CITY PARK (LAKESHORE) HOMES INC.

TRAFFIC IMPACT AND PARKING STUDY

1381 Lakeshore Road East

Project No. 2019-0263







COLE ENGINEERING GROUP LTD.

AUGUST 2020

HEAD OFFICE

70 Valleywood Drive Markham, ON L3R 4T5 **T.** 905 940 6161 | 416 987 6161 **F.** 905 940 2064 www.coleengineering.ca



August 20, 2020 Reference No. 2019-0263

Christopher Zeppa City Park (Lakeshore) Homes Inc. 950 Nashville Road Kleinburg, ON LOJ 1C0

Dear Mr. Zeppa:

Re: Traffic Impact and Parking Study

1381 Lakeshore Road East

City of Mississauga

Cole Engineering Group Ltd. ("COLE") was retained by City Park (Lakeshore) Homes Inc. (the "Owner") to undertake a Traffic Impact and Parking Study (TIPS) in support of a Site Plan Application for a proposed mixed-use development located at 1381 Lakeshore Road East in the City of Mississauga (the "City") and Peel Region (the "Region").

The subject site is currently occupied by a commercial plaza that will be removed. The proposed development will consist of a mixed-use building comprised of 15-storey residential high-rise building with 8 storey residential podium and ground floor retail area.

This TIPS report details the existing and future traffic conditions and the anticipated impact on the surrounding road network as a result of the proposed development. The study has concluded that the proposed development will have minimal impact on the operation of study area intersections throughout the study horizon periods and no mitigation measures will be required as a result of the traffic generated from the site. The access safety analysis concluded that sufficient sightlines are provided at the access locations. Besides, the site provides adequate space for garbage and loading vehicles ingress/egress maneuvering.

Should you have any questions, please do not hesitate to contact the undersigned.

Yours truly,

COLE ENGINEERING GROUP LTD.

Dumitru Liubeznii, EIT

Traffic Analyst

Urban Development, Traffic

Rao Marthi, B.Eng., MCIP, RPP

Project Manager

Urban Development, Traffic





PREPARED BY:

COLE ENGINEERING GROUP LTD.

Dumitru Liubeznii, EIT

Traffic Analyst

Urban Development, Traffic

CHECKED BY:

COLE ENGINEERING GROUP LTD.

Rao Marthi, B.Eng., MCIP, RPP

Project Manager

Urban Development, Traffic

Issues and Revisions Registry

Identification	Date	Description of Issued and/or Revision				
Final Report	June 2020	For Client Review				
Final Report	August 2020	For Submission				



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Traffic Impact and Parking Study Mixed-Use Development, 1381 Lakeshore Road East City of Mississauga



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1 Introduction

Cole Engineering Group Ltd. ("COLE") was retained by City Park (Lakeshore) Homes Inc. (the "Owner") to undertake a Traffic Impact and Parking Study in support of Site Plan for a proposed mixed-use development (residential and neighborhood retail on the ground floor), located on the northeast quadrant of Dixie Road and Lakeshore Road East, in the City of Mississauga (the "City"), within the Region of Peel (the "Region"). The site location is shown in **Figure 1-1** following the report.

The purpose of this TIPS report is:

- Assess the existing operations of the following study area intersections as well as the existing site
 accesses during the weekday AM and PM peak hours using traffic volume counts collected in the
 year of 2020;
 - Lakeshore Road East and Dixie Road (Signalized);
 - Lakeshore Road East and Cherriebell Road (Unsignalized);
 - Dixie Road and St James Avenue (Unsignalized)
 - Dixie Road and Orchard Hill Road (Unsignalized);
 - Cherriebell Road and Proposed Site Access;
 - Dixie Road and Existing Site Access #1 (West);
 - Lakeshore and Existing Site Access #2 (South); and,
 - Cherriebell Road and Existing Site Access #3 (East).
- Assess the future background traffic operations for a five-year (2025) horizon period incorporating traffic growth and traffic generated by adjacent developments;
- Estimate site traffic based on information published in the *Trip Generation Manual, 10th Edition,* by the Institute of Transportation Engineers (ITE);
- Assess the future total traffic operations (including the anticipated site traffic) at the key study area intersections and accesses for the analysis period;
- Recommend improvements, if applicable, should capacity constraints be observed in the existing and future horizons;
- Review the functionality of the internal vehicular circulation to facilitate fire route/emergency services, as well as garbage collection activities related to the proposed development;
- Review the proposed access sightlines;
- Undertake a review of the proposed parking supply and loading spaces in comparison to the City's Zoning By-Law, and,
- Prepare Transportation Demand Management (TDM) Plan to discourage single-occupant vehicles and encourage the transit and active transportation modes.



1.1 Existing Site and Proposed Development

Currently, there are four commercial units and two restaurants located at 1381 Lakeshore Road. Based on the latest Site Plan along with site statistics provided by Graziani + Corazza Architects is illustrated in **Figure 1-2** following the report.

The proposed development will include:

- A total of 242 residential units where:
 - 165 one-bedroom units:
 - 69 two-bedroom units; and,
 - 8 three-bedroom units.
- Two retail units 127 m² and 145 m² GFA.
- A total of 324 parking spaces, out of which 318 spaces are in the underground parking, while 6
 parking spaces are located on the surface level.

The details of parking provided are as below:

- 279 parking spaces including one accessible parking space are provided for residents at P1,
 P2, and P3 underground parking levels;
- 36 parking spaces, including one accessible parking space are provided for visitors at P1 underground parking level; and,
- 9 parking spaces, where 6 spaces including one accessible parking space provided at ground floor and 3 spaces provided at P1 underground parking level, provided for patrons to commercial units.

The proposed development is planned to connect to Charriebell Road via full move site access located approximately 55 metres from the intersection of Lakeshore Road East and Charriebell Road, and to Dixie Road via right-in-right-out site access located approximately 65 meters from the intersection of Lakeshore Road East and Dixie Road measured center-to-center of the road.



2 Scope and Study Approach

2.1 Study Scope

The scope of the current study is based on the Region's TIS guidelines and City's Signal Timing, Operation, and Capacity Analysis guidelines for Synchro Analysis. The scope of the study is elaborated in **Table 2.1**.

Table 2.1 Summary of Study Scope

Study Area Analysis Future Horizon	The site is bounded by other residential developments to the north, Lakeshore Road East to the south, Dixie Road to the west, and other Cherriebell Road to the east. Year 2025 is included as a horizon year in the traffic analysis.					
Analyses Time Periods	Weekday AM and PM Peak Hours.					
Intersections Analyzed	 Under Existing and Future Background Conditions (3 Existing Intersections): Lakeshore Road East and Dixie Road (Signalized); Lakeshore Road East and Cherriebell Road (Unsignalized); Dixie Road and St James Avenue (Unsignalized) Dixie Road and Orchard Hill Road (Unsignalized); Cherriebell Road and Proposed Site Access Dixie Road and Existing Site Access #1 – West (retained); Lakeshore and Existing Site Access #2 – South (removed); and, Cherriebell Road and Existing Site Access #3 – East (retained). Additionally, under Future Total Conditions (1 Proposed Intersection): Dixie Road and Future RIRO Site Access #1 (West); and, Cherriebell Road and Future Full Move Site Access #2 (East). 					

2.2 Study Approach

The study's methodology and analysis reflect the City's Traffic Impact Study (TIS) Guidelines and Region's Guidelines for Using Synchro. The Terms of Reference (TOR) for this study and feedback from the City is provided in **Appendix A**. Traffic data (i.e. intersection turning movement counts (TMCs)) were collected in 2018 and 2019 for the weekday AM and PM peak periods at all study area existing intersections from COLE commissioned traffic surveys. The growth rate was applied to through movements to simulate existing (2020) traffic conditions. The existing traffic volumes are discussed in **Section 3.4**.

Intersection capacity and queuing analysis were completed using Synchro software (version 9.0) for existing conditions. Analysis and results are further discussed in **Section 3.5**.

Traffic growth rates were provided by the City staff and further discussed in Section 4.0.

The details of background development in the vicinity of the proposed development were collected from the City online database and discussed in **Section 4.1**. The proposed roadway, transit and active transportation improvements in the vicinity of the study area were collected from the City's website and further discussed in **Section 4.2**.



Based on City's TIS guidelines, a 5-year (2025) horizon was selected to analyze future traffic conditions. To estimate the future background traffic volumes in the vicinity of the subject site, annual growth rates were acquired from the City and applied to the study area intersections in addition to traffic generated from the background developments.

The intersection capacity and queuing analysis under the future background conditions are discussed in **Section 4.2** of the report.

The site generated traffic was derived using the information contained in the *Trip Generation Manual*, 10^{th} *Edition* published by the Institute of Transportation Engineers (ITE). Trip distribution for the proposed development was based on the 2016 Transportation Tomorrow Survey (TTS) data analysis. The site trip generation and distribution are discussed in **Section 5.0**.

The future total traffic is estimated by adding the site generated traffic to future background traffic. The traffic analysis under the future total traffic conditions. The traffic analysis under the future total traffic conditions is discussed in **Section 6.0**.

On-site circulation and review of the proposed access sightlines are discussed in **Section 7.0**.

Parking justification explores parking rates for the residents, visitors (residential and commercial), parking requirements using the City's Zoning By-law, proxy site data, and the site-specific By-laws for some of the latest approved high-density residential developments, located in south Mississauga. The details are discussed in **Section 8.0**.

A Transportation Demand Management (TDM) plan was prepared, which refers to a variety of strategies to reduce congestion, minimize the number of single-occupant vehicles, encourage non-auto modes of travel, and reduce vehicle dependency to create a sustainable transportation system. The TDM plan is presented in **Section 9.0**.

The findings of the TIS study for the proposed development are concluded in **Section 10.0**.

2.3 Input Parameters and Assumptions

Analysis of the intersection operations will be conducted using Synchro 9.

The signalized and unsignalized intersection analysis is representative of the following assumptions:

- Peak hour factors (PHFs) are calculated based on existing peak hour volumes;
- Heavy Vehicle percentage based on the existing traffic volumes;
- Saturation flow ratio will be based on the Synchro default value of 1,900 vphpl; and,
- Synchro results for the signalized intersections will be provided in the Highway Capacity Manual (HCM) format.

The unsignalized intersections operational analysis in this report will be completed using the *Synchro 9* software, which employs the 2000 Highway Capacity Manual (HCM) Methodology for the intersection analysis. All parameters for the unsignalized intersection analysis are based on *Synchro* default values. *Synchro* results for the unsignalized intersections will be provided in the Highway Capacity Manual (HCM) format.

Trip generation for the proposed development will be based on *Trip Generation Manual, 10th Edition* prepared by the Institute of Transportation Engineers (ITE) for High-Rise Residential with 1st Floor Commercial (land use code 232). Trip distribution and assignments are based on the latest 2016 Transportation Tomorrow Survey (TTS) data and the existing traffic patterns.



2.4 Collected Information

Information used in the preparation of this update includes:

- Weekday AM and PM peak hours turning movement counts conducted at study area intersections; and,
- Signal Timing Plans (STPs).

In addition to data collected above, COLE referred the following guidelines/reports:

- City's Development Standards, Policies, and Guidelines;
- City's Official Plan;
- City's Transportation Master Plan; and,
- Geometric Design Standards for Canadian Roads (TAC) Manual.

3 Existing Traffic Conditions

The overview of existing traffic conditions includes a review of the existing road network, existing transit routes, and the existing active transportation network. Also, it covers traffic operations of the intersection and mitigation measures required to improve congested intersections.

3.1 Existing Road Network

The existing road network, lane configuration, and traffic control for the study intersections are shown in **Figure 3-1** following the report.

The details are described as follows:

- Lakeshore Road East is a major arterial road under the jurisdiction of the City. The roadway has
 four lane undivided with a center two-way lane roadway with a posted speed limit of 50km/h.
 Development along the roadway consists of residential and commercial units with a sidewalk on
 the north side and urbanized cross-section within the vicinity of the subject site. There is no
 dedicated bike lane on both sides.
- **Dixie Road** also is known as Peel Regional Road 4 (and also, as of 2016, Veterans Memorial Roadway) has a posted speed limit of 50km/h. There are sidewalks available on either side of Dixie Road. Development along the roadway consists of only residential units.
- Cherriebell Road is a residential two-lane undivided roadway with an unposted speed limit of 50km/h. Development along the roadway consists of only residential units with no sidewalks or curbs along either side within the study area.
- Orchard Hill Road is a residential two-lane undivided roadway with an unposted speed limit of 50km/h. Development along the roadway consists of only residential units with no sidewalks or curbs along either side within the study area.



3.2 Existing Transit Routes

Transit services are provided by MiWay and Go Transit. Bus services that operate within the vicinity of the site are described below:

- MiWay Route 23-Lakeshore travels mostly in the east-west direction with headways of approximately 8-10 minutes during peak periods. This route can connect the residents to Long Branch GO Station to Clarkson GO Station. Route 23 Lakeshore connects passengers to Toronto via a looped Streetcar network. The Long Branch GO Station approximately 800 meters east of the subject site. It operates with an approximate 15 minutes headways during the weekday peak commuter times.
- MiWay Route 5-Dixie travels mostly in the north-south direction with headways of approximately 8-10 minutes during peak periods. This route can connect the residents to Long Branch GO Station to the South and it travels north up to Cardiff Boulevard.

Existing transit routes are provided in **Appendix B**.

3.3 Cycling Network

Currently, dedicated cycling facilities are provided along Dixie Road in north-south directions that connect to the Waterfront multi-use path (MUP) just south of the proposed development. The existing cycling facilities for the subject area are illustrated in **Figure 3-2** following the report.

The future plans for the active transportation network improvement are discussed in **Section 4.1** of the report.



3.4 Existing Traffic Counts

The traffic volumes are based on intersection turning movement counts (TMCs) collected in 2018 and 2019 at the study area intersections during weekday AM and PM peak hours. The TMC survey information is presented in **Table 3.1**. Traffic volume data and signal timing plan are provided in **Appendix C**. The collected traffic counts are shown in **Figure 3-3**.

Table 3.1 Intersection Turning Movement Count

Intersection	Count Date	Count Hours	Peak Hours
1: Dixie Road & Orchard	Thursday, November 28,	7:00 to 9:00	7:45 to 8:45
Hill Road	2019	16:00 to 18:00	16:45 to 17:45
2: Dixie Road & St James	Thursday, March 06, 2018	7:00 to 9:00	7:45 to 8:45
Ave	Thursday, March 00, 2018	16:00 to 18:00	16:30 to 17:30
3: Private Access/Dixie	Thursday, November 28,	7:00 to 9:00	7:30 to 8:30
Road & Lakeshore Road East	2019	16:00 to 18:00	16:45 to 17:45
4: Lakeshore Road East &	Thursday, November 28,	7:00 to 9:00	7:30 to 8:30
Cherriebell Road	2019	16:00 to 18:00	16:45 to 17:45
5: Dixie Road & Existing	Thursday, November 28,	7:00 to 9:00	7:45 to 8:45
Site Access #1	2019	16:00 to 18:00	16:45 to 17:45
6: Lakeshore Road East &	Thursday, November 28,	7:00 to 9:00	7:30 to 8:30
Existing Site Access #2	2019	16:00 to 18:00	16:45 to 17:45
7: Cherriebell Road &	Thursday, November 28,	7:00 to 9:00	8:00 to 9:00
Existing Site Access #3	2019	16:00 to 18:00	16:15 to 17:15

The existing (2020) traffic volumes have been estimated by applying a 1% growth rate to the through movements of all intersections. Intersection counts for Dixie Road through movements at St James Avenue were balanced with respect to adjacent intersection through movements along Dixie Road. The resulting volumes are illustrated in **Figure 3-4** following the report.

3.5 Existing Traffic Analysis

These peak hour volumes were analyzed using the *Synchro 9.0* software which employs the 2000 HCM Methodology for the intersection analysis. The results of the existing intersection operations are summarized in **Table 3.2** with the level of service (LOS) and the volume to capacity (v/c) ratios for overall and individual movements. As per the City's Traffic Impact Study Guidelines, v/c ratios for overall signalized intersection operations, through movements, or shared through / turning movements greater than 0.85, and v/c ratios for exclusive movements greater than 0.90 have been shown in bold. The intersection capacity analysis reports under the existing conditions are presented in **Appendix D**.



Table 3.2 Existing Weekday AM and PM Peak Hour Traffic Analysis Summary

	Avail	able	AM Peak Hour					PM Peak Hour				
Intersection Name	Key Movement	Storage Length (m)	Taper Length (m)	LOS (v/c)	Delay (s)	Queue Length 50 th PCTL (m)	Queue Length 95 th PCTL (m)	LOS (v/c)	Delay (s)	Queue Length 50 th PCTL (m)	Queue Length 95 th PCTL (m)	
	WBL + WBT	-	-	C (0.1)	15.2	-	-	B (0.1)	14.3	-	-	
1: Dixie Road & Orchard	NBT + NBR	-	-	- (0.46)	0	-	-	- (0.42)	0	-	-	
Hill Road	SBL	25	50	B (0.02)	10.2	-	3	A (0.03)	9.2	-	3	
	SBT	-	-	- (0.31)	0	-	-	- (0.37)	0	-	-	
	EBL + EBT	-	-	C (0.23)	17.4	-	-	C (0.15)	15.6	-	-	
2: Dixie Road & St James	NBL	TW	LTL	A (0.02)	10	-	-	A (0)	8.8	-	-	
Ave	NBT	-	-	- (0.46)	0	-	-	- (0.41)	0	-	-	
	SBT + SBR	-	-	- (0.32)	0	-	-	- (0.37)	0	-	-	
	Overall	-	-	C (1.03)	34.1		-	D (0.97)	43.7	-	-	
	EBL	60	TWLTL	F (1.07)	92.1	90	170	F (0.98)	88.1	55	112	
	EBT + EBR	-	-	A (0.39)	7.7	45	68	A (0.3)	9.2	36	46	
3: Private Access/Dixie	WBL	20	15	B (0)	15.2	-	1	-	-	-	-	
Road & Lakeshore Road	WBT+WBR	-	-	C (0.74)	26.6	112	154	D (0.98)	47.4	207	268	
East	NBL+ NBT+NBR	-	-	D (0)	36.9	-	3	C (0.01)	34.8	1	4	
	SBL	27	20	E (0.81)	59.8	49	78	E (0.85)	66.1	68	116	
	SBT + SBR	-	-	D (0.22)	38.9	-	39	D (0.3)	39.6	9	36	
	EBL	10	30	B (0.01)	11.3	-	-	C (0.03)	15.1	-	1	
4: Lakeshore Road East &	EBT	-	-	- (0.36)	0	-	-	- (0.29)	0	-	-	
Cherriebell Road	WBT + WBR	-	-	- (0.24)	0	-	-	- (0.34)	0	-	-	
	SBL + SBT	-	-	C (0.04)	19.9	-	-	D (0.05)	26.2	-	-	
	WBL + WBT	-	-	B (0)	13.9	-	-	B (0.01)	13.1	-	-	
5: Dixie Road & Existing	NBT + NBR	-	-	- (0.45)	0	-	-	- (0.38)	0	-	-	
Site Access #1	SBL	10	25	A (0)	9.4	-	-	A (0)	8.9	-	-	
	SBT	-	-	- (0.32)	0	-	-	- (0.34)	0	-	-	
	EBL	5	12	- (0)	0	-	-	- (0)	0	-	-	
6: Lakeshore Road East &	EBT	-	-	- (0.35)	0	-	-	- (0.29)	0	-	-	
Existing Site Access #2	WBT + WBR	-	-	- (0.23)	0	-	-	- (0.33)	0	-	-	
	SBL + SBT	-	-	C (0.01)	16.8	-	-	A (0)	0	-	-	
7.01 . 1 . 1 . 1	EBL + EBT	-	-	A (0)	8.8	-	-	A (0)	8.3	-	-	
7: Cherriebell Road &	NBL + NBT	-	-	A (0)	0.4	-	-	A (0)	0.8	-	-	
Existing Site Access #3	SBT + SBR	-	-	- (0.01)	0	-	-	- (0)	0	-	-	



The results of the analysis of the existing (2020) traffic conditions indicate that all intersections will operate at acceptable LOS with v/c ratios below capacity level with the exception of:

- 1. The overall v/c ratio for the intersection of Dixie Road & Lakeshore Road East in the morning peak hour will be over capacity; and,
- 2. The eastbound left-turn movement at the intersection of Dixie Road & Lakeshore Road East will operate over capacity during AM peak hour of traffic operation.

Soft mitigation measures were implemented to improve the operation of the congested movements at the intersection of Dixie Road & Lakeshore Road East.

3.5.1 Existing Traffic Analysis with Mitigation

Soft mitigation measures were implemented by adjusting the signal timing total splits to improve the operation of the congested movements at the intersection of Dixie Road & Lakeshore Road East. The resulting The results of the existing intersection operations are summarized in **Table 3.3** with the level of service (LOS) and the volume to capacity (v/c) ratios for overall and individual movements.

Table 3.3 Existing Weekday AM Peak Hour Traffic Analysis with Mitigation Summary

		Avail	able	AM Peak Hour				
Intersection Name	Key Movement	Storage Length (m)	Taper Length (m)	LOS (v/c)	Delay (s)	Queue Length 50 th PCTL (m)	Queue Length 95 th PCTL (m)	
	Overall	-	-	C (0.92)	32.1	-	-	
	EBL	60	TWLTL	D (0.91)	53.4	86	148	
2. Drivete Acces/Divie	EBT + EBR	-	-	A (0.39)	7.5	47	62	
3: Private Access/Dixie Road & Lakeshore	WBL	20	15	B (0)	19	-	-	
Road East	WBT + WBR	-	-	D (0.83)	35.3	138	187	
ROdu Edst	NBL + NBT + NBR	-	-	D (0)	38.3	-	-	
	SBL	27	20	E (0.83)	65.1	51	86	
	SBT + SBR	-	-	D (0.22)	40.4	-	-	

The intersection capacity analysis reports under the existing conditions with mitigation measures are presented in **Appendix D-1.** The results of the analysis of the existing conditions with mitigations indicate that intersection will operate at acceptable LOS with v/c ratios below capacity level.



