh/h)	4	0	4	12	0	19	7	613	28	24	509	7
Future Volume (Veh/h)	4	0	4	12	0	19	7	613	28	24	509	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	4	0	4	13	0	20	8	659	30	26	547	8
Pedestrians	•		•									
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (m)								_			_	
pX, platoon unblocked												
vC, conflicting volume	1298	1308	551	1293	1297	674	555			689		
vC1, stage 1 conf vol	603	603		690	690							
vC2, stage 2 conf vol	695	705		603	607							
vCu, unblocked vol	1298	1308	551	1293	1297	674	555			689		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.3	4.1			4.2		
tC, 2 stage (s)	6.1	5.5		6.2	5.5							
tF (s)	3.5	4.0	3.3	3.6	4.0	3.4	2.2			2.3		
p0 queue free %	99	100	99	96	100	95	99			97		
cM capacity (veh/h)	317	337	538	328	348	444	1026			848		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	8	33	8	689	26	555						
Volume Left	4	13	8	0	26	0						
Volume Right	4	20	0	30	0	8						
cSH	399	390	1026	1700	848	1700						
Volume to Capacity	0.02	0.08	0.01	0.41	0.03	0.33						
Queue Length 95th (m)	0.5	2.2	0.2	0.0	8.0	0.0						
Control Delay (s)	14.2	15.1	8.5	0.0	9.4	0.0						
Lane LOS	В	С	А		Α							
Approach Delay (s)	14.2	15.1	0.1		0.4							
Approach LOS	В	С										
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utiliza	ation		44.0%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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	•	•	†	<i>></i>	/		
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		1>		*		
Traffic Volume (veh/h)	0	0	636	0	0	540	
Future Volume (Veh/h)	0	0	636	0	0	540	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	0	0	684	0	0	581	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			TWLTL			TWLTL	
Median storage veh)			2			2	
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	1265	684			684		
vC1, stage 1 conf vol	684						
vC2, stage 2 conf vol	581						
vCu, unblocked vol	1265	684			684		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4						
tF (s)	3.5	3.3			2.2		
p0 queue free %	100	100			100		
cM capacity (veh/h)	407	452			919		
Direction, Lane #	WB 1	NB 1	SB 1	SB 2			
Volume Total	0	684	0	581			
Volume Left	0	0	0	0			
Volume Right	0	0	0	0			
cSH	1700	1700	1700	1700			
Volume to Capacity	0.00	0.40	0.00	0.34			
Queue Length 95th (m)	0.0	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0	0.0			
Lane LOS	А						
Approach Delay (s)	0.0	0.0	0.0				
Approach LOS	А						
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utiliza	ation		36.8%	IC	U Level	of Service	9
Analysis Period (min)			15				

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APPENDIX C Background Developments Trip Generation Calculation

Land Use	Commercia	al/Retail Pla	aza- Shopp	ing Center		
Total	111.9	000 square	e feet			
Land Use	820	Weekday A	AM peak Ho	our		
Equation	Ln(T)=0.61	LN(x)+2.24	1			
In	62%			Α	vg. Rate	0.96
Out	38%					
Internal Captu	re 0%					
Pass-by	0%					
Source ITE	IN	OUT	TOTAL	Avg	Eqn	
Gross	104	63	167	107	167	
Gross Rate	0.92	0.57	1.49			
Internal	0	0	0			
Passby	0	0	0			
New	104	63	167			
New Rate	0.93	0.56	1.49			

To	/From	Propo	ortions		Trips	
'	#F10111	Inbound	Outbound	In	Out	Total
North	Mississauç	58%	41%	61	26	87
South	Mississaug	42%	59%	43	37	80
West						
East						
	Total	100%	100%	104	63	167