4 Future Background Traffic

The future background traffic volumes consist of the following components:

- Background traffic growth from outside the study area; and,
- Traffic generated from other proposed developments in the vicinity of the study area.

Based on the information provided by the City, a 2% (westbound during morning peak hour) and 2% (eastbound during afternoon peak hour) growth rate per annum along Lakeshore Road East were applied. According to the conversation with City, the growth rate provided includes future traffic generated by the potential development of Lakeview Village located south-west of the proposed development as shown in **Figure 4-1** following the report. It was communicated to the City, that no growth rate is applied to Dixie Road and other local roads in the study area. The correspondence with the City is provided in **Appendix E**.

The following are the two active background development immediately east and west of the proposed site.

The first background development is a proposed mixed-use development at 1345 Lakeshore Road East. This development will have 8 and 12 storey buildings. Together these two buildings will have a total of 397 condominium units and approximately 310m² of commercial/retail space fronting onto Lakeshore Road East. A Traffic Impact Study for this development was prepared by the BA Group.

The second background development is a proposed mixed-use development at 1407 Lakeshore Road East. According to the information found on City's Development Applications Map, the development will have a total of 24 stacked townhouse units and approximately 139m² of commercial/retail space. No study was conducted for this site. Background trip generation was calculated and assigned to the network based on the proposed development trip distribution discussed in **Section 5.2**.

The background development generated trips are provided in **Appendix F.**



4.1 Future Planned Roadway Improvements

The road improvements are outlined in the *Lakeshore Connecting Communities Transportation Master Plan* prepared by HDR in partnership with the City in May 2019. According to the study, Lakeshore Road East will be widened from 26m to 45.5m right-of-way (ROW) in the vicinity of the proposed development, also referred to as Segment 7 in the TMP. The proposed widening is designed to accommodate Express Bus exclusive lanes in east-west directions at the center of the roadway. According to the Implementation and Phasing section of the TMP, the transit service improvements will take place in Phase 1 (2019-2025), multi-modal road work and further transit improvements will take place in Phase 2A (2025 to 2030) and Phase 2B (2031-2041), and protection for extension of TTC streetcar services will take place in Phase 3 (beyond 2041). Also, the future roadway improvements will accommodate exclusive bicycle lanes.

It must be noted that the physical widening of Lakeshore Road East ROW will take place beyond the proposed development study horizons and, therefore, not considered in the future traffic analysis.

For reference, specific pages were extracted from the study in support of the information provided above and are supplied in **Appendix G**.

4.2 Future (2025) Background Traffic Analysis

The future (2025) background traffic volumes are illustrated in **Figure 4-2** following the report. The future (2025) background traffic volumes were analyzed using the *Synchro 9.0* software which employs the 2000 HCM Methodology for the intersection analysis. The results are summarized in **Table 4.1**. As per the City's Traffic Impact Study Guidelines, v/c ratios for overall signalized intersection operations, through movements, or shared through / turning movements greater than 0.85, and v/c ratios for exclusive movements greater than 0.90 have been shown in bold. Detailed calculations are provided in **Appendix H**.



Table 4.1 Future (2025) Background Weekday AM and PM Peak Hour Analysis Summary

		Available AM Peak Hour						PM Peak Hour				
		Avail	abie		Aivi Pear							
Intersection Name	Key Movement	Storage Length (m)	Taper Length (m)	LOS (v/c)	Delay (s)	Queue Length 50 th PCTL (m)	Queue Length 95 th PCTL (m)	LOS (v/c)	Delay (s)	Queue Length 50 th PCTL (m)	Queue Length 95 th PCTL (m)	
	WBL + WBT	-	-	C (0.11)	16.3	-	3	B (0.1)	14.9	-	3	
1: Dixie Road &	NBT + NBR	-	-	- (0.5)	0	-	-	- (0.44)	0	-	-	
Orchard Hill Road	SBL	25	50	B (0.02)	10.5	-	1	A (0.03)	9.3	-	1	
	SBT	-	-	- (0.32)	0	-	-	- (0.41)	0	-	-	
	EBL + EBT	-	-	D (0.51)	27.5	-	22	C (0.4)	24.8	-	15	
2: Dixie Road & St	NBL	TW	LTL	B (0.08)	10.4	-	2	A (0.11)	9.5	-	3	
James Ave	NBT	-	-	- (0.46)	0	-	-	- (0.41)	0	-	-	
	SBT + SBR	-	-	- (0.33)	0	-	-	- (0.41)	0	-	-	
	Overall	-	-	D (0.95)	37.9	-	-	D (1.02)	46	-	-	
	EBL	60	TWLTL	E (0.95)	62.6	96	159	F (1.03)	102.6	63	121	
3: Private Access/Dixie	EBT + EBR	-	-	A (0.39)	7.8	50	62	A (0.34)	9.6	42	52	
Road & Lakeshore	WBL	20	15	B (0)	19.6	-	-	()		-	-	
Road East	WBT + WBR	-	-	D (0.93)	45.1	171	223	D (0.99)	49.5	211	272	
Noau Last	NBL + NBT + NBR	-	-	D (0)	38.1	-	-	C (0.01)	34.8	-	4	
	SBL	27	20	E (0.85)	67.5	54	3	E (0.88)	70.5	71	123	
	SBT + SBR	-	-	D (0.23)	40.3	-	-	D (0.33)	40.2	11	39	
	EBL	10	30	B (0.01)	12.2	-	-	C (0.05)	15.4	-	1	
4: Lakeshore Road East	EBT	-	-	- (0.36)	0	-	-	- (0.33)	0	-	-	
& Cherriebell Road	WBT + WBR	-	-	- (0.27)	0	-	-	- (0.35)	0	-	-	
	SBL + SBT	-	-	C (0.09)	22.4	-	2	D (0.08)	25.5	-	2	



The results of the analysis of the future (2025) background traffic conditions indicate that all intersections will operate at acceptable LOS with v/c ratios below capacity level with exception of:

- The overall v/c ratio for the intersection of Dixie Road & Lakeshore Road East in the PM peak hour will be over capacity; and,
- 2. The eastbound left-turn movement at the intersection of Dixie Road & Lakeshore Road East will operate over capacity during PM peak hour of traffic operation.

Soft mitigation measures were implemented to improve the operation of the congested movements at the intersection of Dixie Road & Lakeshore Road East.

4.2.1 Future (2025) Background Traffic Analysis with Mitigation

Soft mitigation measures were implemented by adjusting the signal timing total splits and lost time adjustment of (-1) second to improve the operation of the congested movements at the intersection of Dixie Road & Lakeshore Road East. The resulting The results of the existing intersection operations are summarized in **Table 4.2** with the level of service (LOS) and the volume to capacity (v/c) ratios for overall and individual movements.

Table 4.2 Future (2025) Background Weekday PM Peak Hour Analysis with Mitigation Summary

		Avail	able	AM Peak Hour					
Intersection Name	Key Movement	Storage Length (m)	Taper Length (m)	LOS (v/c)	Delay (s)	Queue Length 50 th PCTL (m)	Queue Length 95 th PCTL (m)		
	Overall	-	-	D (0.95)	40.1	-	-		
	EBL	60	TWLTL	E (0.94)	77.1	58	114		
2. Duivete Asses /Divis	EBT + EBR	-	-	A (0.33)	8.2	38	48		
3: Private Access/Dixie Road & Lakeshore	WBL	20	15	-	-	-	-		
Road East	WBT + WBR	-	-	D (0.96)	41.5	203	264		
ROBU EBSI	NBL + NBT + NBR	-	-	D (0.01)	36.4	-	-		
	SBL	27	20	E (0.91)	77.2	72	126		
	SBT + SBR	-	-	D (0.32)	41.5	126	28		

The intersection capacity analysis reports under the existing conditions with mitigation measures are presented in **Appendix H-1.** The results of the analysis of the existing conditions with mitigations indicate that intersection will operate at acceptable LOS with v/c ratios below capacity level.

5 Future Total Traffic

The future total traffic analysis covers sections with regards to site trip generation and assignment as well as a cumulative total traffic operation which consists of future background traffic and site-related traffic.

5.1 Site Generated Traffic

Trip generation was undertaken using information contained in the *Trip Generation Manual, 10th Edition* published by the Institute of Transportation Engineers (ITE). Vehicle trips during the weekday AM and PM peak hours for the *High-Rise Residential with 1st Floor Commercial* (ITE LUC 232) for the proposed residential land uses were used.



In order to recognize the multi-modal supportive environment near the Site, the information on primary modes of transportation was extracted from the 2016 TTS for traffic zones 3643, 3649, 3648, and 3642 the modes of transportation within the study area are provided in **Table 5.1** and **Table 5.2**.

Table 5.1 Non-Auto Modal Split Calculation – AM Peak Hour

Primary Travel Mode		TTS Zones									
Primary Travel Wode	3642	3643	3648	3649	sum	Percentage					
Transit excluding GO rail	131	125	138	95	489	5%					
Cycle	38	0	0	0	38	0%					
Auto driver	1496	1746	2185	2944	8371	77%					
GO rail only	49	138	164	143	494	5%					
Joint GO rail and local transit	102	16	19	75	212	2%					
Auto passenger	126	122	98	296	642	6%					
Taxi passenger	0	12	0	0	12	0%					
Walk	31	146	161	230	568	5%					
Total	1973	2305	2765	3783	10826	100%					
Non	-Auto (AM	Peak Hour)			·	12%					

Table 5.2 Non-Auto Modal Split Calculation – PM Peak Hour

Duine and Transal Manda		TTS Zones									
Primary Travel Mode	3642	3643	3648	3649	sum	Percentage					
Transit excluding GO rail	146	91	162	173	572	4%					
Cycle	1661	1807	2946	3916	10330	79%					
Auto driver	82	149	133	187	551	4%					
GO rail only	78	28	32	68	206	2%					
Joint GO rail and local transit	195	214	363	342	1114	8%					
Auto passenger	0	12	0	0	12	0%					
Taxi passenger	40	0	11	0	51	0%					
Walk	39	49	77	109	274	2%					
Total	2241	2350	3724	4795	13110	100%					
Non	-Auto (PM	Peak Hour)				8%					

The trip generation from the proposed development is summarized in **Table 5.3** below. It must be noted, that directional distribution is not available for the ITE LUC 232. Therefore, for the analysis purposes, the directional distribution of ITE LUC 222 (Multifamily Housing – High-Rise) was applied.

Table 5.3 Site Trip Generation

Land Use	Parameter	We	ekday A	M Peak Hour	Weekday PM Peak Hour			
Lanu Ose	Parameter	In	Out	Two-Way Trips	In	Out	Two-Way Trips	
	Gross Vehicular Trips	19	57	76	32	29	51	
High-Rise Residential with 1st Floor Commercial (ITE LUC 232)	Gross Trip Rates (trips/unit)	0.08	0.23	0.31	0.13	0.08	0.21	
	Non-auto split reduction	3	7	10	4	1	5	
	Net New Trips	16	50	66	28	18	46	
	Net Trip Rate (trips/unit)	0.07	0.21	0.28	0.13	0.07	0.20	



The proposed development is expected to generate 66 new vehicular two-way trips in the morning peak hour (16 trips in and 50 trips out), 46 new vehicular two-way trips in the afternoon peak hour (28 trips in and 18 trips out).

5.2 Site Trip Distribution

The trip distribution was based on existing turning movements in the vicinity of the site along with the 2016 TTS data analysis and presented in **Table 5.4**. Site trip distribution is shown in **Figure 5-1** following the report.

Table 5.4 Site Trip Distribution

	Direction (From / To)	Trip Distribution
North	Via Dixie Road	55%
West	Via North Service Road	15%
East	Via Sherway Drive	30%

5.3 Future (2025) Total Traffic Analysis

The future total traffic consists of future background traffic plus site-related traffic. The future total traffic volumes in the weekday AM and PM peak hours for the horizon year 2025 are illustrated in **Figure 5-2** following the report.

Intersection capacity analysis under the future (2025) total traffic conditions was completed using *Synchro 9* software which employs the 2000 HCM Methodology for the intersection analysis. The traffic analysis results for future (2025) total traffic are summarized in **Table 5.5**. As per the City's Traffic Impact Study Guidelines, v/c ratios for overall signalized intersection operations, through movements, or shared through / turning movements greater than 0.85, and v/c ratios for exclusive movements greater than 0.90 have been shown in bold. The intersection capacity analysis under the future (2025) total traffic conditions are presented in **Appendix I**.



Table 5.5 Future (2025) Total Weekday AM and PM Peak Hour Traffic Analysis

		Available			AM Peak Hour				PM Peak Hour			
						Queue	Queue			Queue	Queue	
Intersection Name	Key Movement	Storage	Taper			Length	Length			Length	Length	
intersection Name	Key Wovernent	Length	Length	LOS (v/c)	Delay (s)	50 th	95 th	LOS (v/c)	Delay (s)	50 th	95 th	
		(m)	(m)			PCTL	PCTL			PCTL	PCTL	
						(m)	(m)			(m)	(m)	
	WBL + WBT	-	-	C (0.11)	16.8	-	3	C (0.1)	15	-	3	
1: Dixie Road &	NBT + NBR	-	-	- (0.52)	0	-	-	- (0.45)	0	-	-	
Orchard Hill Road	SBL	25	50	B (0.02)	10.7	-	-	A (0.03)	9.3	-	-	
	SBT	-	-	- (0.32)	0	-	-	- (0.42)	0	-	-	
	EBL + EBT	-	-	D (0.53)	29	-	23	D (0.41)	25.3	-	15	
2: Dixie Road & St	NBL	TWI	LTL	B (0.08)	10.4	-	-	A (0.12)	9.6	9.6	-	
James Ave	NBT	-	-	- (0.48)	0	-	-	- (0.42)	0	-	-	
	SBT + SBR	-	-	- (0.34)	0	-	-	- (0.42)	0	-	-	
	Overall	-	-	D (0.95)	39.3	-	-	D (0.96)	41.8	-	-	
	EBL	60	TWLTL	E (0.95)	63.1	97	160	F (0.96)	81.2	60	117	
2. Dulyanta	EBT + EBR	-	-	A (0.39)	8	50	62	A (0.33)	8.2	38	48	
3: Private Access/Dixie Road &	WBL	20	15	B (0)	19.9	-	-	()		-	-	
Lakeshore Road East	WBT + WBR	-	-	D (0.95)	47.6	180	225	D (0.96)	41.8	204	265	
Lakeshore Road East	NBL + NBT + NBR	-	-	D (0)	37.9	-	-	D (0.01)	36.4	-	-	
	SBL	27	20	E (0.88)	71.1	57	100	F (0.96)	87.1	77	135	
	SBT + SBR	-	-	D (0.23)	40.1	-	-	D (0.32)	41.5	9	38	
A. I. alicada ana Dagad	EBL	10	30	B (0.03)	12.4	-	-	C (0.09)	16	-	3	
4: Lakeshore Road	EBT	-	-	- (0.36)	0	-	-	- (0.33)	0	-	-	
East & Cherriebell Road	WBT + WBR	-	-	- (0.27)	0	-	-	- (0.35)	0	-	-	
Noau	SBL + SBT	-	-	D (0.22)	27.1	-	7	D (0.15)	29.2	-	4	
5: Dixie Road &	WBR	-	-	B (0.07)	14.8	-	2	B (0.02)	13.1	-	-	
Proposed West Site	NBT + NBR	-	-	- (0.46)	0	-	-	- (0.4)	0	-	-	
Access	SBT	-	-	- (0.34)	0	-	-	- (0.36)	0	-	-	
7: Cherriebell Road	EBL + EBT	-	-	A (0.01)	8.4	-	-	A (0.02)	8.4	-	-	
& Proposed East Site	NBL + NBT	-	-	A (0.01)	2.4	-	-	A (0.02)	3.2	-	-	
Access	SBT + SBR			- (0.01)	0			- (0)	0		-	



Based on the analysis conducted for the future (2025) total traffic conditions in the AM and PM peak hours, all intersections are performing at a good level of service and v/c ratios. No road network improvements are recommended or required in these conditions in the vicinity of the subject site.

6 Site Access Operations and Safety Review

The assessment of operations and safety of the site accesses and drive aisles was reviewed using maneuvering diagrams and sightline analysis.

6.1 Loading Spaces Review

Region's *Waste Collection Design Manual* provides garbage and recycling receptacle requirements. As per the Region's requirements, a loading area of 18.0 m depth and 6.0 m width is proposed on the site plan that will serve both commercial and residential uses.

6.2 On-Site Circulation Review

To ensure heavy vehicles maneuver throughout the subject site, a typical garbage truck was used as per Regions Waste Collection Design Guidelines. Access routes were assessed using *AutoTURN 10.0* analysis depicting the swept path of vehicles in relation to the proposed driveway system utilizing the Transportation Association of Canada's (TAC) design vehicles.

The maneuvering diagrams showing the assessment for the garbage truck (Figure 6-1 and Figure 6-2) and passenger vehicle (Figure 6-3, Figure 6-4, and Figure 6-5) are provided following the report. As illustrated in the diagrams, adequate space is available for the trucks to circulate in the site access egress to/from the proposed site.

6.3 Sightline Analysis

Sightlines and available sight distances were reviewed based on the *Geometric Design Guide for Canadian Roads*, published by the *Transportation Association of Canada (TAC)*, and dated June 2017. *Chapter 9, Section 9.9* was reviewed to identify the appropriate cases applicable to the proposed site. *Case B*, which refers to intersections with stop control on the minor road was reviewed. *Table 9.9.4* along with *Table 9.9.6*, which covers stopping sight distance and turning distances for left and right turns, as well, as crossing maneuver was also reviewed.

The summary of intersection sight distance is illustrated in **Table 6.1**. Available sight distances provide the minimum distances required for a driver, at a stop, to navigate an intersection safely. The sight distances requiring consideration for this case are:

- 1. Stopping Sight Distance (SSD) the minimum sight distance criterion for vehicles approaching the intersection that allows the passengers to carry out the actions needed to negotiate the intersection safely;
- 2. Intersection Sight Distance (ISD) the longer sight distance that allows the intersection to operate smoothly under left turn, right turn, and crossing maneuvers without forcing a major-road vehicle to slow down to less than 70% of their initial speed; and,
- Decision Sight Distance (DSD) the criteria applied in critical locations in evaluating the suitability
 of the sightlines. An example of such is the limitation due to horizontal or vertical road curvature.
 Traffic control devices are required that provide advance warning of conditions to be
 encountered.



Table 6.1 Safe, Intersection and Decision Sight Distances Requirement

Posted Speed	Design Speed	Movement	SSD (m)	ISD (m)	DSD (m)	
50 km/h 70 km/h		Left from Stop		150	200	
	70 km/h	Right from Stop	105	130		
		Crossing Maneuver		130		

The SSD, ISD for the left-turning vehicle from the stop and DSD along Cherriebell Rd with a current posted speed limit of 50 km/h are 105 m, 150 m, and 200 m, respectively.

Considering the maximum speed of turning vehicle from Lakeshore Rd E to Cherriebell Rd as 15 km/h and the critical gap for the left-turning vehicle at site access as 6.5 seconds, 1.5 seconds for perception time and 1.0 second for reaction time, the distance required to safely stop the vehicle (SSD) is approximately 38m.

The available sight distance at the proposed site access was plotted in **Figure 6-6** following the report shows that the available sight distance for the vehicle approaching from the left and right of the proposed site access are 200 m and 46 m, respectively. The review concludes that the available sight distance from the current proposed site access location meets TAC requirements.

7 Parking Review

This section reviews the parking requirement for bicycle and vehicle users based on By-law standards, desktop research, and proxy site data.

7.1 Bicycle Parking

Currently, City's Zoning By-law 0225-2007 does not require the development to provide dedicated bicycle parking. However, the site plan and site statistics indicate a total of 46 bicycle parking spaces (16 spaces for residents and 30 spaces for visitors) allocated within development boundaries.

7.2 Vehicular Parking

The following sections under review contain parking rates for the residents, visitors (residential and commercial), parking requirements using the City's Zoning By-law, proxy site data, and the site-specific By-laws for some of the latest approved high-density residential developments, located in south Mississauga.

7.2.1 Review of City's Zoning By-law 0225-2007 Parking Requirement

While reviewing the parking requirements using the City's Zoning By-law, the parking rates of the condominium apartment and retail center were used. The City agreed using a parking rate of 0.15 space/unit for the residential visitors without supporting survey data.

Based on the revised site stats provided, the parking requirement as per the City Zoning By-law 0225-2007 is summarized in **Table 7.1**.



Table 7.1 Parking Requirement – City's Zoning By-law 0225-2007

Land Use (Unit/Size)	No. of Units/ GFA			Composite Parking Rate for Residents
1. Residents Parking:				
 One-Bedroom 	165	1.25 space/unit	207	
 Two-Bedroom 	69	1.4 spaces/unit	97	
Three-Bedroom	8	1.75 spaces/unit	14	1.31 spaces/unit
2. Visitors Parking	242	0.15 spaces/unit	36	
3. Commercial/Retail Parking	272 m²	4.3 spaces/100m ²	12	
		366		

In the case of mixed-use developments, the City's Zoning By-law allows a shared parking formula. Based on the proposed land uses, the shared parking formula provided in Table 3.1.2.3 in the City's Zoning By-law is applied and the results are summarized in **Table 7.2**.