Check	ok	ok
Difference	0	0
	0	0
	0	0
	0	0

111900 ft2 1000

Existing Distribution

-Moung D	otribation	
	IN	
V	649	58%
5	462	42%
Γotal	1111	100%



	Rd Residential Deve	elopmen	t			
Land Use:	Residential					
Variable:		10	Units			
Land Use		210	AM Peak Ho	our	Avg.Rat	e 0.75
Eqn	T=0.70(x)+9.74				Pg	297
IN		25%			3	
Out		75%				
Non-Auto		0%				
Internal Capture		0%				
Pass-by		0%				
Visitor		0%				
	IN		Out	Total	Eqn	Avg Rate
Gross		4	13	17	1	17 8
Gross Rate		0.40	1.30	1.70		
Transit		0	0	0		
Internal Capture		0	0	0		
Pass-by		0	0	0		
Visitors		0	0	0		
New		4	13	17	1	
Rate		0.40	1.30	1.70		

	To/From	Propo	ortions		Trips		Ch	eck
	10/FIOIII	Inbound	Outbound	In	Out	Total	OK	OK
North	Mississauga Road	58%	41%	2	5	7		
South	Mississauga Road	42%	59%	2	8	10		
East				0	0	0		
West				0	0	0		
	Total	100%	100%	4	13	17		

Existing Distribution

	IN		OUT	
N	649	58%	465	41%
S	462	42%	659	59%
Total	1111	100%	1124	100%

Land Use:	Rd Residential Deve Residential	юринен				
Variable:	Residential	15	Units			
variable.		.0	Ornio			
Land Use		210	AM Peak Ho	our	Avg.Ra	te 0.75
Eqn	T=0.70(x)+9.74				Pg	297
IN		25%				
Out		75%				
Non-Auto		0%				
Internal Capture		0%				
Pass-by		0%				
Visitor		0%				
	IN		Out	Total	Eqn	Avg Rate
Gross		5	15	20		20 11
Gross Rate		0.33	1.00	1.33		
Transit		0	0	0		
Internal Capture		0	0	0		
Pass-by		0	0	0		
Visitors		0	0	0		
New		5	15	20		
Rate		0.33	1.00	1.33		

	To/From	Propo	Proportions		Trips			eck
	10/FIOIII	Inbound	Outbound	ln	Out	Total	OK	OK
North	Mississauga Road	58%	41%	3	6	9		
South	Mississauga Road	42%	59%	2	9	11		
East				0	0	0		
West				0	0	0		
	Total	100%	100%	5	15	20		

	IN		OUT	
N	649	58%	465	41%
S	462	42%	659	59%
Total	1111	100%	1124	100%

Land Use Commercial/Retail Plaza-Shopping Center
Total 111.9 000 square feet

Land Use 820 Weekday PM peak Hour

Equation Ln(T)=0.67LN(x)+3.31

In 48%
Out 52%
Internal Capture 0%
Pass-by 33%

Source ITE	IN	OUT	TOTAL
Gross	199	216	415
Gross Rate	1.78	1.93	3.71
Internal	0	0	0
Passby	69	69	138
New	130	147	277
New Rate	1.16	1.32	2.48

Avg	Eqn	
415		646

Avg. Rate

3.71

То	/From	Proportions		Trips			
10	7710111	Inbound	Outbound	In	Out	Total	
North	Mississauq	45%	55%	59	81	140	
South	Mississau	55%	45%	71	66	137	
West							
East							
	Total	100%	100%	130	147	277	

Check	ok	ok
Difference	0	0
	0	0
	0	0
	0	0

Existing Distribution						
	IN		OUT			
N	540	45%	636	55%		
S	648	55%	525	45%		
Total	1188	100%	1161	100%		

111900 ft2 1000



Land Use:	Rd Residential Developmer Residential				
Variable:		Units			
Land Use	210	PM Peak H	our	Avg.Ra	te 1
Eqn	$Ln(T)=0.90LN^*(x)+0.51$			Pg	298
IN	63%			J	
Out	37%				
Non-Auto	0%				
Internal Capture	0%				
Pass-by	0%				
Visitor	0%				
	IN	Out	Total	Eqn	Avg Rate
Gross	8	5	13	1	13 10
Gross Rate	0.80	0.50	1.30		
Transit	0	0	0	1	
Internal Capture	0	0	0	1	
Pass-by	0	0	0	1	
Visitors	0	0	0	1	
New	8	5	13	1	
Rate	0.80	0.50	1.30	1	

	To/From	Propo	ortions		Trips	Trips		eck
	TO/FIOIII	Inbound	Outbound	In	Out	Total	OK	OK
North	Mississauga Road	58%	41%	5	2	7		
South	Mississauga Dood	42%	59%	2	2	6		
South	Mississauga Road	42%	59%	3	3	0		
East				0	0	0		
West				0	0	0		
	Total	100%	100%	8	5	13		

	IN		OUT	
N	649	58%	465	41%
S	462	42%	659	59%
Total	1111	100%	1124	100%

5267 Mississauga Rd Residential Development						
Land Use:	Residential					
Variable:	15	Units				
Land Use	210	AM Peak Ho	our	Avg.Rate	e 1	
Eqn	Ln(T)=0.90*LN(x)+0.51			Pg	298	
IN .	63%			3		
Out	37%					
Non-Auto	0%					
Internal Capture	0%					
Pass-by	0%					
Visitor	0%					
	IN	Out	Total	Eqn	Avg Rate	
Gross	12	7	19	1	L9	
Gross Rate	0.80	0.47	1.27			
Transit	0	0	0			
Internal Capture	0	0	0			
Pass-by	0	0	0			
Visitors	0	0	0			
New	12	7	19			
Rate	0.80	0.47	1.27	1		

	To/From	Propo	Proportions		Trips	Check		
	10/FIOIII	Inbound	Outbound	In	Out	Total	OK	OK
North	Mississauga Road	58%	41%	7	3	10		
South	Mississauga Road	42%	59%	5	4	9		
East				0	0	0		
West				0	0	0		
	-	1000/	4000/		_	40		
	Total	100%	100%	12	7	19		

	IN		OUT	
N	649	58%	465	41%
S	462	42%	659	59%
Total	1111	100%	1124	100%

APPENDIX D Future Background Traffic Analysis

	•	→	•	•	←	•	4	†	/	>	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ň	f)		Ť	f)	
Traffic Volume (veh/h)	7	0	10	16	0	12	3	552	14	14	785	3
Future Volume (Veh/h)	7	0	10	16	0	12	3	552	14	14	785	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	0	11	17	0	13	3	594	15	15	844	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1488	1490	846	1492	1484	602	847			609		
vC1, stage 1 conf vol	876	876		608	608							
vC2, stage 2 conf vol	613	615		885	877							
vCu, unblocked vol	1488	1490	846	1492	1484	602	847			609		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.3	4.1			4.2		
tC, 2 stage (s)	6.1	5.5	0.2	6.2	5.5	0.0						
tF (s)	3.5	4.0	3.3	3.6	4.0	3.4	2.2			2.3		
p0 queue free %	97	100	97	94	100	97	100			98		
cM capacity (veh/h)	282	302	365	270	304	489	799			909		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	19	30	3	609	15	847						
Volume Left	8	17	3	0	15	0						
Volume Right	11	13	0	15	0	3						
cSH	325	335	799	1700	909	1700						
Volume to Capacity	0.06	0.09	0.00	0.36	0.02	0.50						
Queue Length 95th (m)	1.5	2.3	0.00	0.0	0.4	0.0						
0 , ,	16.8	16.8	9.5	0.0	9.0	0.0						
Control Delay (s) Lane LOS	10.0	C	7.5 A	0.0	7.0 A	0.0						
Approach Delay (s)	16.8	16.8	0.0		0.2							
Approach LOS	C	C	0.0		0.2							_
Intersection Summary												
Average Delay			0.6									
Intersection Capacity Utiliza	ition		51.5%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