Table 7.2 Total Mixed-Use Building Shared Parking Requirement – City's Zoning By-law 0225-2007

Type of Use	Spaces	Morning		Noon		Afternoon		Evening	
	Spaces	%	Spaces	%	Spaces	%	Spaces	%	Spaces
Residential	318	90%	286	65%	207	90%	286	100%	318
Visitor	36	20%	7	20%	7	60%	22	100%	36
Retail	12	80%	10	100%	12	100%	12	70%	8
Total			303		226		320		362

Applying the City's Zoning By-law, the composite parking rate for the residents is 1.31 spaces/unit. A total of 362 parking spaces (318 spaces for residents, 36 spaces for residential visitors, and 8 spaces for commercial visitors) will be required as per the City's Zoning By-law. This will result in a parking deficiency of 38 spaces.

7.2.2 Review of Proxy Site Parking Demand Data

The results of the parking utilization surveys conducted at two proxy sites conclude that the proxy site parking demand observed for the residents is 1.05 space/unit. Applying the City's Zoning By-law parking rate for commercial visitors and agreed parking rate for the residential visitors, the parking requirement is summarized in **Table 7.3**.



Table 7.3 Parking Requirement – Based on Proxy Sites Data

Land Use (Unit/Size)	No. of Units/GFA	Parking Rate – Proxy Site and Zoning By-law (Parking Space/Dwelling Unit)	Required Parking Spaces
1. Residents Parking:			
One-Bedroom	165		
Two-Bedroom	69	1.05	254
Three-Bedroom	8		
2. Visitors Parking	242	0.15	36
3. Commercial/Retail Parking	272 m ²	4.3/100m²	12
		Total	303

Applying the shared parking formula, the parking requirement is summarized in Table 7.4.

Table 7.4 Total Mixed-Use Building Shared Parking Requirement – Based on Proxy Sites Data

Type of Use	Spaces	Morning		Noon		Afternoon		Evening	
	Spaces	%	Spaces	%	Spaces	%	Spaces	%	Spaces
Residential	255	90%	230	65%	166	90%	230	100%	255
Visitor	37	20%	7	20%	7	60%	22	100%	36
Retail	12	80%	10	100%	12	100%	12	70%	8
Total			247		185		264		299

Using the proxy site parking demand rate of 1.05 spaces/unit and applying the City's Zoning By-law parking rate for commercial visitors and agreed parking rate for the residential visitors, the parking requirement will be 299 parking spaces (255 spaces for residents, 36 spaces for residential visitors, and 8 spaces for commercial visitors). This will result in a parking surplus of 25 spaces.

7.2.3 Parking Review of Approved High-Density Residential Developments

As part of the parking review, site-specific By-laws for a list of approved developments both densely stacked townhouses and high-density residential developments, located in the south Mississauga, are collated. Of the 11 developments collected, 6 developments are not considered in this review as these developments include densely stacked townhouses, which don't match with the current development.

Using the information collected, key comparable information, such as site statistics, accessibility to GO transit, and amended parking rates, are summarized in **Table 7.5**. The detail of missing information these developments are included in the comments column of the table shows the comparison summary of high-density residential developments with the proposed development parameters.



 Table 7.5
 Comparison Between High-Density Residential Developments and Proposed Development Parameters

#	Property Title	Site Statistics	Distance from Port Credit GO Station	Parking Rates	Composite Parking Rate for Residents	Comments
1	Benson Holding Apartments Lakeshore West Located at the northeast quadrant of Benson Avenue and Lakeshore Road West intersection	Dwelling units=325 Townhouses=16 Non-Residential (Office/Retail) = 775 m ²	1.800 Km.	Residents 1 or 2 Bedroom - 1.00 space/unit Visitor - 0.19 space/unit Non-Residential (Office/Retail) - 1.0 space/unit	1.00	The parking rate for 3-Bedroom is not available, therefore, 1 or 2 Bedroom parking rate is used to calculate the composite parking rate.
2	Edenshaw Apartment Ann Street Located at the northwest quadrant of Ann Street and Park Street East intersection	1-Bedroom=91 2-Bedroom=206 3-Bedroom=16 Live-Work=3 Total=316	0.25 Km.	Residents 1-Bedroom - 0.75 space/unit 2-Bedroom - 0.90 space/unit 3-Bedroom - 1.10 space/unit Visitor - 0.10 space/unit	0.92	Commercial land use is not available.
3	Edenshaw Apartment Park Street Located at the southeast quadrant of Stavebank Road and Park Street East intersection		0.5 Km.	Residents 1-Bedroom - 0.80 space/unit 2-Bedroom - 1.00 space/unit 3-Bedroom - 1.30 space/unit Visitor - 0.10 space/unit	1.03	Unit mix statistics is not available for comparison with the proposed development.
4	FRAM Apartments	Dwelling Units=69 Semi-Detached=2		Residents - 0.80 space/unit Visitor - 0.10 space/unit	0.80	Bedroom wise parking rates are not available and property location is also not available for comparison with the proposed development.
5	Plazacorp condo Located at 420 Lakeshore Road East	Condominium Units – 172 Units Commercial-Site statistics not available	1.9 Km.	Residents - 1.00 space/unit Visitor - 0.15 space/unit Commercial – 1.00 space/100m ²	1.00	Unit mix statistics and commercial area are not available for comparison with the proposed development.



Based on the comparison provided in the table above, Developments #1, #2, and #5 look similar to the current proposed development. Since Development #4 has a significantly lower number of dwelling units, it is not considered in the comparison.

Based on the available data, composite parking rates for the residents range from 0.92 spaces/unit to 1.03 spaces/unit.

By using the City's Zoning By-law parking rate for the commercial visitors, agreed parking rate for the residential visitors, parking rates for the residents from the approved site-specific By-laws, and applying the shared parking formula, the total parking spaces required for the proposed development will range from 271 to 297 parking spaces.

7.2.4 Comparison of Parking Requirement

The following **Table 7.6** summarized the parking requirements noted in **Sections 7.1**, **7.2**, and **7.3**.

Table 7.6 Comparison of Required and Provided Parking Spaces

Land Use	Zoning By-law 0225-	Proxy Site Parking	Site-Specific Parking		
	2007	Demand	Rates		
Residential + Commercial	366	299	271 to 298		

By using the site-specific parking rates, the total parking spaces required for the proposed development will range from 271 to 298 parking spaces, whereas, using proxy site parking demand, the parking requirement will result in 299 parking spaces.

7.2.5 Proposed Parking

Based on the latest site plan dated August 14, 2020, a parking rate of 1.15 space/unit for the residents can be justified based on the approved site-specific By-laws of the developments similar to the current proposed development and proximity to the Port Credit GO station.

Applying the City's Zoning By-law parking rates for the commercial visitors and agreed parking rate for the residential visitors, the parking requirement is summarized in **Table 7.7**.

Table 7.7 Proposed Parking Based on Individual Land Uses

Land Use (Unit/Size)	No. of Units/ GFA	Parking Rate – Zoning By-law (Parking Space/Dwelling Unit)	Required Parking Spaces
1. Residents Parking			
 One-Bedroom 	165		
 Two-Bedroom 	69	1.15 space/unit	279
Three-Bedroom	8		
2. Visitors Parking	242	0.15	36
3. Commercial/Retail Parking	272 m ²	4.3 space/100m ²	12
		Total	326

Applying the shared parking formula, the parking requirement is summarized in **Table 7.8**.



Table 7.8 Total Mixed-Use Development Shared Parking Requirement

Type of Use	Spaces	Mor	Morning		Noon		Afternoon		Evening	
	Spaces	%	Spaces	%	Spaces	%	Spaces	%	Spaces	
Residential	279	90%	251	65%	181	90%	251	100%	279	
Visitor	36	20%	7	20%	7	60%	22	100%	36	
Retail	12	80%	10	100%	12	100%	12	70%	9	
Total			268		200		285		324	

Using a parking rate of 1.15 space/unit for the residents and applying the City's Zoning By-law parking rate for commercial visitors and agreed parking rate for the residential visitors, the parking requirement will be 324 parking spaces (279 spaces for residents, 36 spaces for residential visitors, and 9 spaces for commercial visitors). Based on survey data and past approved applications, it is our opinion that the proposed parking provision is sufficient to support the needs of the development. Parking study supporting documentation is provided in **Appendix J.**

8 Transportation Demand Management Plan

Transportation Demand Management (TDM) refers to a variety of strategies to reduce congestion, minimize the number of single-occupant vehicles, encourage non-auto modes of travel, and reduce vehicle dependency to create a sustainable transportation system. In short, TDM works to change how, when, where, and why people travel.

The TDM measures have been prepared to address the following required transportation demand management objectives:

- Provision of facilities/operations to minimize single-occupant vehicle (SOV) access to the study area and encourage the use of alternate transportation modes, particularly transit; and,
- Identify the operational and financial roles and responsibilities of the landowner including program development, implementation, and ongoing management and operations of the travel demand management plan/program.

TDM education and promotion related strategies support mobility by making the public aware of the options available and actively encourages the use of alternative transportation. These marketing initiatives may take the form of general marketing to targeted outreach. TDM marketing and promotion approaches include:

- Area marketing campaigns Promoting transit services, carpooling or other TDM;
- Education Programs Campaigns to inform the public about the specifics of services; and,
- Employer-Based Outreach Partnerships with employers to support TDM.



8.1 TDM Programs

8.1.1 Area Marketing Campaigns

Area marketing campaigns are premised on the objective of behavioral change. The methods of persuasion include conveying messages of:

- Personal benefit;
- Social benefit, and,
- Adhering to the "social norm".

It is recognized that individuals are more likely to change their current behavior or continue that behavior if it is the social norm amongst their peers.

One of the first area marketing TDM campaigns within the GTA was established by Metropolitan Toronto in 1994. The Diamond Lane Campaign was implemented in coordination with the expansion of the High Occupancy Lane network. The campaign included billboards, transit vehicles, radio, and newspaper ads. Transportation Management Association (TMA)'s can help facilitate TDM directives by developing promotional materials, advocate for funding, conduct transportation fairs, and many other things.

8.1.2 Education Programs and Information Services

The effectiveness of transportation systems to provide mobility to the public is only as good as the awareness of the public to the services provided. Education programs and information services (such as call-lines and internet-based route planners) can supplement marketing campaigns, increasing the commuter's understanding of their mobility options.

Miway provides http://www.mississauga.ca/portal/miway a map-based navigator. Progressive trip planners including web-based trip data can add to the commuter's understanding of travel options by providing real-time information.

A Denver, Colorado TMA (Transportation Solutions) managed a program designed to improve ridership. Before-and-after surveys were to gauge the impact of the efforts. The results clearly demonstrated the direct impact of the project on improved perceptions of bus transportation in the area. The results showed an increase in awareness:

- 17% increase in those who "know which bus routes to take to work/school"; and,
- 19% increase in those who feel they have "the information I need to ride the bus".



8.1.3 Walking / Cycling

The City has a long-standing commitment to an off-road trail system, having built over 250km of pathways and trails in parks and green spaces across the City. Pathways in the neighborhood parks provide cycling and walking-friendly connections to schools, community centers, and libraries. Also, cycling safety programs were offered such as CAN-BIKE courses and community safety workshops. The Road Safety Handbook was published and includes a chapter on cycling safety.

As identified in the Regions of Peel's Active Transportation Plan Implementation Strategy 2014 Program Update, Hanlan Feedermain Water Project and the City to improve walking and cycling facilities along Dixie Road, as a component of the Preferred Plan, MTO will be accommodating the Peel Region's request to extend their multi-use trail along Dixie Road from south of Kendall Road southerly through the study limits to the newly relocate Dixie Outlet Mall south entrance. As a result, a sidewalk on the east side and multi-use pedestrian/cyclist trail on the west side.

Based on the COLE's previously submitted TIS reports, a parking rate of 0.08 bicycle parking space per unit was used for short-term bicycle parking and a rate of 0.7 parking space per unit was used for long-term parking space. The long-term bicycle parking for common-element is been assumed to be covered with the provided garages. For short-term bicycle parking, two parking spots are required. It is recommended that one on-site bike rack, which can park two bicycles, be provided for the detached dwellings and common element condominium detached dwellings.

8.1.4 Carpooling

The Smart Commute program has been established in the form of 10 transportation management associations (TMAs) across the GTHA. The Smart Commute TMAs are supported by Metrolinx to coordinate and implement TDM initiatives.

Smart Commute – the City is a TMA committed to reducing traffic congestion, improving air quality, and advocating for sustainable transportation within the study area. Since inception in 2001, Smart Commute has collectively reduced approximately 1.2 million single-occupant vehicles (SOV) trips, which avoided more than 61 million vehicle kilometers traveled (VKT) and prevented the emission of over 14,000 tonnes of greenhouse gases (GHG) and 88 tonnes of smog-causing air pollutants.

Smart Commute works to:

- Implement employee trip reduction programs at local workplaces;
- Decrease traffic congestion, and improve air quality and health by reducing vehicle emissions;
- Improve employee productivity and morale, and reduce employee turnover;
- Advocate for improved transit service, and increased local transportation infrastructure;
- Bus-only and cycling lanes, and a wider network of subway and light rapid transit;
- Promote the benefits of transit-supportive development and smart-growth strategies;
- Encourage legislative flexibility in support of high-value, cost-effective transportation strategies such as vanpools, telework, transit subsidies, and shuttle services; and,
- Increase opportunities for TMA collaboration with business and government.



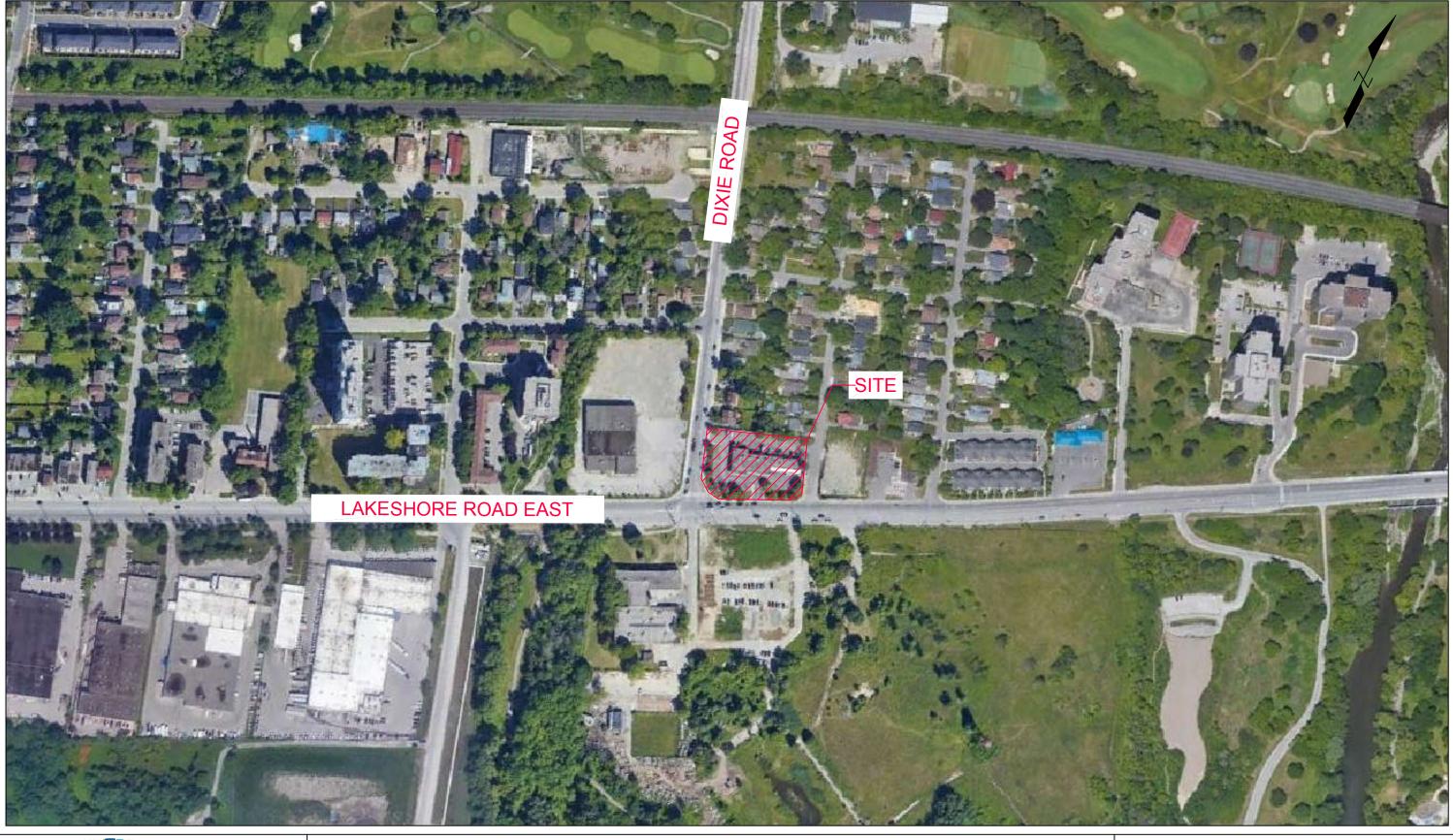
Smart Commute offers one-on-one support for staff to use sustainable modes of transportation and provides promotions and incentives on an on-going basis. A web-based car matching tool is provided to allow carpoolers to easily identify ride-matching options among a large membership base and form carpooling arrangements.

9 Conclusions

The findings and conclusions of our analysis are represented as follows:

- The study area intersection of Dixie Road & Lakeshore Road East is operating over-capacity in the
 existing traffic conditions during both the AM peak hour. Soft mitigation measures were
 implemented by adjusting the signal timing total splits are sufficient to improve the operation of
 the congested movements.
- The proposed development is expected to generate 66 new vehicular two-way trips in the morning peak hour (16 trips in and 50 trips out), 46 new vehicular two-way trips in the afternoon peak hour (28 trips in and 18 trips out).
- The review of access operations and safety concludes that design vehicles can safely maneuver within the site and there is sufficient sightlines at Cherriebell Road full moves access.
- The proposed accesses from Dixie Road and Cherriebell Road will function from the traffic circulation perspective and represents a suitable location for entry/exit means.
- The proposed development is expected to have a negligible impact on the surrounding road network as future total traffic is expected to operate similarly to the future background scenario (below-capacity and with an acceptable level of service) in 2025 horizon year;
- The site plan and site statistics indicate a total provision of 46 bicycle parking spaces (16 spaces for residents and 30 spaces for visitors) is sufficient for development needs.
- The parking justification concludes that the proposed total parking supply of 324 parking spaces (279 spaces for residents, 36 spaces for residential visitors, and 9 spaces for commercial visitors) is sufficient for development needs.

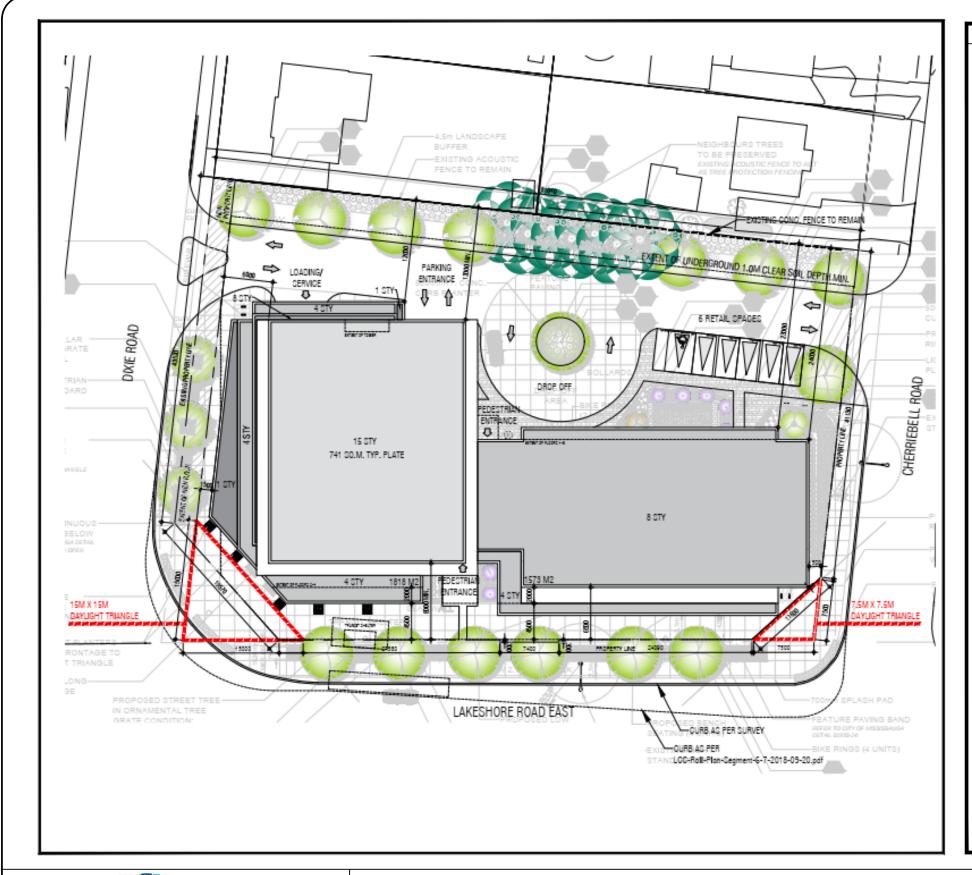
Based on the traffic analysis presented in this report, it is concluded that the existing road network has sufficient capacity to accommodate both the roadway growth and new traffic generated from the proposed development throughout the study horizon periods and no physical improvements to the study intersections is required.