	•	•	†	/	\	↓
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>		ሻ	†
Traffic Volume (veh/h)	0	0	571	0	0	802
Future Volume (Veh/h)	0	0	571	0	0	802
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	0	614	0	0	862
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL			TWLTL
Median storage veh)			2			2
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1476	614			614	
vC1, stage 1 conf vol	614					
vC2, stage 2 conf vol	862					
vCu, unblocked vol	1476	614			614	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	348	496			975	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	0	614	0	862		
Volume Left	0	0	0	0		
Volume Right	0	0	0	0		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.00	0.36	0.00	0.51		
Queue Length 95th (m)	0.0	0.0	0.0	0.0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS	A	0.0	0.0	3.0		
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	A	0.0	0.0			
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		45.5%	IC	امرما	of Service
Analysis Period (min)	LuliUII		15	10	O LCVCI	OI JOI VICE
Analysis Penou (IIIII)			10			

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T: Micolocaaga rta			4 = 00									
	۶	→	•	•	←	4	•	†	~	/	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	f)	
Traffic Volume (veh/h)	4	0	4	12	0	19	7	809	29	25	678	7
Future Volume (Veh/h)	4	0	4	12	0	19	7	809	29	25	678	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	4	0	4	13	0	20	8	870	31	27	729	8
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1693	1704	733	1688	1692	886	737			901		
vC1, stage 1 conf vol	787	787		902	902							
vC2, stage 2 conf vol	906	917		787	791							
vCu, unblocked vol	1693	1704	733	1688	1692	886	737			901		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.3	4.1			4.2		
tC, 2 stage (s)	6.1	5.5		6.2	5.5							
tF (s)	3.5	4.0	3.3	3.6	4.0	3.4	2.2			2.3		
p0 queue free %	98	100	99	95	100	94	99			96		
cM capacity (veh/h)	229	257	424	242	268	335	878			703		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	8	33	8	901	27	737						
Volume Left	4	13	8	0	27	0						
Volume Right	4	20	0	31	0	8						
cSH	297	291	878	1700	703	1700						
Volume to Capacity	0.03	0.11	0.01	0.53	0.04	0.43						
Queue Length 95th (m)	0.7	3.0	0.2	0.0	1.0	0.0						
Control Delay (s)	17.4	18.9	9.1	0.0	10.3	0.0						
Lane LOS	С	С	Α		В							
Approach Delay (s)	17.4	18.9	0.1		0.4							
Approach LOS	С	С										
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utiliza	ation		54.3%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									

Movement WBL WBR NBT NBR SBL SBT
Lane Configurations Y 1
Traffic Volume (veh/h) 0 0 832 0 0 710
Future Volume (Veh/h) 0 0 832 0 0 710
Sign Control Stop Free Free
Grade 0% 0% 0%
Peak Hour Factor 0.93 0.93 0.93 0.93 0.93 0.93
Hourly flow rate (vph) 0 0 895 0 0 763
Pedestrians
Lane Width (m)
Walking Speed (m/s)
Percent Blockage
Right turn flare (veh)
Median type TWLTL TWLTL
Median storage veh) 2 2
Upstream signal (m)
pX, platoon unblocked
vC, conflicting volume 1658 895 895
vC1, stage 1 conf vol 895
vC2, stage 2 conf vol 763
vCu, unblocked vol 1658 895 895
tC, single (s) 6.4 6.2 4.1
tC, 2 stage (s) 5.4
tF (s) 3.5 3.3 2.2
p0 queue free % 100 100 100
cM capacity (veh/h) 315 342 767
Direction, Lane # WB 1 NB 1 SB 2
Volume Total 0 895 0 763
Volume Left 0 0 0 0
Volume Right 0 0 0 0
cSH 1700 1700 1700 1700
Volume to Capacity 0.00 0.53 0.00 0.45
Queue Length 95th (m) 0.0 0.0 0.0 0.0
Control Delay (s) 0.0 0.0 0.0 0.0
Lane LOS A
Approach Delay (s) 0.0 0.0 0.0
Approach LOS A
Intersection Summary
Average Delay 0.0
Intersection Capacity Utilization 47.1% ICU Level of Service
Analysis Period (min) 15

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APPENDIX E Site Trip Generation Calculations

Land Use:	Residential-Common Eler	ments Townh	ouse (Bloc	k 1. 2.an	d 3)
Variable:		Units	(=	, _,	,
Land Use	230	AM Peak Ho	our	Avg.Rate	0.44
Eqn	Ln(T)=0.80Ln(X)+0.26			Pg	395
IN	17%				
Out	83%				
Non-Auto	0%				
Internal Capture	0%				
Pass-by	0%				
Visitor	0%				
				'n	
	IN	Out	Total	Eqn	Avg Rate
Gross	2	12	14		14 9
Gross Rate	0.10	0.60	0.70		
Transit	0	0	0		
Internal Capture	0	0	0		
Pass-by	0	0	0		
Visitors	0	0	0		
New	2	12	14		
Rate	0.10		0.70		

	To/From	Propo	ortions		Trips		Ch	eck
			Outbound	In	Out	Total	OK	OK
North	Mississauga Road	58%	41%	1	5	6		
South	Mississauga Road	42%	59%	1	7	8		
East				0	0	0		
West				0	0	0		
	Total	100%	100%	2	12	14		

	IN		OUT	
N	649	58%	465	41%
S	462	42%	659	59%
Total	1111	100%	1124	100%

Mississauga Rd					
Land Use:	Residential-	Free Hold	Fownhouse	(Block 4 and	5)
Variable:	4	Units			
Land Use	210	AM Peak H	our	Avg.Rate	0.75
Egn	T=0.70(x)+9	9.74		Pg	395
IN.	25%			•	
Out	75%				
Non-Auto	0%				
Internal Capture	0%				
Pass-by	0%				
Visitor	0%				
	IN	Out	Total	Eqn A	vg Rate
Gross	3	10	13	13	3
Gross Rate	0.75	2.50	3.25		
Transit	0	0	0		
Internal Capture	0	0	0		
Pass-by	0	0	0		
Visitors	0	0	0	1	
New	3	10	13	1	
Rate	0.75	2.50	3.25	1	

To/From			ortions		Trips	Check		
	10/From	Inbound	Outbound	In	Out	Total	OK	0
North	Mississauga Road	58%	41%	2	4	6		
South	Mississauga Road	42%	59%	1	6	7		
East	_		-	0	0	0		
West				0	0	0		
	Total	100%	100%	3	10	13		

Mississauga Rd					
Land Use:	Residential-Common Elem	ents Townh	ouse (Block '	, 2,and 3)
Variable:	20	Units			
Land Use	230	PM Peak Ho	our A	vg.Rate	0.52
Eqn	Ln(T)=0.82Ln(X)+0.32		Р	a	396
IN	67%			5	
Out	33%				
Non-Auto	0%				
Internal Capture	0%				
Pass-by	0%				
Visitor	0%				
	l IN	Out	Total E	qn	Avg Rate
Gross	11	5	16	16	•
Gross Rate	0.55	0.25	0.80		
Transit	0.00	0.20	0.00		
Internal Capture	0	0	0		
Pass-by	0	0	0		
Visitors	1 0	0	0		
New	11	5	16		
Rate	0.55	0.25	0.80		

	To/From	Prope	ortions		Trips		Ch	eck
	10/110111	Inbound	Outbound	In	Out	Total	OK	OK
North	Mississauga Road	45%	55%	5	3	8		
South	Mississauga Road	55%	45%	6	2	8		
East				0	0	0		
West				0	0	0		
	Total	100%	100%	11	5	16		