SITE LOCATION PROPOSED MIXED-USE DEVELOPMENT 1381 LAKESHORE ROAD CITY OF MISSISSAUGA

DATE:	JUNE 2020	PROJECT No.:	2019-0263
SCALE:	N.T.S.	FIGURE No.:	1-1



PROJECT STATISTICS JOB No: 1629.18 - 1381 Lakesi	nore Road Eas	t			
			/IDED		PERMITTED/REQUIRED
01. SITE AREA	(m2)	(ha)	(ft2)	(ac)	FERIVITI ED/REQUIRED
GROSS SITE AREA	4204.76	0.43	45259.62	1.07	
NET SITE AREA	4084.47	0.41	43964.83	1.01	
LAND AREA DEDICATION	120.29	0.01	1294.79	0.03	
02.COVERAGE*	(m2)	(ft2)	PERCEN	ITAGE%	
LOT COVERAGE	1909.1	17619.00	47	%	
LANDSCAPE AREA	1271.49	10312.00	31	%	40%
03. G.F.A. (m2)					
RESIDENTIAL		173	17.20		
COMMERCIAL			72		
TOTAL		1/5	89.20		
04. F.S.I					
GROSS RESIDENTIAL		4.	31		
05. SETBACKS ** (m)					
BELOW GRADE	NORTH	4	.50		
	EAST	0.	.00		
	SOUTH		.00		
TOWER SETBACKS	WEST NORTH		.00 D min.		
TOTAL TOLIDATION	EAST		0 min.		
	SOUTH		min.		
	WEST	6.	.00		
06. UNIT BREAKDOWN***					
RESIDENTIAL UNITS		udio	0	0%	
		droom droom	165 69	68% 29%	
		droom	8	3%	
TOTAL			242	100%	
07. BARRIER FREE UNIT BREAK	DOWN				
RESIDENTIAL UNITS		udio	0		15% min
		droom	25		15% min
		droom droom	10 1		15% min
TOTAL	3 De	aroon	36		15% min
08. PARKING RESIDENTIAL	CE.		0		
NEGIDENTIAL	GF P1	4.40	64		
	P2	1.18	106	(1 B.F.)	
MOITOD	P3	0.45	109	(1.0.5)	
VISITOR RETAIL	P1 GF	0.15	36 6	(1 B.F.) (1 B.F.)	
	P1	4.3/100M2	3		
TOTAL			31	8	
09. BICYCLE PARKING					
RESIDENTIAL			16		
VISITOR			30		
10. BUILDING HEIGHT (STY)					
PODIUM		8 STY	(27.0M)		
TOWER			(53.4M)		
11 AMENITY (
11.AMENITY (m2)					
INDOOR		4	92		5.6M2/UNIT
OUTDOOR		8	63		
TOTAL		18	355		1355
NOTES:					



SITE PLAN PROPOSED MIXED-USE DEVELOPMENT 1381 LAKESHORE ROAD CITY OF MISSISSAUGA

DATE:	JUNE 2020	PROJECT No.:	2019-0263	
SCALE:	N.T.S.	FIGURE No.:	1-2	

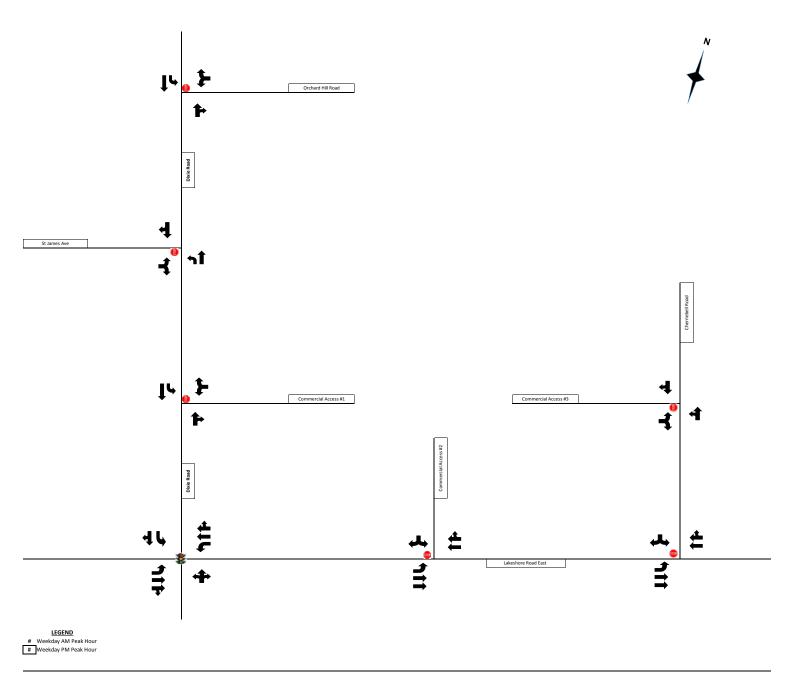
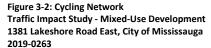


Figure 3-1: Study Intersections and Lane Configuration Traffic Impact Study - Mixed-Use Development 1381 Lakeshore Road East, City of Mississauga 2019-0263









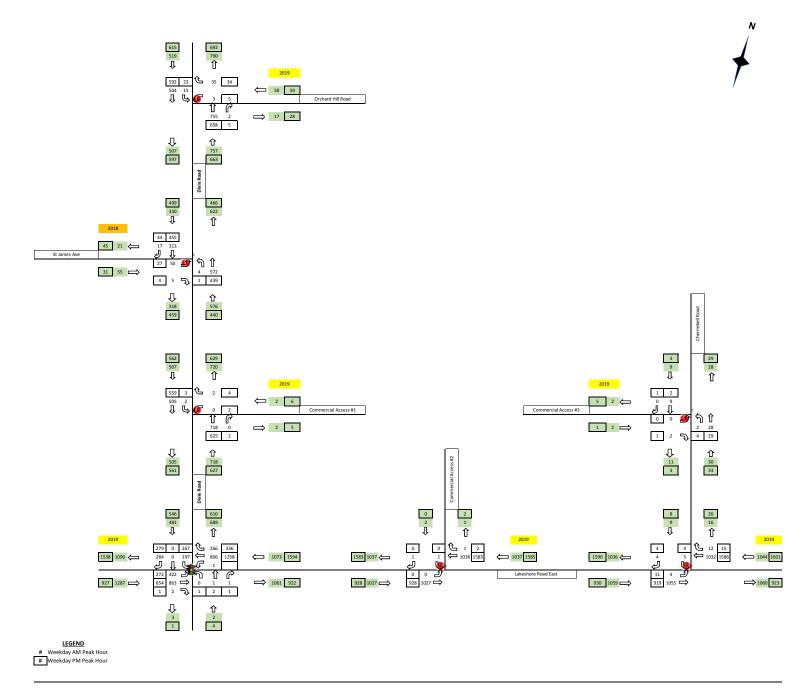
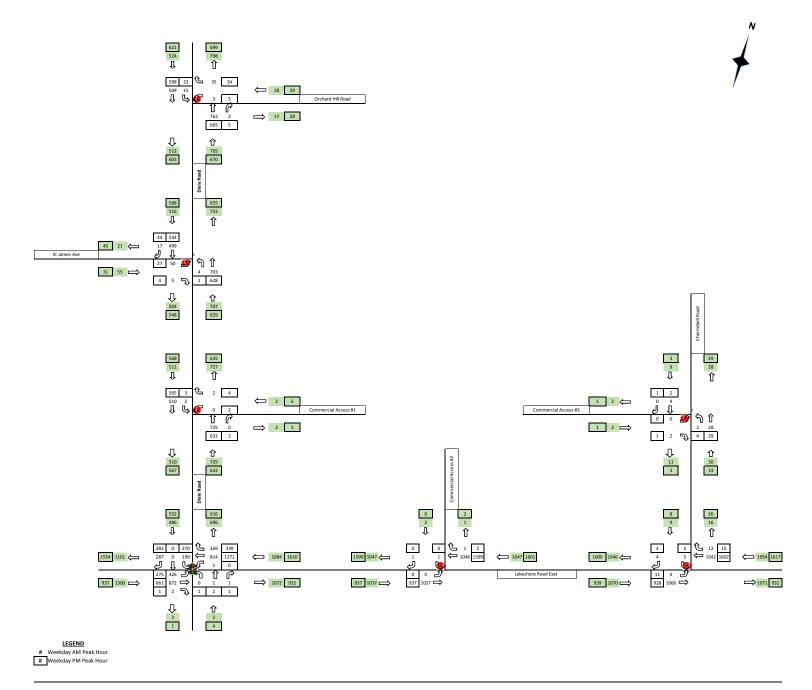


Figure 3-3: 2018-2019 Traffic Survey Volumes Traffic Impact Study - Mixed-Use Development 1381 Lakeshore Road East, City of Mississauga 2019-0263





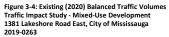
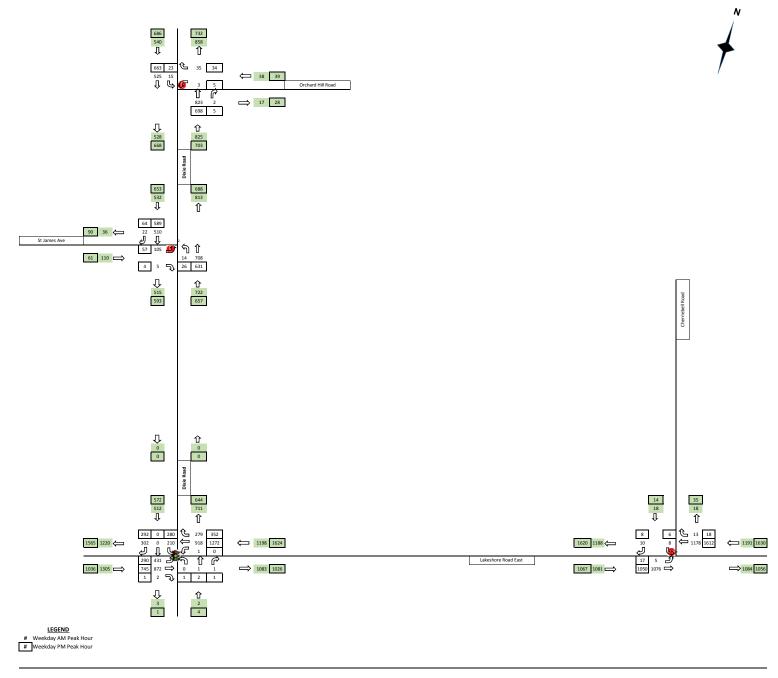


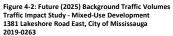




Figure 4-1: Future Background Development - Lakeview Village Traffic Impact Study - Mixed-Use Development 1381 Lakeshore Road East, City of Mississauga 2019-0263









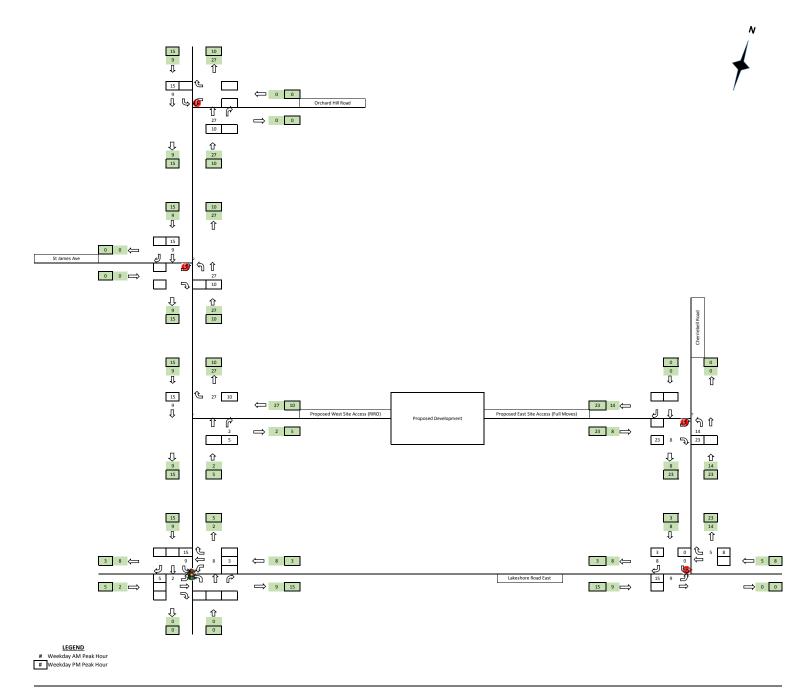


Figure 5-1: Site Generated Traffic Volumes Traffic Impact Study - Mixed-Use Development 1381 Lakeshore Road East, City of Mississauga 2019-0263



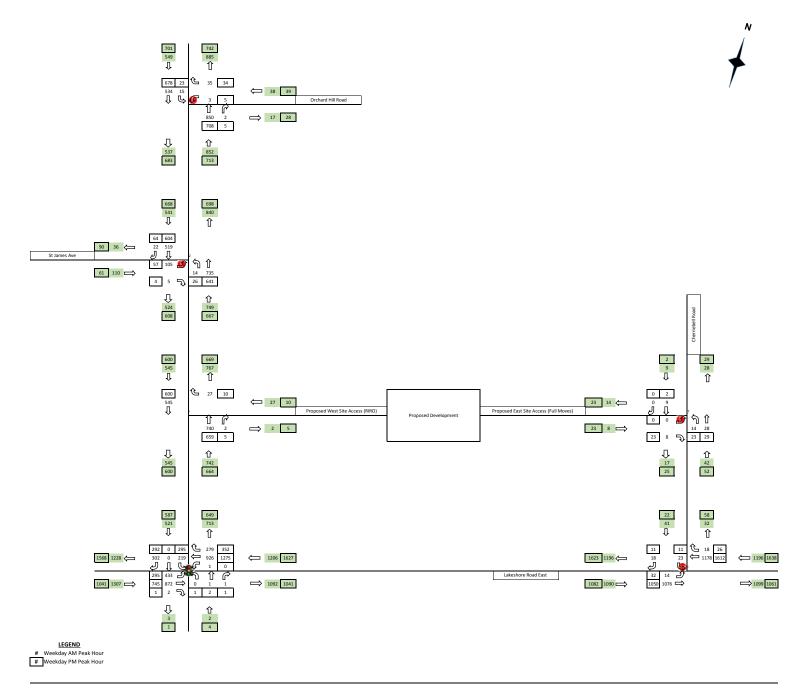
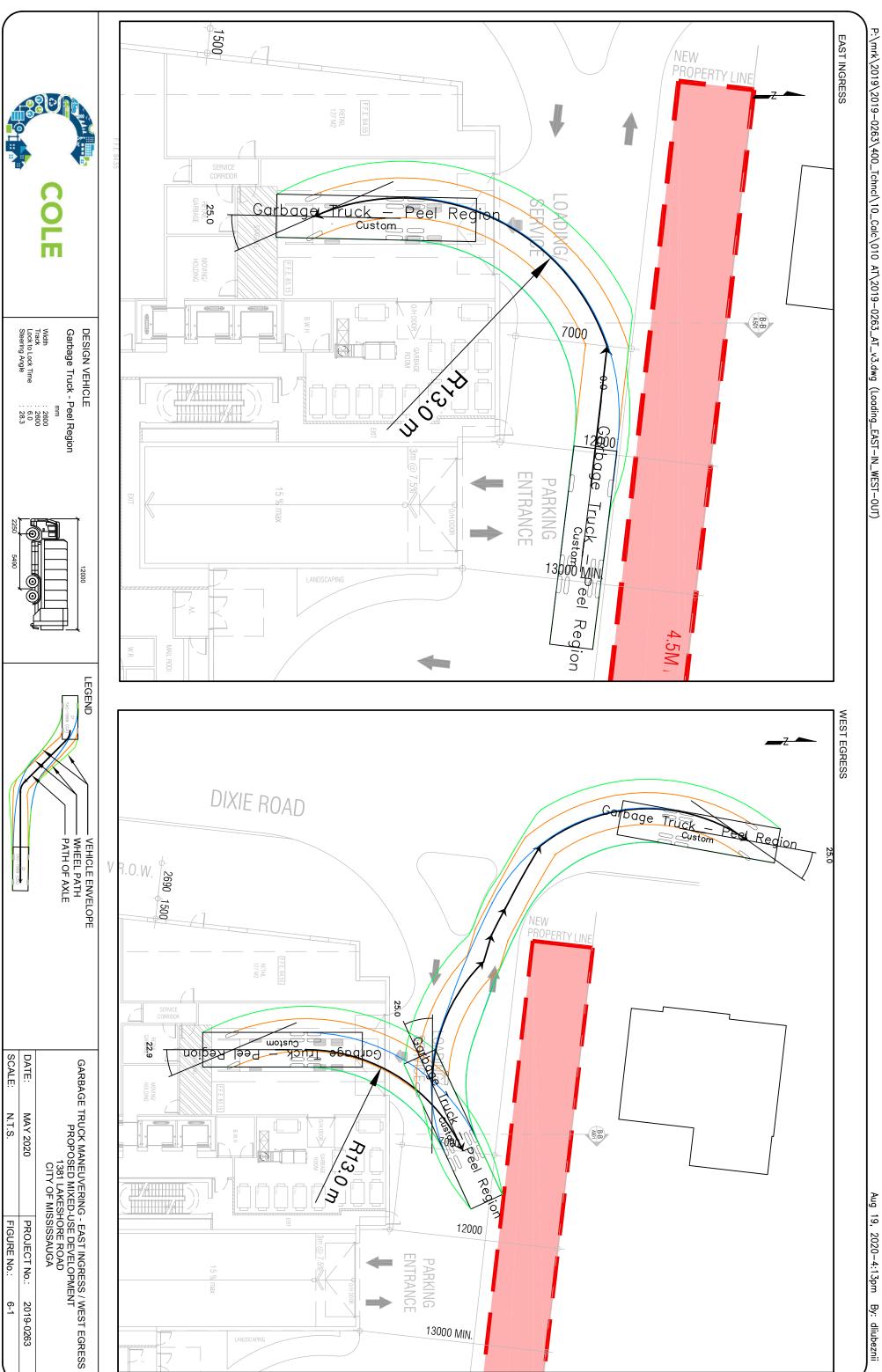
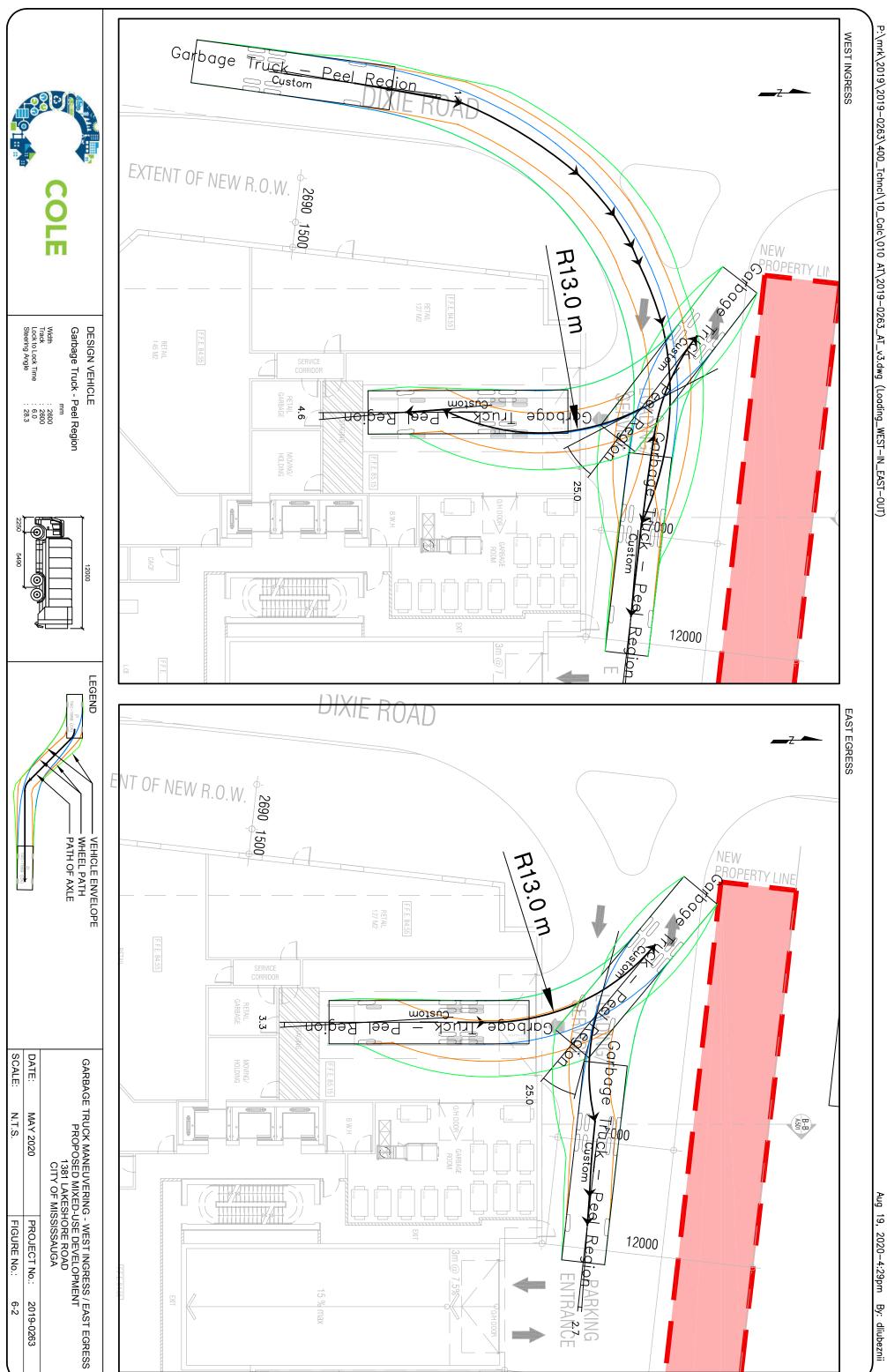


Figure 5-2: Future (2025) Total Traffic Volumes Traffic Impact Study - Mixed-Use Development 1381 Lakeshore Road East, City of Mississauga 2019-0263







Aug 19, 2020-4:29pm By: dliubeznii

Aug 19, 2020—4:37pm By: dliubeznii

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Aug 19, 2020—4:39pm By: dliubeznii

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Aug 19, 2020-4:41pm By: dliubeznii



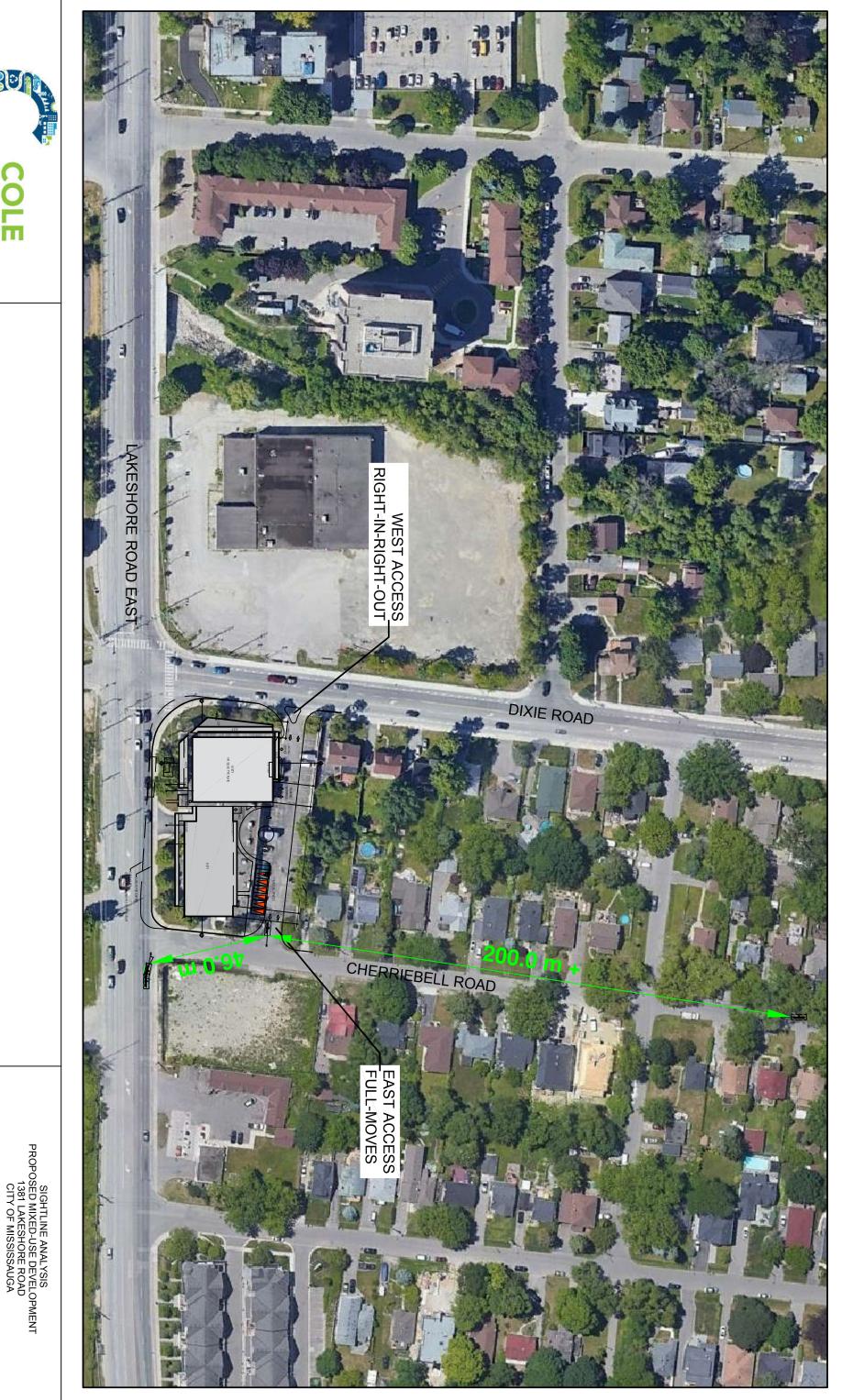
SCALE:

N.T.S. **JUNE 2020**

FIGURE No.: PROJECT No.:

6-6 2019-0263

DATE:



APPENDIX A Correspondence with the City



June 27, 2019 Our Ref: 2019-0263

Gregory Borys, C.E.T. City of Mississauga 300 City Centre Drive Mississauga, ON L5C 3C1

Attention: Mr. Borys
Traffic Planning Technologist

Re: Terms of Reference for Traffic Impact and Parking Study

1381 Lakeshore Road East

City of Mississauga

Cole Engineering Group Ltd. (COLE) has been retained to undertake a Traffic Impact and Parking Study in support of an Official Plan Amendment, and Zoning By-law Amendment application for the above noted development located in the northwest quadrant of the Lakeshore Road East and Dixie Road intersection in the City of Mississauga (the "City").

The subject site consists of one (1) property, municipally known as 1381 Lakeshore Road West, in the City. Based on the attached preliminary Conceptual Site Plan, the intention is to provide a 12 story mixed-use building consisting of 175 residential units and 1,000 m² of neighborhood retail on the ground floor. Vehicular access to the subject site will be provided by full access via Cherriebell Road. A total of 193 parking spaces are provided on the preliminary Conceptual Site Plan.

If available we would like to obtain weekday AM (7:00 - 9:00), and PM (4:00 - 6:00) traffic turning movement counts and signal timing plans at the following key intersections:

- Lakeshore Road East and Dixie Road (Signalized);
- Lakeshore Road East and Cherriebell Road (Unsignalized); and,
- Dixie Road and Orchard Hill Road (Unsignalized).

Trip generation for the proposed development will be based on *Trip Generation Manual*, 10th Edition prepared by the Institute of Transportation Engineers (ITE) for Multifamily Housing (Mid-Rise) (land use code 221) and Shopping Center (land use code 820). We will evaluate the development in a five (5) and 10-year horizons from the full-built out year (2026 and 2036). AM and PM peak hours will be analyzed using the *Synchro 9.0* analysis package and *Highway Capacity Manual* procedures for the level of service, volume to capacity ratio, and queue analysis. A 15 m by 15 m daylight triangle at the intersection of Dixie Road and Lakeshore Road and a 7.5m by 7.5m daylight triangle at the northwest corner of

P:\mrk\2019\2019-0263\400_Tchncl\70_Rprts\TOR\2019-0263 1381 Lakeshore Rd E TOR_City_v2.docx

COLE ENGINEERING GROUP LTD.





Gregory Borys, C.E.T. City of Mississauga Page 2 June 27, 2019



Lakeshore Road and Cherriebell Road will be provided. Using the information contained in the 2016 Transportation Tomorrow Survey (TTS) for zones 3643, 3649, 3648 and 3642 the modes of transportation within the study area are provided in **Table 1** and **Table 2**.

Table 1 Non – Auto Modal Split Calculation (AM Peak Hour)

Duime and Travel Made			Т	TS Zones		
Primary Travel Mode	3642	3643	3648	3649	sum	Percentage
Transit excluding GO rail	131	125	138	95	489	5%
Cycle	38	0	0	0	38	0%
Auto driver	1496	1746	2185	2944	8371	77%
GO rail only	49	138	164 143		494	5%
Joint GO rail and local transit	102	16	19	75	212	2%
Auto passenger	126	122	98	296	642	6%
Taxi passenger	0	12	0	0	12	0%
Walk	31	146	161	230	568	5%
Total	1973	2305	2765	3783	10826	100%
Non-	Auto (AM	Peak Hour	12%			

Table 2 Non-Auto Modal Split Calculation (PM Peak Hour)

Drive and Travel Manda		·	Ť	TS Zones		
Primary Travel Mode	3642	3643	3648	3649	sum	Percentage
Transit excluding GO rail	146	91	162	173	572	4%
Auto driver	1661	1807	10330	79%		
GO rail only	82	149	551	4%		
Joint GO rail and local transit	78	28	32	68	206	2%
Auto passenger	195	214	363	342	1114	8%
Taxi passenger	0	12	0	0	12	0%
Paid rideshare	40	0	11	0	51	0%
Walk	39	49	77	109	274	2%
Total	2241	2350	3724	4795	13110	100%
Non	-Auto (PM	Peak Hou	r)			8%

Gregory Borys, C.E.T. City of Mississauga Page 3 June 27, 2019



The subject zone and adjacent TTS Zones have an existing non-auto modal split of approximately 12% and 8% during the AM and PM peak hours, respectively. We will apply appropriate transit mode split reduction to the residential trip generation calculations.

The trip distribution will be based on the latest 2016 Transportation Tomorrow Survey (TTS) and existing traffic patterns.

Please provide traffic growth rates for Dixie Road and Lakeshore Road East in the vicinity of the subject site

In our study, we are considering the following developments in **Table 3** as future background developments in the vicinity of the subject site.

Table 3 Future Background Developments

Location	Land Use
1345 Lakeshore Road East	4-12 Storeys, Condominium apartments, townhouses and 308 m² commercial space (Rezoning Application In process)
1407 Lakeshore Road East	Stacked Townhouses proposal with 24 residential units and 139 m2 retail space (Site Plan application in process)

If there are any traffic studies available for the above sites, we would like to obtain the associated site traffic figures.

The proposed vehicle and bicycle supply for residential uses will be compared to the requirements per the City Zoning By-laws. Accessible and secure short term and long term bicycle storage facilities will be provided as per **Table 4**.

Table 4 Bicycle Parking Requirements

Туре	Long Term (space/100m2)	Short Term (space/100m2)
Residential	0.70	0.08
Retail	0.10	0.25

A parking utilization study will be provided with the TIPS to justify the parking reduction. Parking surveys will be conducted two days per week for two consecutive weeks. The survey will be conducted from 6 pm to 12 am in 30 minute intervals for existing parking demand (occupied/unoccupied space) during weekdays (Tuesday, Wednesday or Thursday).

The internal site circulation and the functionality of the fire route/emergency service access, loading, and parking activities will be analyzed using *AutoTurn 10*, to confirm the turning radius requirements and site circulation for both passenger and heavy vehicles related to the proposed development. A sight line analysis will be provided about ingress and egress as per the TAC Guidelines.

Gregory Borys, C.E.T. City of Mississauga Page 4 June 27, 2019



We would also like to know if there any planned road improvements within the next five (5) years in the subject area.

We trust this terms of reference is to your satisfaction, and should you have any questions or comments, or require a meeting to discuss other matters; please do not hesitate to contact the undersigned. We thank you for your assistance.

Yours truly,

COLE ENGINEERING GROUP LTD.

Kim Nystrom, L.E.L., C.E.T.

Principal

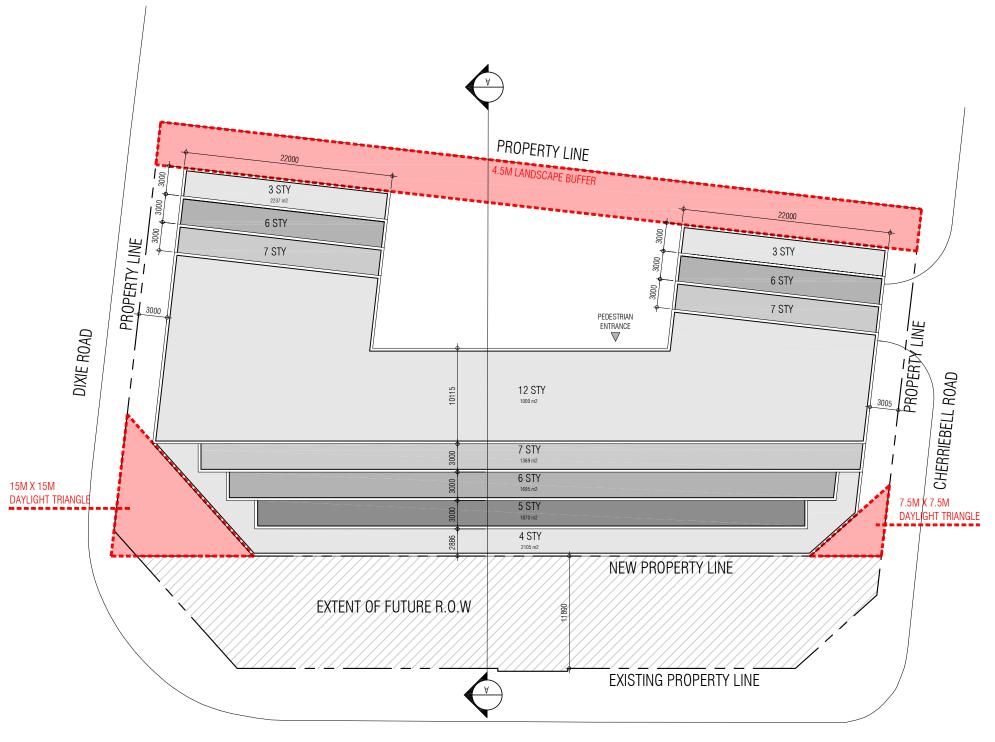
Urban Development, Traffic

Sevim Coskun, C.E.T. Senior Transportation Analyst

Urban Development, Traffic

SC:

Encls. Conceptual Site Plan, dated March 27, 2019



PROJECT STATISTICS	MID-RISE
SITE AREA	3,363m2
SIIL ANLA	3,3031112
G.C.A (above grade)	18,294m2
F.S.I (based on G.C.A)	5.44
LINUT COUNT	175
UNIT COUNT	175
	1.1/U
PARKING (based on 2 lvls)	193
BLDG HEIGHT (STY)	12

LAKESHORE ROAD EAST

Dumitru Liubeznii

From: Teresa Kerr <Teresa.Kerr@mississauga.ca>
Sent: Monday, October 28, 2019 5:05 PM

To: Sevim Coskun

Cc: David Ferro; Kim Nystrom; Greg Borys; Anthony Fotino; Teresa Kerr

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference - Parking Utilization Study

Follow Up Flag: Follow up Flag Status: Flagged

Hello Sevim,

Thank you for the information regarding your plans to conduct a parking utilization study for the above noted proposal. The information provided indicates that the proposal is for a 12-storey mixed-use building consisting of 175 residential units and 1,000 m² of retail on the ground floor. Also from the information provided it appears that the study is being conducted for the purpose of justifying reduced parking for the resident component of the proposal.

Staff support the two proxy locations; 25 Agnes St and 2929 Aquitaine Ave. Please note that there appears to be a discrepancy with regard to the number of existing units at 2929 Aquitaine Ave. Our records indicate that there are 176 units on the site vs 554 units noted in your information.

Staff support the general methodology proposed, conducting the surveys two days/week for two consecutive weeks, but would like to suggest minor revisions to the days and times. For the first week we suggest surveying a Sunday and Monday evening; for the second week, resurvey the day which yielded the greatest demand from the first week, and an additional day such as Tuesday or Wednesday. We also suggest shifting the survey time from 6 pm to 12 am, to 7 pm to 1 am.

Further, the proposed parking standards should be per number of bedrooms. To help with this analysis the following information on the survey sites is provided:

25 Agnes St – 95 one bedroom units, 2 two bedroom units.

2929 Aquitaine Ave – 22 one bedroom units, 88 two bedroom units, 66 two bedroom and den units.

Should you have any questions please call me.

Teresa



Teresa Kerr

Planner, Community Relations T 905-615-3200 ext. 5521 teresa.kerr@mississauga.ca

<u>City of Mississauga</u> | Planning and Building Department, City Planning Strategies

Please consider the environment before printing.

From: Sevim Coskun [mailto:SCoskun@coleengineering.ca]

Sent: Wednesday, October 23, 2019 2:54 PM

To: Teresa Kerr

Cc: David Ferro; Kim Nystrom; Greg Borys

Subject: FW: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi Teresa,

It was nice talking with you over the phone. Further to our conversation, please find the attached Terms of Reference for the proposed 1381 Lakeshore Road mixed-use development.

As stated in our Terms of Reference, on page 3, COLE will be conducting a parking utilization study to justify the parking reduction. Parking surveys will be conducted two days per week for two consecutive weeks. The survey will be conducted from 6 pm to 12 am in 30-minute intervals for existing parking demand (occupied/unoccupied space) during weekdays (Tuesday, Wednesday or Thursday).

We have contacted condo cooperations in the City of Mississauga if they would allow us to conduct a parking survey on their property, so we can get a better idea of parking demands at similar condominium developments, and find out if it is feasible to provide fewer parking spaces for the proposed development. We have the below list that allowed us to conduct the survey. Please kindly review the list and let us know that we can go ahead get the counts done.

"Waterford" at 25 Agnes St, Mississauga	102 Units	Mereka Properties (2000) Ltd
2929 Aquitaine Ave, Mississauga	554 Units	360 Community Management Ltd.

Regards,

Sevim Coskun, C.E.T. Senior Transportation Analyst, Urban Development Industrial, Commercial, Institutional & Traffic

Cole Engineering Group Ltd.

151 Superior Boulevard, Unit 1 & 2, Mississauga, ON L5T 2L1

T: 905-754-8060 Ext: 618 Tor. Line: 416-987-6161

F: 905-364-6162

E: scoskun@ColeEngineering.ca www.ColeEngineering.ca

CONFIDENTIALITY NOTE

This email may contain confidential information and any rights to privilege have not been waived. If you have received this transmission in error, please notify us by telephone or e-mail. Thank you.



Please consider the environment before printing this email

From: David Ferro [mailto:David.Ferro@mississauga.ca]

Sent: October-23-19 1:30 PM

To: Sevim Coskun < SCoskun@coleengineering.ca>

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi Sevim, please contact Teresa Kerr x 5521.

Thanks

David

David Ferro BURPL MCIP RPP

Development Planner – Mississauga South <u>City of Mississauga</u> | Planning and Building Department, Development and Design



From: David Ferro

Sent: October 23, 2019 12:00 PM

To: 'Sevim Coskun' **Cc:** Greg Borys

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi Sevim,

I am still in the process of confirming the appropriateness of those sites with our Policy Division whom review the PUS reports. You can come in an any time to review the site plan at 1041 Lakeshore Road East.

Thanks

David

David Ferro BURPL MCIP RPP

Development Planner – Mississauga South <u>City of Mississauga</u> | Planning and Building Department, Development and Design



From: Sevim Coskun [mailto:SCoskun@coleengineering.ca]

Sent: October 23, 2019 11:51 AM

To: David Ferro **Cc:** Greg Borys

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi David,

I am following up with the below email. I was wondering if you had a chance look into and have any questions or feedback. We would like to go ahead and get the proxy survey completed.

Looking to hearing from you.

Regards,

Sevim Coskun, C.E.T. Senior Transportation Analyst, Urban Development Industrial, Commercial, Institutional & Traffic

Cole Engineering Group Ltd.

151 Superior Boulevard, Unit 1 & 2, Mississauga, ON L5T 2L1

T: 905-754-8060 Ext: 618 Tor. Line: 416-987-6161

F: 905-364-6162

E: scoskun@ColeEngineering.ca

www.ColeEngineering.ca

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Please consider the environment before printing this email

From: Sevim Coskun

Sent: October-18-19 4:30 PM

To: 'David Ferro' < <u>David.Ferro@mississauga.ca</u>>
Cc: Greg Borys < <u>Gregory.Borys@mississauga.ca</u>>

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi David,

I hope this email finds you well and thank you for your reply and for sending through the concept plan and TIS. Yes, I would like to come in and take a look at the latest concept plan.

Also, as stated in our Terms of Reference, COLE will be conducting a parking utilization study to justify the parking reduction. Parking surveys will be conducted two days per week for two consecutive weeks. The survey will be conducted from 6 pm to 12 am in 30-minute intervals for existing parking demand (occupied/unoccupied space) during weekdays (Tuesday, Wednesday or Thursday).

We have contacted condo cooperation in the City of Mississauga if they would allow us to conduct a parking survey on their property, so we can get a better idea of parking demands at similar condominium developments, and find out if it is feasible to provide fewer parking spaces for proposed development. We have the below list that allowed us to conduct the survey. Please kindly review the list and let us know that we can go ahead get the counts done.

"Waterford" at 25 Agnes St, Mississauga	102 Units	Mereka Propertie
2929 Aquitaine Ave, Mississauga	554 Units	360 Community N

Regards,

Sevim Coskun, C.E.T. Senior Transportation Analyst, Urban Development Industrial, Commercial, Institutional & Traffic

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From: David Ferro [mailto:David.Ferro@mississauga.ca]

Sent: October-04-19 9:43 AM

To: Sevim Coskun < <u>SCoskun@coleengineering.ca</u>> **Cc:** Greg Borys < <u>Gregory.Borys@mississauga.ca</u>>

Subject: FW: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi Sevim,

I cant provide comments on a TIS as im not a traffic engineer. I see that you have Greg from our Traffic Planning section cc'd on this email. I will defer to him.

For 1345 Lakeshore Road East – attached is the most up to date concept plan and TIS.