	IN		OUT	
N	540	45%	636	55%
S	648	55%	525	45%
Total	1188	100%	1161	100%

Land Use:	Residential.	Free Hold T	ownhouse	(Block 4 and	15)
Variable:		Units	OWIIIIOUSE	(DIOCK 4 and	10)
variable.	4	Office			
Land Use	210	PM Peak H	our	Avg.Rate	1
Egn	Ln(T)=0.90l	_n(X)+0.51		Pg	298
IN.	63%	. ,		•	
Out	37%				
Non-Auto	0%				
Internal Capture	0%				
Pass-by	0%				
Visitor	0%				
				_,	
	IN	Out	Total	Eqn	Avg Rate
Gross	4	2	6	6	4
Gross Rate	1.00	0.50	1.50		
Transit	0	0	0	ī	
Internal Capture	0	0	0	ī	
Describer	0	0	0	i	
Pass-by		^	0	1	
Visitors	0	0			
	0 4	2	6		

	To/From	Propo	ortions		Trips		Ch	eck
	10/F10111	Inbound	Outbound	ln	Out	Total	OK	0
North	Mississauga Road	45%	55%	2	1	3		
South	Mississauga Road	55%	45%	2	1	3		
East				0	0	0		
West				0	0	0		
	Total	100%	100%	4	2	6		

APPENDIX F Future Total Traffic Analysis

	٠	→	•	•	←	•	1	†	<i>></i>	-	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44		ň	f)		¥	f)	
Traffic Volume (veh/h)	7	0	10	22	0	16	3	553	15	16	792	3
Future Volume (Veh/h)	7	0	10	22	0	16	3	553	15	16	792	3
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	0	11	24	0	17	3	595	16	17	852	3
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1506	1504	854	1506	1498	603	855			611		
vC1, stage 1 conf vol	888	888		609	609							
vC2, stage 2 conf vol	618	617		897	889							
vCu, unblocked vol	1506	1504	854	1506	1498	603	855			611		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.3	4.1			4.2		
tC, 2 stage (s)	6.1	5.5	0.2	6.2	5.5	0.0						
tF (s)	3.5	4.0	3.3	3.6	4.0	3.4	2.2			2.3		
p0 queue free %	97	100	97	91	100	97	100			98		
cM capacity (veh/h)	276	298	362	266	301	488	793			908		
							, , ,			700		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2						
Volume Total	19	41	3	611	17	855						
Volume Left	8	24	3	0	17	0						
Volume Right	11	17	0	16	0	3						
cSH	320	328	793	1700	908	1700						
Volume to Capacity	0.06	0.12	0.00	0.36	0.02	0.50						
Queue Length 95th (m)	1.5	3.4	0.1	0.0	0.5	0.0						
Control Delay (s)	17.0	17.5	9.6	0.0	9.0	0.0						
Lane LOS	С	С	А		Α							
Approach Delay (s)	17.0	17.5	0.0		0.2							
Approach LOS	С	С										
Intersection Summary												
Average Delay			8.0									
Intersection Capacity Utiliza	tion		51.9%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.		ሻ	†
Traffic Volume (veh/h)	7	5	575	1	1	804
Future Volume (Veh/h)	7	5	575	1	1	804
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	8	5	618	1	1	865
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL			TWLTL
Median storage veh)			2			2
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1486	618			619	
vC1, stage 1 conf vol	618					
vC2, stage 2 conf vol	867					
vCu, unblocked vol	1486	618			619	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	99			100	
cM capacity (veh/h)	346	493			971	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	13	619	<u> </u>	865		
Volume Left	8	019	1			
	8 5	1	0	0		
Volume Right	391					
cSH		1700	971	1700		
Volume to Capacity	0.03	0.36	0.00	0.51		
Queue Length 95th (m)	0.8	0.0	0.0	0.0		
Control Delay (s)	14.5	0.0	8.7	0.0		
Lane LOS	В	0.0	A			
Approach Delay (s)	14.5	0.0	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliz	ation		52.3%	IC	U Level	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	₽		7	₽	
Traffic Volume (veh/h)	4	0	4	13	0	20	7	815	31	27	680	7
Future Volume (Veh/h)	4	0	4	13	0	20	7	815	31	27	680	7
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	4	0	4	14	0	22	8	876	33	29	731	8
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			TWLTL	
Median storage veh)								2			2	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1707	1718	735	1702	1706	892	739			909		
vC1, stage 1 conf vol	793	793		908	908							
vC2, stage 2 conf vol	914	925		793	797							
vCu, unblocked vol	1707	1718	735	1702	1706	892	739			909		
tC, single (s)	7.1	6.5	6.2	7.2	6.5	6.3	4.1			4.2		
tC, 2 stage (s)	6.1	5.5	0.2	6.2	5.5	0.0						
tF (s)	3.5	4.0	3.3	3.6	4.0	3.4	2.2			2.3		
p0 queue free %	98	100	99	94	100	93	99			96		
cM capacity (veh/h)	224	254	423	239	266	332	876			698		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2	0,0			070		
Volume Total		36				739						
	8		8	909	29							
Volume Left	4	14	8	0	29	0						
Volume Right	4	22	07/	33	0	1700						
cSH	293	289	876	1700	698	1700						
Volume to Capacity	0.03	0.12	0.01	0.53	0.04	0.43						
Queue Length 95th (m)	0.7	3.4	0.2	0.0	1.0	0.0						
Control Delay (s)	17.6	19.2	9.1	0.0	10.4	0.0						
Lane LOS	C	C	A		В							
Approach Delay (s)	17.6	19.2	0.1		0.4							
Approach LOS	С	С										
Intersection Summary												
Average Delay			0.7									
Intersection Capacity Utiliza	ation		54.8%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									
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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)		ሻ	↑
Traffic Volume (veh/h)	2	3	833	6	5	712
Future Volume (Veh/h)	2	3	833	6	5	712
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	2	3	896	6	5	766
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL			TWLTL
Median storage veh)			2			2
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1675	899			902	
vC1, stage 1 conf vol	899					
vC2, stage 2 conf vol	776					
vCu, unblocked vol	1675	899			902	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	5.4					
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	99			99	
cM capacity (veh/h)	311	340			762	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	5	902	5	766		
Volume Left	2	902	5	0		
Volume Right	3	6	0	0		
cSH	328	1700	762	1700		
Volume to Capacity	0.02	0.53	0.01	0.45		
Queue Length 95th (m)	0.02	0.0	0.01	0.45		
Control Delay (s)	16.2	0.0	9.8	0.0		
Lane LOS	10.2 C	0.0	9.0 A	0.0		
Approach Delay (s)	16.2	0.0	0.1			
	10.2 C	0.0	0.1			
Approach LOS	C					
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliz	zation		54.2%	IC	U Level	of Service
Analysis Period (min)			15			

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APPENDIX G Corresponding from Region of Peel Regarding Garbage Collection

From: Martino, Alex [mailto:alex.martino@peelregion.ca]

Sent: September-25-17 11:36 AM **To:** Jim Levac < iml@gsai.ca>

Cc: Ahmad, Munir <<u>munir.ahmad@peelregion.ca</u>>; Stephanie Segreti <<u>Stephanie.Segreti@mississauga.ca</u>>; Evan Perlman <<u>Evanp@gsai.ca</u>> **Subject:** OZ 16/011 W11: 5155 Mississauga Road (Old Barber House Lands)

Good morning Jim,

The Region has reviewed the attached site plan and feels that, given the constraints of the site, the Region will allow exceptions to the backup distance requirement. We will provide curbside collection subject to the following conditions:

Unit 17-20

- Applicant to provide space for storage of waste carts for units 17-20 on respective units.
- On the day of collection, carts to be set out close to curb for collection
- Applicant is responsible for meeting City of Mississauga requirements, if any, for setting out waste carts on the sidewalk

Units 11-16

Region will provide curbside collection of waste. A common collection pad will not be allowed. Waste collection vehicle driver will backup for approximately 15m for automated collection from some units, and for the remaining units he will manoeuver the carts manually for collection.

If you have any questions or concerns let me know.

Thanks,

Alex Martino

Junior Planner
Region of Peel
Development Services – Planning
10 Peel Centre Drive
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E-mail Alex.Martino@peelregion.ca