For 1407 Lakeshore Road East – the proposed development is going through a site plan and as such, no TIS was required. The application has been inactive for 3 years and we recently learned that there is a new purchaser for the site, which means the proposal will most likely change. I only have hard copies of the current Site Plan so you will have to come in and view them.

Thanks

David

David Ferro BURPL MCIP RPPDevelopment Planner – Mississauga South
<u>City of Mississauga</u> | Planning and Building Department,
Development and Design



From: Sevim Coskun [mailto:SCoskun@coleengineering.ca]

Sent: October 3, 2019 5:29 PM

To: David Ferro

Cc: Kim Nystrom; Greg Borys

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi David,

I hope this email finds you well.

Cole has been retained to undertake a Traffic Impact and Parking Study in support of an Official Plan Amendment, and Zoning By-law Amendment application for the above-noted development located in the northwest quadrant of the Lakeshore Road East and Dixie Road intersection in the City.

I am writing to obtain your feedback regarding the attached Terms of Reference for the proposed site.

Also, I would like to come in and review the associated Traffic Impact Studies and Site Plans for 1345 Lakeshore Road East and 1407 Lakeshore Road East which will be the future background developments to the 1381 Lakeshore Road East site

Looking to hear from you.

Regards,

Sevim Coskun, C.E.T. Senior Transportation Analyst, Urban Development Industrial, Commercial, Institutional & Traffic

Cole Engineering Group Ltd.

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From: Greg Borys [mailto:Gregory.Borys@mississauga.ca]

Sent: July-05-19 2:56 PM

To: Sevim Coskun < <u>SCoskun@coleengineering.ca</u>> **Cc:** Kim Nystrom < KNystrom@coleengineering.ca>

Subject: RE: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Good afternoon Sevim,

Thank you for providing the Terms of Reference for the proposed development at Lakeshore Road East. Please find some additional comments below:

- The historical AADT data and Turning Movement Count can be obtained from William Wright (<u>William.Wright@mississauga.ca</u>, Ext. 3221). If the data is older than 2 years, than consultant is responsible to conduct the latest counts;
- Please contact Tyler Xuereb < tyler.xuereb@mississauga.ca> to confirm growth rates;
- The signal timing plan for signalized intersections can be obtained from Jim Kartsomaniz (<u>Jim.Kartsomanis@mississauga.ca</u>, Ext. 3964)
- Internal Road System Driveway Locations Please include the truck turning templates for the proposed Car Wash/Jiffy Lube and Tanker Trucks;
- Safety & Operational Analysis required under the TIS, site line analysis to be included;
- Any Site Plan that will be included in the TIS will require to show required land dedication and sight triangles;
- TIS to include Lakeshore Connecting Communities, City Council endorsed Transportation Master Plan;
- Please review Mississauga TIS Guidelines (http://www.mississauga.ca/file/COM/Traffic-Impact-Study-

Guidelines.pdf)

Note: Parking Utilization Study ToR will require to be reviewed and approved by the Planner on file, the study will not be reviewed by this department.

Additional information: I will check if any previous TIS have been complete for the proposed developments as outlined in Table 3.

If you have any other questions or concerns please feel free to contact me.

Regards,



Gregory Borys, C.E.T.

Traffic Planning Technologist, Transportation & Works T 905-615-3200 ext.3597 gregory.borys@mississauga.ca

<u>City of Mississauga</u> | Transportation and Works Department Transportation and Infrastructure Planning Division

Please consider the environment before printing.

From: Sevim Coskun [mailto:SCoskun@coleengineering.ca]

Sent: 2019/06/27 3:14 PM

To: Greg Borys **Cc:** Kim Nystrom

Subject: 2019-0263 1381 Lakeshore Rd E Terms of Reference (TIPS)

Hi Gregory,

Please find the attached Terms of Reference for the proposed 1381 Lakeshore Road mixed-use development.

Should you have any questions let us know.

Regards,

Sevim Coskun, C.E.T. Senior Transportation Analyst, Urban Development Industrial, Commercial, Institutional & Traffic

Cole Engineering Group Ltd.

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APPENDIX B Exisitng Transit Routes

APPENDIX C Existing Traffic Movement Counts and Signal Timing Plans



Accu-T	raffic Inc.							
Morning Peak Diagram	Specified Period One Hour Peak From: 7:00:00 From: 7:45:00 To: 9:00:00 To: 8:45:00							
Municipality: Mississauga Site #: 1920200001 Intersection: Dixie Rd & Orchard Hill Rd TFR File #: 1 Count date: 28-Nov-19	Weather conditions: Person counted: Person prepared: Person checked:							
** Non-Signalized Intersection **	Major Road: Dixie Rd runs N/S							
North Entering: 523 Trucks 13 0 North Peds: 0 Cars 471 15 Peds Cross: ✓ Totals 504 19	Heavys 16 Trucks 8 Cars 766 Totals 790 Cars Trucks Heavys Totals 35 0 0 35 N Corchard Hill Rd Corchard Hill Rd							
Trucks 13 Trucks 13 Heavys 21 Heavys 21 Heavys 21 Trucks 13	Cars Trucks Heavys Totals 17 0 4 21 Cars 731 2 733 Cars 8 0 8 South Peds: 0 South Entering: 757 Stals 755 2 South Leg Total: 1265							

Comments



Accu-Traffic Inc. **Specified Period One Hour Peak Afternoon Peak Diagram** From: 16:00:00 From: 16:45:00 To: To: 18:00:00 17:45:00 Weather conditions: Municipality: Mississauga Site #: 1920200001 Intersection: Dixie Rd & Orchard Hill Rd Person counted: TFR File #: Person prepared: Count date: 28-Nov-19 Person checked: ** Non-Signalized Intersection ** Major Road: Dixie Rd runs N/S North Leg Total: 1307 Heavys 3 0 3 Heavys 6 East Leg Total: 67 North Entering: 615 Trucks 0 Trucks 8 East Entering: North Peds: Cars 588 23 611 Cars 678 East Peds: X Peds Cross: Totals 592 23 Totals 692 Peds Cross: Dixie Rd Trucks Heavys Totals Cars 34 0 5 Orchard Hill Rd Cars Trucks Heavys Totals 28 0 0 28 Dixie Rd 649 Peds Cross: M Cars 593 Cars 644 5 8 Trucks 1 Trucks 8 0 South Peds: 0 6 Heavys Heavys 6 South Entering: 663

Comments

Totals

658

5

South Leg Total: 1260

Totals 597



Total Count Diagram

Municipality: Mississauga

Site #: 1920200001

Intersection: Dixie Rd & Orchard Hill Rd

TFR File #:

Count date: 28-Nov-19 Weather conditions:

Person counted: Person prepared:

Person checked:

** Non-Signalized Intersection **

North Leg Total: 4837 North Entering: 2102 North Peds: Peds Cross:

Heavys 39 7 46 29 Trucks 29 0 Cars 1963 64 2027 Totals

2031 71 Dixie Rd



Heavys 40 Trucks 29 Cars 2666

Totals 2735

Major Road: Dixie Rd runs N/S

East Peds: Peds Cross:

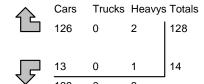
East Entering:

East Leg Total: 221

142

X









Cars 1976 Trucks 29 Heavys Totals 2045

Trucks Heavys Totals

2547 29 0 29 38 1 39 2607 8

Cars Trucks Heavys Totals 71 0 8 79

> Peds Cross: M South Peds: South Entering: 2615 South Leg Total: 4660

Comments



Accu-Traffic Inc. Traffic Count Summary

Intersection:	Dixie Ro	d & Orch	ard Hill F	₹d	Count [Date: 28-Nov-1	9 Mu	^{ınicipality:} Mi	ssissau	ga					
		h Appro				Namth /Cauth				ach To	tals				
Hour	Includ	es Cars, T	rucks, & H		Total	North/South Total	Hour	Includ	es Cars, T	rucks, & F		Total			
Ending	Left	Thru	Right	Grand Total	Peds	Approaches	Ending	Left	Thru	Right	Grand Total	Peds			
7:00:00	0	0	0	0	0	0	7:00:00		0	0	0	0			
8:00:00	14	385	0	399	0	1004	8:00:00		603	2	605	0			
9:00:00	13	492	0	505	0	1239	9:00:00		734	0	734	0			
16:00:00	0	0_	0	0	0	0	16:00:0		0	0	0	0			
17:00:00	18 26	577 577	0	595	0	1225	17:00:0		628	2 4	630	3 0			
18:00:00	26	577	0	603	0	1249	18:00:0	0 0	642	4	646	U			
Totals:	71 Eas :	2031 t Appro	0 ach Tota	2102 als	0	4717 East/West	S Totals	Wes		8 ach Tot		3			
Hour Ending	Includ	es Cars, T	Tucks, & F	Grand	Total Peds	Total	Hour Ending	Includ	es Cars, i	rucks, & F	Grand	Total Peds 0 0 0			
	Left	Thru	Right	Total		Approaches		Left	Thru	Right	Total				
7:00:00	0	0	0	0	0	0	7:00:00		0	0	0				
8:00:00	2	0	22	24	1	24	8:00:00		0	0	0				
9:00:00 16:00:00	5 0	0	36 0	41 0	1 0	41 0	9:00:00 16:00:0		0	0	0 0	0			
17:00:00	2	Ö	<i>4</i> 3	45	3	45	17:00:0		O	o	0	0			
18:00:00	5	Ö	27	32	Ö	32	18:00:0		Ö	Ö	l ő l	Ö			
16.00.00	3	U	27	32	U	<i>3</i> 2	78.00.0				O	Ü			
										_					
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Totals: Hours E Crossing	nding:	7:00					•	Major Str		0:00	0 1	0			



Count	Date:	ZQ-NOV	-19	Site #:	192020	0001	1												1	
		Passeng	ger Cars -	North A	pproach			True	cks - Nort	h Approa	ach			He	avys - No	rth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	2	2	62	62	0	0	0	0	3	3	0	0	0	0	2	2	0	0	0	0
7:30:00	6	4	136	74	0	0	0	0	3	0	0	0	0	0	3	1	0	0	0	0
7:45:00	6	0	248	112	0	0	0	0	6	3	0	0	0	0	5	2	0	0	0	0
8:00:00	12	6	368	120	0	0	0	0	9	3	0	0	2	2	8	3	0	0	0	0
8:15:00	14	2	483	115	0	0	0	0	13	4	0	0	3	1	14	6	0	0	0	0
8:30:00	20	6	600	117	0	0	0	0	17	4	0	0	3	0	19	5	0	0	0	0
8:45:00	21	1	719	119	0	0	0	0	19	2	0	0	4	1	25	6	0	0	0	0
9:00:00	22	11	822	103	0	0	0	0	21	2	0	0	5	1	34	9	0	0	0	0
9:15:00	22	0	822	0	0	0	0	0	21	0	0	0	5	0	34	0	0	0	0	0
16:00:00	22	0	822	0	0	0	0	0	21	0	0	0	5	0	34	0	0	0	0	0
16:15:00	26	4	967	145	0	0	0	0	23	2	0	0	6	1	35	1	0	0	0	0
16:30:00	31	5	1092	125	0	0	0	0	24	1	0	0	7	1	36	1	0	0	0	0
16:45:00	34	3	1232	140	0	0	0	0	27 27	3	0	0	7	0	36	0	0	0	0	0
17:00:00 17:15:00	38 44	4	1389 1535	157 146	0	0	0	0	27	0	0	0	7	0	38 38	2	0	0	0	0
17:15:00	51	6 7	1680	145	0	0	0	0	27	0	0	0	7	0	39	0 1	0	0	0	0
17:30:00	57	6	1820	140	0	0	0	0	28	1	0	0	7	0	39	0	0	0	0	0
18:00:00	64	7	1963	143	0	0	0	0	29	1	0	0	7	0	39	0	0	0	0	0
18:15:00	64	0	1963	0	0	0	0	0	29	0	0	0	7	0	39	0	0	0	0	0
18:15:15	64	0	1963	0	0	0	0	0	29	0	0	0	7	0	39	0	0	0	0	0
10.15.15	0-7	0	1303	- 0		0	0	0	25	0	U	- 0	,	0	00	0		0	0	- 0



	Date:			Site #:																
		Passen	ger Cars	- East Ap	proach			Tru	cks - Eas	t Approa	ch			Не	avys - Ea	ast Appro	pach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	jht	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Rig	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	2	2	0	0	2	2	0	0	0	0	0	0	0	0	0	0	0	0	1	1
7:30:00	2	0	0	0	6	4	0	0	0	0	0	0	0	0	0	0	0	0	1	0
7:45:00	2	0	0	0	12	6	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:00:00	2	0	0	0	22	10	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:15:00	2	0	0	0	30	8	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:30:00	5	3	0	0	39	9	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:45:00	5	0	0	0	47	8	0	0	0	0	0	0	1	1	0	0	0	0	2	1
9:00:00	6	1	0	0	57	10	0	0	0	0	0	0	1	0	0	0	1	1	2	0
9:15:00	6	0	0	0	57	0	0	0	0	0	0	0	1	0	0	0	1	0	2	0
16:00:00	6	0	0	0	57	0	0	0	0	0	0	0	1	0	0	0	1	0	2	0
16:15:00	6	0	0	0	64	7	0	0	0	0	0	0	1	0	0	0	1	0	4	2
16:30:00	7	1	0	0	76	12	0	0	0	0	0	0	1	0	0	0	2	1	4	0
16:45:00	7	0	0	0	89	13	0	0	0	0	0	0	1	0	0	0	2	0	5	1
17:00:00 17:15:00	8	1 1	0	0	99 109	10 10	0	0	0	0	0	0	1	0	0	0	2	0	5 5	0
17:15:00	9 12	3	0	0	120	11	0	0	0	0	0	0	1	0	0	0	2	0	5	0
17:30:00	12	0	0	0	123	3	0	0	0	0	0	0	1	0	0	0	2	0	5	0
18:00:00	13	1	0	0	126	3	0	0	0	0	0	0	1	0	0	0	2	0	5	0
18:15:00	13	0	0	0	126	0	0	0	0	0	0	0	1	0	0	0	2	0	5	0
18:15:15	13	0	0	0	126	0	0	0	0	0	0	0	1	0	0	0	2	0	5	0
10.10.10	13		0	- 0	120	- 0	0	0		- 0	U	- 0		- 0	U	- 0		0		



Count	Date:	28-Nov	-19	Site #:	192020	0001	1												1	
		Passeng	er Cars -	South A	pproach			Truc	ks - Sout	h Appro	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	114	114	0	0	0	0	1	11	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	246	132	0	0	0	0	3	2	0	0	0	0	2	2	0	0	0	0
7:45:00	0	0	404	158	0	0	0	0	3	0	0	0	0	0	3	1	0	0	0	0
8:00:00	0	0	591	187	2	2	0	0	4	11	0	0	0	0	8	5	0	0	0	0
8:15:00	0	0	789	198	2	0	0	0	7	3	0	0	0	0	9	11	0	0	0	0
8:30:00	0	0	964	175	2	0	0	0	8	11	0	0	0	0	13	4	0	0	0	0
8:45:00	0	0	1135	171	2	0	0	0	11	3	0	0	0	0	19	6	0	0	0	0
9:00:00	0	0	1305	170	2	0	0	0	12	1	0	0	0	0	20	1	0	0	0	0
9:15:00	0	0	1305	0	2	0	0	0	12	0	0	0	0	0	20	0	0	0	0	0
16:00:00	0	0	1305	0	2	0	0	0	12	0	0	0	0	0	20	0	0	0	0	0
16:15:00	0	0	1471	166	2	0	0	0	15	3	0	0	0	0	28	8	0	0	0	0
16:30:00	0	0	1603	132	2	0	0	0	19	4	0	0	0	0	29	1	1	1	0	0
16:45:00	0	0	1740	137	2	0	0	0	21 23	2	0	0	0	0	32	3	1	0	1	1
17:00:00 17:15:00	0	0	1908 2057	168 149	3 4	1 1	0	0	25	2	0	0	0	0	34 36	2	1	0	3	2
17:15:00	0	0	2057	179	4	0	0	0	25	2	0	0	0	0	37	1	1	0	3	0
17:30:00	0	0	2384	148	7	3	0	0	29	2	0	0	0	0	38	1	1	0	3	0
18:00:00	0	0	2540	156	7	0	0	0	29	0	0	0	0	0	38	0	1	0	3	0
18:15:00	0	0	2540	0	7	0	0	0	29	0	0	0	0	0	38	0	1	0	3	0
18:15:15	0	0	2540	0	7	0	0	0	29	0	0	0	0	0	38	0	1	0	3	0
10.13.13	0		2540	- 0	,	0		0	25	0	0	- 0		0	30	0	'	- 0		- 0



Count	Date:	28-Nov	-19	Site #:	192020	0001														
	Passenger Cars - West Ap			proach		Trucks - West Approach						Heavys - West Approach						Pedes	Pedestrians	
Interval	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		West Cross	
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30:00 16:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:15	0	0	0	0	0	0	0	0	ō	0	0	0	0	0	0	0	0	0	0	0



ACCU	Traffic Inc.								
Morning Peak Diagram	Specified Period One Hour Peak From: 7:00:00 From: 7:30:00 To: 9:00:00 To: 8:30:00								
Municipality: Mississauga Site #: 1920200002 Intersection: Lakeshore Rd E & Dixie Rd TFR File #: 1 Count date: 28-Nov-19	Weather conditions: Person counted: Person prepared: Person checked:								
** Signalized Intersection **	Major Road: Lakeshore Rd E runs W/E								
North Leg Total: 1170 Heavys 14 0 3 North Entering: 481 Trucks 10 0 1 North Peds: 13 Cars 260 0 193 Peds Cross: ✓ Totals 284 0 197	17 Heavys 12 East Leg Total: 2134 11 Trucks 5 East Entering: 1073 453 Cars 672 Totals 689 Peds Cross: ▼ Peds Cross: ▼ ■								
Heavys Trucks Cars Totals 40 19 1031 1090 Lakeshore Rd E	Dixie Rd Cars Trucks Heavys Total 260 0 6 266 771 9 26 806 1 1 1032 9 32								
Lakeshole Nd L	E 1032 9 32								
Heavys Trucks Cars Totals 6 5 411 422 26 4 833 863	Lakeshore Rd E								
0 0 2 32 9 1246 2 driv	Cars Trucks Heavys Total 1027 5 29 1061								
Peds Cross: X Cars 3 West Peds: 3 Trucks 0 West Entering: 1287 Heavys 0	Cars 0 1 1 2 Peds Cross: ✓ Trucks 0 0 0 0 South Peds: 2 Heavys 0 0 0 0 South Entering: 2 Totals 0 1 1 South Leg Total: 5								
West Leg Total: 2377 Totals 3	Totals 0 1 1 South Leg Total: 5								

Comments



Afternoon P	eak Dia	agran	''	Specifi From: To:	16:0	Period 00:00 00:00	I	-	• • • • • • • • • • • • • • • • • • • •	ır Pe 6:45:0 7:45:0	00
Municipality: Missis Site #: 19202 ntersection: Lakes FFR File #: 1 Count date: 28-No	00002 hore Rd E & D v-19	Dixie Rd		Weather Persor Persor Major I	n co n pre	unted epared ecked	: 1: :		I E rune	W/E	
North Leg Total: 1156 North Entering: 546 North Peds: 8 Peds Cross: Heavys Trucks Cars Total 14 5 1519 1538	Heavys 1 Trucks 2 Cars 276 Totals 279		3 2 65 541			Heavys Trucks Cars	3	Cars 332	East Leg East En East Pe Peds Cr Trucks 3	g Total: tering: ds: oss:	1594 1 X
Lakesho		,	w N	► E		•	<u>C</u>	1242 0 1574 eshore	3 0 6	13 0	1258
2 5 265 272 12 5 637 654 0 0 1 1 14 10 903		d	S riveway	\ 1	ĵ		Lak	Cars 903	Trucks	Heavy	s Total:
Peds Cross: X West Peds: 6 West Entering: 927 West Leg Total: 2465	Cars 1 Trucks 0 Heavys 0 Totals 1	Cars Trucks Heavys Totals	s 0 (2 0 0 2	0	4 0 0		Peds Cr South P	eds:		



1855

Dixie Rd

Total Count Diagram

Municipality: Mississauga

Site #: 1920200002

Intersection: Lakeshore Rd E & Dixie Rd

TFR File #:

Count date: 28-Nov-19 Weather conditions:

Person counted: Person prepared:

Person checked:

** Signalized Intersection **

North Leg Total: 4352 North Entering: 1924 North Peds: 40 Peds Cross:

Heavys 30 8 38 31 Trucks 28 0 3 Cars 1015 839 Totals 1073 850

Heavys 36 Trucks 26 Cars 2366 Totals 2428

East Leg Total: 8274 East Entering: 4713 East Peds: X Peds Cross:

Trucks Heavys Totals

1105

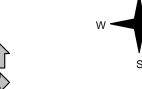
3603

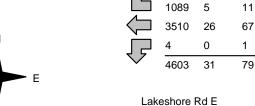
3561

5

Heavys Trucks Cars Totals 98 54 4528 4680







Major Road: Lakeshore Rd E runs W/E

Cars

Cars

3449

29

Heavys Trucks Cars Totals 25 21 1268 1314 75 26 2605 2706 0 0 8 8 100 3881

X Peds Cross: West Peds: West Entering: 4028 West Leg Total: 8708

Cars 13 Trucks 0 Heavys Totals 14

Cars 3 5 17 0 Trucks 0 0 Heavys 1 0 Totals 4 5

Peds Cross: M South Peds: South Entering: 18 South Leg Total: 32

Trucks Heavys Totals

83

Comments

driveway



Accu-Traffic Inc. Traffic Count Summary

Intersection:	akesho	re Rd F	& Divie	Rd	Count [Date: 28-Nov-1	g Mur	nicipality: Mi	ssissaur	na		
<u>'</u>			ach Tot			20-1107-1	<u> </u>			oach To	tale	
Hour			rucks, & H		Total	North/South	Hour			rucks, & F		Total
Ending				Grand	Peds	Total Approaches	Ending				Grand	Peds
7:00:00	Left 0	Thru O	Right 0	Total 0	0	0	7:00:00	Left 0	Thru O	Right 0	Total 0	0
8:00:00	157	0	236	393	10	395	8:00:00		1	Ö	2	2
9:00:00	205	Ō	271	476	14	477	9:00:00		Ô	1	1	1
16:00:00	0	0	0	0	0	0	16:00:00		0	0	0	0
17:00:00	224 264	1 0	294 272	519 536	11 5	533 537	17:00:00 18:00:00		8 0	4 0	14 1	2 0
18:00:00	204	U	212	536	5	537	10.00.00	Ί '	U		'	U
_ , ,	050		4070	4004	40	4040			•	_	40	_
Totals:	850 F act	1	1073	1924	40	1942	S Totals		9	5 ach Tot	18 <u> </u>	5
Hour			rucks, & H		Total	East/West	Hour			rucks, & F		Total
Ending				Grand	Peds	Total Approaches	Ending				Grand	Peds
7.00.00	Left	Thru	Right	Total			7.00.00	Left	Thru	Right	Total	
7:00:00 8:00:00	0 1	0 563	0 204	0 768	0 0	0 18 4 5	7:00:00 8:00:00		0 72 0	0 2	0 1077	<i>0</i> 3
9:00:00	1	683	251	935	0	2134	9:00:00		768	2	1199	3 1
16:00:00	ó	0	0	0	Ö	0	16:00:00		0	0	0	Ö
17:00:00	3	1146	306	1455	1	2287	17:00:00		554	4	832	8
18:00:00	0	1211	344	1555	1	2475	18:00:00	256	664	0	920	6
Totals:	5	3603	1105	4713	2	8741	 W Totals	1314	2706	8	4028	18
i otalo.	<u> </u>	0000				or Traffic Cr					1020	
Hours E	ndina.	7:00	8:00	9:00	16:00		17:00	18:00	0:00	0:00		
			162	206	0		243	272	0	0		
Crossing	j values	. 0	102	200	U		243	212	U	U		



		Passen	ger Cars -	North A	pproach			True	cks - Norti	h Approa	ach			He	avys - No	orth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	jht	Le	ft	Th	ru	Rig	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	28	28	0	0	37	37	0	0	0	0	4	4	1	1	0	0	2	2	5	5
7:30:00	58	30	0	0	86	49	1	1	0	0	6	2	1	0	0	0	3	1	6	1
7:45:00	99	41	0	0	155	69	1	0	0	0	9	3	1	0	0	0	5	2	10	4
8:00:00	154	55	0	0	217	62	1	0	0	0	12	3	2	1	0	0	7	2	10	0
8:15:00	204	50	0	0	283	66	1	0	0	0	14	2	3	1	0	0	13	6	14	4
8:30:00	251	47	0	0	346	63	2	1	0	0	16	2	4	1	0	0	17	4	19	5
8:45:00	315	64	0	0	403	57	2	0	0	0	18	2	4	0	0	0	22	5	21	2
9:00:00	354	39	0	0	459	56	3	1	0	0	20	2	5	1	0	0	28	6	24	3
9:15:00	354	0	0	0	459	0	3	0	0	0	20	0	5	0	0	0	28	0	24	0
16:00:00	354	0	0	0	459	0	3	0	0	0	20	0	5	0	0	0	28	0	24	0
16:15:00	413	59	1	1	536	77	3	0	0	0	21	1	5	0	0	0	29	1	26	2
16:30:00	461	48	1	0	600	64	3	0	0	0	23	2	6	1	0	0	29	0	28	2
16:45:00	516	55	1	0	675	75	3	0	Ö	0	25	2	6	0	0	0	29	0	32	4
17:00:00	576	60	1	0	746	71	3	0	0	0	25	0	7	1	0	0	30	1	35	3
17:15:00	653	77	1	0	818	72	3	0	0	0	26	1	7	0	0	0	30	0	36	1
17:30:00	711	58	1	0	881	63	3	0	0	0	26	0	8	1	0	0	30	0	39	3
17:45:00	781	70	1	0	951	70	3	0	0	0	27	1	8	0	0	0	30	0	40	1
18:00:00	839	58	1	0	1015	64	3	0	0	0	28	1	8	0	0	0	30	0	40	0
18:15:00	839	0	1	0	1015	0	3	0	0	0	28	0	8	0	0	0	30	0	40	0
18:15:15	839	0	1	0	1015	0	3	0	0	0	28	0	8	0	0	0	30	0	40	0
10.13.13	000	- 0	'	0	1013	U	3		0	- 0	20	U	0	- 0	U	- 0	30		40	- 0



		Passen	ger Cars	- East Ap	proach			Tru	cks - Eas	t Approa	ch			He	eavys - Ea	ast Appro	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	ft	Th	ru	Riç	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	69	69	26	26	0	0	1	1	0	0	1	1	2	2	0	0	0	0
7:30:00	0	0	149	80	68	42	0	0	1	0	0	0	1	0	6	4	1	1	0	0
7:45:00	0	0	313	164	123	55	0	0	2	1	0	0	1	0	10	4	1	0	0	0
8:00:00	0	0	543	230	199	76	0	0	5	3	0	0	1	0	15	5	5	4	0	0
8:15:00	0	0	728	185	266	67	0	0	6	1	0	0	1	0	23	8	5	0	0	0
8:30:00	1	1	920	192	328	62	0	0	10	4	0	0	1	0	32	9	7	2	0	0
8:45:00	1	0	1058	138	392	64	0	0	14	4	0	0	1	0	35	3	7	0	0	0
9:00:00	1	0	1191	133	448	56	0	0	15	1	0	0	1	0	40	5	7	0	0	0
9:15:00	1	0	1191	0	448	0	0	0	15	0	0	0	1	0	40	0	7	0	0	0
16:00:00	1	0	1191	0	448	0	0	0	15	0	0	0	1	0	40	0	7	0	0	0
16:15:00	1	0	1417	226	532	84	0	0	16	1	1	1	1	0	43	3	9	2	1	1
16:30:00	2	1	1721	304	597	65	0	0	19	3	2	1	1	0	47	4	9	0	1	0
16:45:00	4	2	1983	262	657	60	0	0	19	0	2	0	1	0	50	3	10	1	1	0
17:00:00	4	0	2321	338	747	90	0	0	19	0	4	2	1	0	52	2	10	0	1	0
17:15:00	4	0	2588	267	810	63	0	0	20	1	4	0	1	0	58	6	11	1	1	0
17:30:00	4	0	2927	339	908	98	0	0	21	1	5	1	1	0	61	3	11	0	2	1
17:45:00	4	0	3225	298	989	81	0	0	22	1	5	0	1	0	63	2	11	0	2	0
18:00:00	4	0	3510	285	1089	100	0	0	26	4	5	0	1	0	67	4	11	0	2	0
18:15:00	4	0	3510	0	1089	0	0	0	26	0	5	0	1	0	67	0	11	0	2	0
18:15:15	4	0	3510	0	1089	0	0	0	26	0	5	0	1	0	67	0	11	0	2	0
10.10.10			0010		1000								<u> </u>		0,		- ' '			
			1																	



Count	Sount Date: 28-Nov-19																ı			
		Passeng	er Cars -	South A	pproach			Truc	ks - Sout	h Appro	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1	1
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	0
7:45:00	0	0	1	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	0
8:00:00	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2	11
8:15:00	0	0	1	0	1	11	0	0	0	0	0	0	1	0	0	0	0	0	3	1
8:30:00	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	3	0
8:45:00	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	3	0
9:00:00	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	3	0
9:15:00	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	3	0
16:00:00	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	3	0
16:15:00	0	0	2	1	3	2	0	0	0	0	0	0	1	0	0	0	0	0	4	1
16:30:00 16:45:00	1	1 1	4 7	2	4	1	0	0	0	0	0	0	1	0	0	0	0	0	4	0
17:00:00	2	0	9	3 2	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5 5	0
17:00:00	2	0	9	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5	0
17:13:00	3	1	9	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5	0
17:45:00	3	0	9	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5	0
18:00:00	3	0	9	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5	0
18:15:00	3	0	9	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5	0
18:15:15	3	0	9	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	5	0
10.10.10																				



		Passen	ger Cars -	West A	oproach			Tru	cks - Wes	t Approa	ıch			Не	eavys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	jht	Le	ft	Th	ru	Rig	ght	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	74	74	102	102	0	0	1	1	2	2	0	0	0	0	7	7	0	0	1	1
7:30:00	155	81	256	154	0	0	3	2	6	4	0	0	1	1	12	5	0	0	1	0
7:45:00	252	97	454	198	1	1	3	0	6	0	0	0	2	1	16	4	0	0	3	2
8:00:00	346	94	689	235	2	1	4	1	8	2	0	0	5	3	23	7	0	0	3	0
8:15:00	467	121	909	220	2	0	7	3	9	1	0	0	5	0	29	6	0	0	3	0
8:30:00	566	99	1089	180	2	0	8	1	10	1	0	0	7	2	38	9	0	0	4	1
8:45:00	661	95	1260	171	3	1	10	2	13	3	0	0	14	7	42	4	0	0	4	0
9:00:00	758	97	1425	165	4	1	11	1	14	1	0	0	15	1	49	7	0	0	4	0
9:15:00	758	0	1425	0	4	0	11	0	14	0	0	0	15	0	49	0	0	0	4	0
16:00:00	758	0	1425	0	4	0	11	0	14	0	0	0	15	0	49	0	0	0	4	0
16:15:00	828	70	1550	125	5	1	13	2	16	2	0	0	19	4	53	4	0	0	7	3
16:30:00	892	64	1663	113	6	1	15	2	16	0	0	0	20	1	59	6	ō	0	10	3
16:45:00	954	62	1804	141	7	1	16	1	18	2	0	0	23	3	61	2	ő	0	10	0
17:00:00	1018	64	1960	156	8	1	16	0	18	0	0	0	24	1	64	3	0	0	12	2
17:15:00	1096	78	2119	159	8	0	18	2	20	2	0	0	25	1	67	3	0	0	13	1
17:30:00	1163	67	2286	167	8	0	19	1	21	1	0	0	25	0	70	3	0	0	16	3
17:45:00	1219	56	2441	155	8	0	21	2	23	2	0	0	25	0	73	3	0	0	16	0
18:00:00	1268	49	2605	164	8	0	21	0	26	3	0	0	25	0	75	2	0	0	18	2
18:15:00	1268	0	2605	0	8	0	21	0	26	0	0	0	25	0	75	0	0	0	18	0
18:15:15	1268	0	2605	0	8	0	21	0	26	0	0	0	25	0	75	0	0	0	18	0
10.13.13	1200	- 0	2003	U		U		- 0	20	- 0	U	U	25	- 0	73	- 0		U	10	



Accu-Traffic Inc. Morning Peak Diagram Specified Period

From: 7:00:00 To: 9:00:00

 Specified Period
 One Hour Peak

 From: 7:00:00
 From: 7:30:00

 To: 9:00:00
 To: 8:30:00

Major Road: Lakeshore Rd E runs W/E

Municipality: Mississauga **Site #:** 1920200003

Intersection: Lakeshore Rd E & Cherriebell Rd

TFR File #: 1

Count date: 28-Nov-19

Weather conditions:

Person counted: Person prepared: Person checked:

** Non-Signalized Intersection **

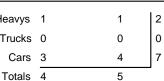
 North Leg Total: 25
 Heavys 1

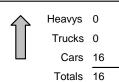
 North Entering: 9
 Trucks 0

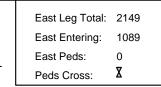
 North Peds: 12
 Cars 3

 Peds Cross: ⋈
 Totals 4

Lakeshore Rd E

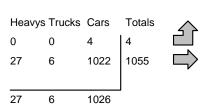






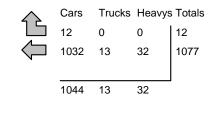




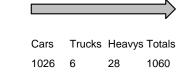




Cherriebell Rd



Lakeshore Rd E



Peds Cross: X
West Peds: 0
West Entering: 1059
West Leg Total: 2140



Accu-Traffic Inc. **Specified Period Afternoon Peak Diagram One Hour Peak** From: 16:00:00 From: 16:45:00 To: 18:00:00 To: 17:45:00 Weather conditions: Municipality: Mississauga Site #: 1920200003 Intersection: Lakeshore Rd E & Cherriebell Rd Person counted: TFR File #: Person prepared: Count date: 28-Nov-19 Person checked: ** Non-Signalized Intersection ** Major Road: Lakeshore Rd E runs W/E North Leg Total: 34 Heavys 0 0 0 Heavys 0 East Leg Total: 2524 0 North Entering: 8 Trucks 0 0 Trucks 0 East Entering: 1601 North Peds: Cars 4 4 8 Cars 26 East Peds: Totals 26 X Peds Cross: Totals 4 4 Peds Cross: Cherriebell Rd Cars Trucks Heavys Totals Heavys Trucks Cars Totals 15 5 0 1570 1590 0 15 1586 1566 15 Lakeshore Rd E 1581 15 Heavys Trucks Cars Totals Lakeshore Rd E 0 11 14 6 899 919 Cars Trucks Heavys Totals 14 910 903 6 14 923 X Peds Cross:

West Peds: 0
West Entering: 930
West Leg Total: 2520



Total Count Diagram

Municipality: Mississauga

Site #: 1920200003

Intersection: Lakeshore Rd E & Cherriebell Rd

Lakeshore Rd E

TFR File #:

Count date: 28-Nov-19

Weather conditions:

Person counted: Person prepared:

Person checked:

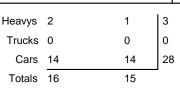
** Non-Signalized Intersection **

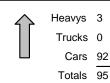
North Leg Total: 126

North Entering: 31

North Peds: 44

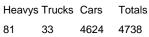
Peds Cross: ▶



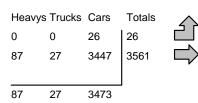


Major Road: Lakeshore Rd E runs W/E



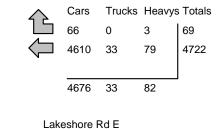








Cherriebell Rd



Cars Trucks Heavys Totals 3461 27 88 3576

Peds Cross:

West Peds: 0

West Entering: 3587

West Leg Total: 8325



Accu-Traffic Inc. Traffic Count Summary

Count Date: 28-Nov-19 Municipality: Mississauga Intersection: Lakeshore Rd E & Cherriebell Rd **North Approach Totals South Approach Totals** North/South Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Total Hour Hour Total Total Grand Grand **Ending** Peds **Ending** Peds Approaches Thru Right Thru Right Total Total 7:00:00 0 0 0 0 0 0 7:00:00 0 0 0 0 0 8:00:00 5 0 6 12 6 8:00:00 0 0 0 0 0 9:00:00 4 0 7 11 11 11 9:00:00 0 0 0 0 0 0 0 0 0 0 0 16:00:00 0 0 0 0 16:00:00 0 0 0 17:00:00 0 0 4 14 17:00:00 0 0 0 4 4 10 0 6 10 18:00:00 18:00:00 Totals: 31 S Totals: 0 16 44 West Approach Totals **East Approach Totals** East/West Includes Cars, Trucks, & Heavys Includes Cars, Trucks, & Heavys Total Hour Hour Total Total Grand **Ending** Peds **Ending** Peds Approaches Right Left Thru Right Left Thru Total Total 7:00:00 0 0 0 7:00:00 0 0 0 0 781 8 789 0 1663 8:00:00 873 874 8:00:00 1 0 0 9:00:00 0 941 23 964 0 1954 9:00:00 8 982 0 990 0 16:00:00 0 0 0 16:00:00 0 0 0 0 0 0 0 17:00:00 0 1449 25 1474 0 2263 17:00:00 9 780 0 789 0 934 18:00:00 1551 13 1564 2498 18:00:00 926 Totals: 0 4722 4791 8378 W Totals: 3561 3587 0 **Calculated Values for Traffic Crossing Major Street** Hours Ending: 9:00 16:00 17:00 18:00 0:00 7:00 8:00 0:00 Crossing Values:



		Passen	ger Cars -	North A	pproach			Truc	cks - Nort	h Approa	ach			He	avys - No	rth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Ri	ght	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Riç	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	6
7:30:00	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	2
7:45:00	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	4
8:00:00	5	3	0	0	1	11	0	0	0	0	0	0	0	0	0	0	0	0	12	0
8:15:00	5	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	1	16	4
8:30:00	6	1	0	0	3	2	0	0	0	0	0	0	1	1	0	0	1	0	20	4
8:45:00	7	11	0	0	5	2	0	0	0	0	0	0	1	0	0	0	1	0	22	2
9:00:00	8	1	0	0	7	2	0	0	0	0	0	0	1	0	0	0	1	0	23	1
9:15:00	8	0	0	0	7	0	0	0	0	0	0	0	1	0	0	0	1	0	23	0
16:00:00	8	0	0	0	7	0	0	0	0	0	0	0	1	0	0	0	1	0	23	0
16:15:00	8	0	0	0	9	2	0	0	0	0	0	0	1	0	0	0	2	1	28	5
16:30:00	8	0	0	0	10	11	0	0	0	0	0	0	1	0	0	0	2	0	29	1
16:45:00	8	0	0	0	10	0	0	0	0	0	0	0	1	0	0	0	2	0	34	5
17:00:00	8	0	0	0	10	0	0	0	0	0	0	0	1	0	0	0	2	0	37	3
17:15:00	9	1	0	0	11	11	0	0	0	0	0	0	1	0	0	0	2	0	38	1
17:30:00	10	1	0	0	11	0	0	0	0	0	0	0	1	0	0	0	2	0	42	4
17:45:00	12	2	0	0	14	3	0	0	0	0	0	0	1	0	0	0	2	0	44	2
18:00:00	14	2	0	0	14	0	0	0	0	0	0	0	1	0	0	0	2	0	44	0
18:15:00	14	0	0	0	14	0	0	0	0	0	0	0	1	0	0	0	2	0	44	0
18:15:15	14	0	0	0	14	0	0	0	0	0	0	0	1	0	0	0	2	0	44	0
					1				l						1					



Count	ount Date: 28-Nov-19 Site #: 1920200003																			
		Passen	ger Cars	- East Ap	proach			Tru	icks - Eas	t Approa	ch			Не	eavys - Ea	ast Appro	pach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	98	98	0	0	0	0	1	1	0	0	0	0	3	3	1	1	0	0
7:30:00	0	0	230	132	1	1	0	0	1	0	0	0	0	0	8	5	1	0	0	0
7:45:00	0	0	453	223	2	1	0	0	3	2	0	0	0	0	13	5	1	0	0	0
8:00:00	0	0	752	299	7	5	0	0	8	5	0	0	0	0	21	8	1	0	0	0
8:15:00	0	0	1010	258	9	2	0	0	10	2	0	0	0	0	29	8	1	0	0	0
8:30:00	0	0	1262	252	13	4	0	0	14	4	0	0	0	0	40	11	1	0	0	0
8:45:00	0	0	1458	196	21	8	0	0	18	4	0	0	0	0	43	3	2	1	0	0
9:00:00	0	0	1655	197	29	8	0	0	19	1	0	0	0	0	48	5	2	0	0	0
9:15:00	0	0	1655	0	29	0	0	0	19	0	0	0	0	0	48	0	2	0	0	0
16:00:00	0	0	1655	0	29	0	0	0	19	0	0	0	0	0	48	0	2	0	0	0
16:15:00	0	0	1966	311	36	7	0	0	21	2	0	0	0	0	54	6	2	0	0	0
16:30:00	0	0	2318	352	43	7	0	0	25	4	0	0	0	0	57	3	3	1	0	0
16:45:00	0	0	2665	347	50	7	0	0	25	0	0	0	0	0	61	4	3	0	0	0
17:00:00	0	0	3081	416	53	3	0	0	27	2	0	0	0	0	63	2	3	0	0	0
17:15:00	0	0	3421	340	59	6	0	0	27	0	0	0	0	0	70	7	3	0	0	0
17:30:00	0	0	3851	430	63	4	0	0	29	2	0	0	0	0	73	3	3	0	0	0
17:45:00	0	0	4231	380	65	2	0	0	30	11	0	0	0	0	76	3	3	0	0	0
18:00:00	0	0	4610	379	66	11	0	0	33	3	0	0	0	0	79	3	3	0	0	0
18:15:00	0	0	4610	0	66	0	0	0	33	0	0	0	0	0	79	0	3	0	0	0
18:15:15	0	0	4610	0	66	0	0	0	33	0	0	0	0	0	79	0	3	0	0	0



		Passeng	jer Cars -	South A	pproach			Truc	ks - Sout	h Approa	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	jht	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00 17:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0		0	0	0		0	0	0	0	0	0	0	0	0	0	0	
18:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10.15.15		U	0	0	"	U	0	U	"	U	0	0	"	- 0	0	0	"	- 0	0	



		Passen	ger Cars	West Ap	proach			Tru	cks - Wes	t Approa	ch			Не	avys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Riç	jht	Le	ft	Th	ru	Ri	ght	West (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	133	133	0	0	0	0	1	1	0	0	0	0	9	9	0	0	0	0
7:30:00	0	0	316	183	0	0	0	0	5	4	0	0	0	0	15	6	0	0	0	0
7:45:00	1	1	559	243	0	0	0	0	5	0	0	0	0	0	19	4	0	0	0	0
8:00:00	1	0	840	281	0	0	0	0	6	1	0	0	0	0	27	8	0	0	0	0
8:15:00	3	2	1113	273	0	0	0	0	8	2	0	0	0	0	33	6	0	0	0	0
8:30:00	4	1	1338	225	0	0	0	0	11	3	0	0	0	0	42	9	0	0	0	0
8:45:00	8	4	1574	236	0	0	0	0	15	4	0	0	0	0	47	5	0	0	0	0
9:00:00	9	1	1782	208	0	0	0	0	16	1	0	0	0	0	57	10	0	0	0	0
9:15:00	9	0	1782	0	0	0	0	0	16	0	0	0	0	0	57	0	0	0	0	0
16:00:00	9	0	1782	0	0	0	0	0	16	0	0	0	0	0	57	0	0	0	0	0
16:15:00	11	2	1974	192	0	0	0	0	17	1	0	0	0	0	62	5	0	0	0	0
16:30:00	13	2	2129	155	0	0	0	0	17	0	0	0	0	0	69	7	0	0	0	0
16:45:00	15	2	2326	197	0	0	0	0	19	2	0	0	0	0	71	2	0	0	0	0
17:00:00	18	3	2541	215	0	0	0	0	19	0	0	0	0	0	75	4	0	0	0	0
17:15:00	19	1	2778	237	0	0	0	0	22	3	0	0	0	0	78	3	0	0	0	0
17:30:00	24	5	3001	223	0	0	0	0	23	1	0	0	0	0	82	4	0	0	0	0
17:45:00	26	2	3225	224	0	0	0	0	25	2	0	0	0	0	85	3	0	0	0	0
18:00:00	26	0	3447	222	0	0	0	0	27	2	0	0	0	0	87	2	0	0	0	0
18:15:00	26	0	3447	0	0	0	0	0	27	0	0	0	0	0	87	0	0	0	0	0
18:15:15	26	0	3447	0	0	0	0	0	27	0	0	0	0	0	87	0	0	0	0	0



Accu-Tra	affic Inc.
Morning Peak Diagram	Specified Period One Hour Peak From: 7:00:00 From: 7:45:00 To: 9:00:00 To: 8:45:00
Municipality: Mississauga Site #: 1920200004 Intersection: Dixie Rd & Access#1 TFR File #: 1 Count date: 28-Nov-19	Weather conditions: Person counted: Person prepared: Person checked:
** Non-Signalized Intersection **	Major Road: Dixie Rd runs N/S
North Leg Total: 1227 Heavys 19 0 19 North Entering: 507 Trucks 12 0 12 North Peds: 0 Cars 474 2 47 Peds Cross: ✓ Totals 505 2 Image: Control of the policy of the po	Trucks 7 Cars 697 Totals 720 East Entering: 2 East Peds: 1 Peds Cross: X Cars Trucks Heavys Totals 2 0 0 2 0 0 0 2 0 0 E Access#1
Cars 474 Ca Trucks 12 Truck Heavys 19 Heavy	ss 7 0 7 South Peds: 0

Comments



Accu-Tr	affic Inc.
Afternoon Peak Diagram	Specified Period One Hour Peak From: 16:00:00 From: 16:45:00 To: 18:00:00 To: 17:45:00
Municipality: Mississauga Site #: 1920200004 Intersection: Dixie Rd & Access#1 TFR File #: 1 Count date: 28-Nov-19	Weather conditions: Person counted: Person prepared: Person checked:
** Non-Signalized Intersection **	Major Road: Dixie Rd runs N/S
Peds Cross: ► Totals 563 3	Heavys 3
Dixie Rd	Cars Trucks Heavys Totals 5 0 0 5
Cars 561 Ca Trucks 1 Trucks 1 Heavys 3 Heav Totals 565 Tota	ks 8 0 8 South Peds: 0 ys 3 0 3 South Entering: 627



Total Count Diagram

Municipality: Mississauga

Site #: 1920200004

Intersection: Dixie Rd & Access#1

TFR File #: 1

Count date: 28-Nov-19

Weather conditions:

Person counted: Person prepared:

Person checked:

** Non-Signalized Intersection **

North Leg Total: 4467 North Entering: 1986

North Peds: 0
Peds Cross: ▶

 Heavys
 36
 0
 36

 Trucks
 30
 0
 30

1912

Totals 1978

Cars

Cars 1916

36

1982

Trucks 30

Heavys

Totals

Major Road: Dixie Rd runs N/S

Heavys 33 East

Trucks 26 Cars 2422

Totals 2481

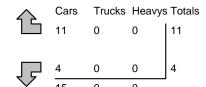
East Leg Total: 25
East Entering: 15

East Peds: 4
Peds Cross: X

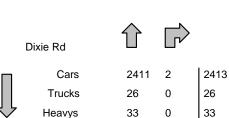




1920



Access#1



2470

2

Cars Trucks Heavys Totals
10 0 0 10

Peds Cross: ► South Peds: 0

South Entering: 2472

South Leg Total: 4454

Comments

Totals



Accu-Traffic Inc. Traffic Count Summary

				- · · · · ·		ount o						
Intersection:	Dixie Ro	l & Acce	ss#1		Count I	Date: 28-Nov-1	9 Munio	cipality: Mi	ssissau	ga		
			ach Tot		•	North/South				ach To		
Hour	Includ	es Cars, T I	rucks, & F	leavys Grand	Total	Total	Hour	Includ	es Cars, T I	rucks, & F	leavys Grand	Total
Ending	Left	Thru	Right	Total	Peds	Approaches	Ending	Left	Thru	Right	Total	Peds
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0
8:00:00	0	392	0	392	0	964	8:00:00	0	572	0	572	0
9:00:00	2	495	0	497	0	1189	9:00:00	0	692	0	692	0
16:00:00 17:00:00	0	0 5 4 2	0	0 5 4 5	0	0 1135	16:00:00 17:00:00	0 0	0 589	0	0 590	0 0
18:00:00	3 3	542 549	0	552	0	1170	18:00:00	0	617	1	618	0
10.00.00	Ü	0.0		002		'''	70.00.00	Ü	077	,	0,0	Ü
Totals:	8	1978	0	1986	0	4458	S Totals:	0	2470	2	2472	0
L	East	t Approa	ach Tota	als		East/West		Wes	t Appro	ach Tot	als	
Hour Ending	Includ	es Cars, i	rucks, & F	Grand	Total Peds	Total	Hour Ending	Includ	es Cars, i	rucks, & F	Grand	Total Peds
	Left	Thru	Right	Total		Approaches	_	Left	Thru	Right	Total	
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0
8:00:00	0	0	2	2	1	2	8:00:00	0	0	0	0	0
9:00:00 16:00:00	0	0	1	0	1	1	9:00:00 16:00:00	0 0	0	0	0	0
17:00:00	0 3	0	0 6	9	0 2	0	17:00:00	0	0	0	0 0	0 0
18:00:00	1	0	2	3	0	9	18:00:00	0	Ö	0		0
10.00.00	•		_				70.00.00	Ü				Ü
Totals:	4	0	11	15	4	15	W Totals:	0	0	0	0	0
			Calc	ulated \		or Traffic Cr	ossing Ma	ajor Str	eet			
Hours E		7:00	8:00	9:00	16:00		17:00	18:00	0:00	0:00		
Crossing	y Values	: <i>0</i>	0	0	0		3	1	0	0		



Count	Date:	28-Nov			192020	0004							1							
		Passen	ger Cars -	North A					cks - Nort	h Approa				He	avys - No	rth Appr			Pedes	trians
Interval Time	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	64	64	0	0	0	0	3	3	0	0	0	0	2	2	0	0	0	0
7:30:00	0	0	142	78	0	0	0	0	5	2	0	0	0	0	3	11	0	0	0	0
7:45:00	0	0	254	112	0	0	0	0	8	3	0	0	0	0	5	2	0	0	0	0
8:00:00	0	0	373	119	0	0	0	0	11	3	0	0	0	0	8	3	0	0	0	0
8:15:00	0	0	489	116	0	0	0	0	15	4	0	0	0	0	14	6	0	0	0	0
8:30:00	2	2	606	117	0	0	0	0	18	3	0	0	0	0	19	5	0	0	0	0
8:45:00	2	0	728	122	0	0	0	0	20	2	0	0	0	0	24	5	0	0	0	0
9:00:00	2	0	833	105	0	0	0	0	23	3	0	0	0	0	31	7	0	0	0	0
9:15:00	2	0	833	0	0	0	0	0	23	0	0	0	0	0	31	0	0	0	0	0
16:00:00	2	0	833	0	0	0	0	0	23	0	0	0	0	0	31	0	0	0	0	0
16:15:00	3	1	970	137	0	0	0	0	25 26	2	0	0	0	0	32	11	0	0	0	0
16:30:00 16:45:00	3	2	1093 1223	123 130	0	0	0	0	28	2	0	0	0	0	33 33	1	0	0	0	0
17:00:00	5 5	0	1366	143	0	0	0	0	28	0	0	0	0	0	35	0 2	0	0	0	0
17:00:00	6	1	1514	148	0	0	0	0	28	0	0	0	0	0	35	0	0	0	0	0
17:13:00	7	1	1644	130	0	0	0	0	28	0	0	0	0	0	36	1	0	0	0	0
17:45:00	8	1	1782	138	0	0	0	0	29	1	0	0	0	0	36	0	0	0	0	0
18:00:00	8	0	1912	130	0	0	0	0	30	1	0	0	0	0	36	0	0	0	0	0
18:15:00	8	0	1912	0	0	0	0	0	30	0	0	0	0	0	36	0	0	0	0	0
18:15:15	8	0	1912	0	0	0	0	0	30	0	0	0	0	0	36	0	0	0	0	0
10.10.10			1012												- 00					



Count	Date.	20-INOV	-19	Site #:	192020	0004														
	 						Tru	cks - Eas	t Approa	ch			Не	eavys - Ea	ast Appro	pach		Pedes	trians	
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
7:45:00	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:00:00	0	0	0	0	2	11	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:15:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:30:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:45:00	0	0	0	0	3	1	0	0	0	0	0	0	0	0	0	0	0	0	2	1
9:00:00	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0
9:15:00	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0
16:00:00	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0
16:15:00	0	0	0	0	4	1	0	0	0	0	0	0	0	0	0	0	0	0	3	1
16:30:00	2	2	0	0	5	1	0	0	0	0	0	0	0	0	0	0	0	0	4	1
16:45:00	2	0	0	0	7	2	0	0	0	0	0	0	0	0	0	0	0	0	4	0
17:00:00 17:15:00	3	0	0	0	9 10	2	0	0	0	0	0	0	0	0	0	0	0	0	4	0
17:15:00	4	1	0	0	10	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0
17:30:00	4	0	0	0	11	1	0	0	0	0	0	0	0	0	0	0	0	0	4	0
18:00:00	4	0	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0
18:15:00	4	0	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0
18:15:15	4	0	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0
10.10.10		0	0	- 0	11	0	U	0		0	U	0		0	U	0		- 0		0



		Passeng	er Cars -	South A	pproach			Truc	ks - Sout	h Approa	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	101	101	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	228	127	0	0	0	0	2	1	0	0	0	0	2	2	0	0	0	0
7:45:00	0	0	381	153	0	0	0	0	2	0	0	0	0	0	3	1	0	0	0	0
8:00:00	0	0	560	179	0	0	0	0	3	1	0	0	0	0	9	6	0	0	0	0
8:15:00	0	0	748	188	0	0	0	0	6	3	0	0	0	0	9	0	0	0	0	0
8:30:00	0	0	914	166	0	0	0	0	7	1	0	0	0	0	13	4	0	0	0	0
8:45:00	0	0	1076	162	0	0	0	0	9	2	0	0	0	0	19	6	0	0	0	0
9:00:00	0	0	1234	158	0	0	0	0	10	1	0	0	0	0	20	1	0	0	0	0
9:15:00	0	0	1234	0	0	0	0	0	10	0	0	0	0	0	20	0	0	0	0	0
16:00:00	0	0	1234	0	0	0	0	0	10	0	0	0	0	0	20	0	0	0	0	0
16:15:00	0	0	1387	153	0	0	0	0	13	3	0	0	0	0	26	6	0	0	0	0
16:30:00	0	0	1518	131	0	0	0	0	16	3	0	0	0	0	27	1	0	0	0	0
16:45:00	0	0	1644	126	0	0	0	0	18	2	0	0	0	0	30	3	0	0	0	0
17:00:00	0	0	1802	158	1	1	0	0	20	2	0	0	0	0	31	1	0	0	0	0
17:15:00	0	0	1944	142	1	0	0	0	22	2	0	0	0	0	33	2	0	0	0	0
17:30:00	0	0	2116	172	2	1	0	0	24	2	0	0	0	0	33	0	0	0	0	0
17:45:00	0	0	2258	142	2	0	0	0	26	2	0	0	0	0	33	0	0	0	0	0
18:00:00	0	0	2411	153	2	0	0	0	26	0	0	0	0	0	33	0	0	0	0	0
18:15:00	0	0	2411	0	2	0	0	0	26	0	0	0	0	0	33	0	0	0	0	0
18:15:15	0	0	2411	0	2	0	0	0	26	0	0	0	0	0	33	0	0	0	0	0



		Passen	ger Cars -	West A	proach			Tru	cks - Wes	t Approa	ich			Не	eavys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	Le	ft	Th	ru	Rig	ght	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45:00	0	0	0	0	0	0	0	0	Ö	0	0	0	0	0	0	0	0	0	0	0
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	0	0	0	0	Ö	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	ō	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00	0	0	0	0	ō	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:15	0	0	0	0	Ö	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10.10.10								- 0								- 0				



Accu-Traffic Inc. **Morning Peak Diagram Specified Period One Hour Peak** From: 7:00:00 From: 7:30:00 To: 9:00:00 To: 8:30:00 Weather conditions: Municipality: Mississauga 1920200005 Intersection: Lakeshore Rd E & Access#2 Person counted: TFR File #: Person prepared: Count date: 28-Nov-19 Person checked: ** Non-Signalized Intersection ** Major Road: Lakeshore Rd E runs W/E North Leg Total: 3 Heavys 0 0 0 Heavys 0 East Leg Total: 2142 0 North Entering: 2 Trucks 0 0 Trucks 0 East Entering: 1079 Cars 1 1 2 Cars 1 East Peds: X Totals 1 Totals 1 Peds Cross: Access#2 Heavys Trucks Cars Totals

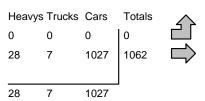


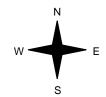
Site #:

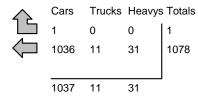
North Peds:

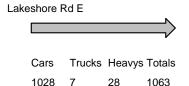
Peds Cross:











X Peds Cross: West Peds: West Entering: 1062 West Leg Total: 2141



Accu-Traffic Inc. **Specified Period One Hour Peak Afternoon Peak Diagram** From: 16:00:00 From: 16:45:00 To: 18:00:00 To: 17:45:00 Weather conditions: Municipality: Mississauga Site #: 1920200005 Intersection: Lakeshore Rd E & Access#2 Person counted: TFR File #: Person prepared: Count date: 28-Nov-19 Person checked: ** Non-Signalized Intersection ** Major Road: Lakeshore Rd E runs W/E North Leg Total: 2 Heavys 0 0 0 Heavys 0 East Leg Total: 2513 0 North Entering: 0 Trucks 0 0 Trucks 0 East Entering: 1585 North Peds: Cars 0 0 0 Cars 2 East Peds: Totals 2 X Peds Cross: Totals 0 0 Peds Cross: Access#2 Cars Trucks Heavys Totals Heavys Trucks Cars Totals 15 5 0 2 1563 1583 0 1583 15 1563 Lakeshore Rd E 1565 15 Heavys Trucks Cars Totals Lakeshore Rd E 0 14 5 909 928 Cars Trucks Heavys Totals 14 909 909 14 928 X Peds Cross: West Peds: West Entering: 928 West Leg Total: 2511 **Comments**



Total Count Diagram

Municipality: Mississauga

Site #: 1920200005

Intersection: Lakeshore Rd E & Access#2

TFR File #:

Count date: 28-Nov-19

Weather conditions:

Person counted: Person prepared:

Person checked:

** Non-Signalized Intersection **

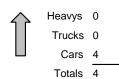
North Leg Total: 11

North Entering: 7

North Peds: 23

Peds Cross: ▶





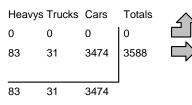
Major Road: Lakeshore Rd E runs W/E





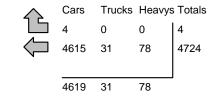








Access#2





Cars Trucks Heavys Totals 3478 31 83 3592

Peds Cross: X
West Peds: 0
West Entering: 3588
West Leg Total: 8315



Accu-Traffic Inc. Traffic Count Summary

Intersection:	Lakesho	re Rd E	& Acces	ss#2	Count [Date: 28-Nov-1	9	Munio	ipality: Mi	ssissaug	na		
			ach Tot							h Appro	-	tals	
Hour			rucks, & F		Total	North/South	Hou	ır	Include	es Cars, T	rucks, & F	leavys	Total
Ending				Grand	Peds	Total Approaches	Endi					Grand	Peds
7 00 00	Left	Thru	Right	Total		- ' '		-	Left	Thru	Right	Total	
7:00:00	0	0	0	0	0	0	7:00.		0	0	0	0	0
8:00:00	0 1	0	0	0 2	8 5	0 2	8:00.		0 0	0	0 0	0	0 0
9:00:00 16:00:00	0	0	0	0	0	0	9:00. 16:00		0	0 0	0	0 0	0
17:00:00	3	Ö	1	4	6	4	17:00		0	0	o	0	0
18:00:00	0	Ö	1	1	4	1	18:00		0	Ö	0	Ö	o O
10.00.00	Ū		·	,	•	,	70.00		ŭ	J			ŭ
Totals:	4	0	3	7	23	7	S Tot	als:	0	0	0	0	0
			ach Tota			East/West				t Appro			
Hour Ending	Includ	es Cars, i	rucks, & F	Grand	Total Peds	Total	Hou Endii		includ	es Cars, T	rucks, & F	Grand	Total Peds
Litaling	Left	Thru	Right	Total	reus	Approaches	Liluii	iig	Left	Thru	Right	Total	reus
7:00:00	0	0	0	0	0	0	7:00.		0	0	0	0	0
8:00:00	0	779	0	779	0	1655	8:00.		0	876	0	876	0
9:00:00	0	944	1	945	0	1933	9:00.		0	988	0	988	0
16:00:00	0	0 1 44 9	0	0	0	0 2241	16:00		0	0 792	0	0 700	0
17:00:00 18:00:00	0 0	1552	0	1449 1555	0 0	2441	17:00 18:00		0 0	792 932	0 0	792 932	0 0
10.00.00	U	1002	3	1555	U	2407	10.00	,,,,,,	U	932	U	932	U
		l											
Totals:	0	4724	4	4728	0		W Tot		0	3588	0	3588	0
			Calc	ulated \	/alues f	8316 or Traffic Cr	ossin	g Ma	ajor Stre	eet		3588	0
Hours E	nding:	7:00	Calc 8:00	ulated \ 9:00	/alues f 16:00		ossin 17:0	g M a	ajor Stre 18:00	eet 0:00	0:00	3588	0
	nding:	7:00	Calc	ulated \	/alues f		ossin	g M a	ajor Stre	eet		3588	0



		Passen	ger Cars -	North A	pproach			Truc	ks - Nort	h Approa	ach			He	avys - No	rth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	jht	Le	ft	Th	ru	Riç	jht	Le	ft	Th	ru	Ri	ght	North	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Inci
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	3
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	1
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	3
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	1
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	2
8:30:00	1	1	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	12	2
8:45:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	13	1
9:00:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0
9:15:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0
16:00:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	13	0
16:15:00	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	15	2
16:30:00	2	1	0	0	2	1	0	0	0	0	0	0	0	0	0	0	0	0	16	1
16:45:00	4	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	19	3
17:00:00	4	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	19	0
17:15:00	4	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	20	1
17:30:00	4	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	22	2
17:45:00	4	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	23	1
18:00:00	4	0	0	0	3	1	0	0	0	0	0	0	0	0	0	0	0	0	23	0
18:15:00	4	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	23	0
18:15:15	4	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	23	0



		Passen	ger Cars	- East Ap	proach			Tru	cks - Eas	t Approa	ch			He	eavys - Ea	ast Appro	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Ri	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	98	98	0	0	0	0	1	1	0	0	0	0	3	3	0	0	0	0
7:30:00	0	0	226	128	0	0	0	0	11	0	0	0	0	0	8	5	0	0	0	0
7:45:00	0	0	449	223	0	0	0	0	3	2	0	0	0	0	12	4	0	0	0	0
8:00:00	0	0	753	304	0	0	0	0	6	3	0	0	0	0	20	8	0	0	0	0
8:15:00	0	0	1007	254	0	0	0	0	8	2	0	0	0	0	28	8	0	0	0	0
8:30:00	0	0	1262	255	1	1	0	0	12	4	0	0	0	0	39	11	0	0	0	0
8:45:00	0	0	1461	199	1	0	0	0	16	4	0	0	0	0	42	3	0	0	0	0
9:00:00	0	0	1659	198	1	0	0	0	17	1	0	0	0	0	47	5	0	0	0	0
9:15:00	0	0	1659	0	1	0	0	0	17	0	0	0	0	0	47	0	0	0	0	0
16:00:00	0	0	1659	0	1	0	0	0	17	0	0	0	0	0	47	0	0	0	0	0
16:15:00	0	0	1971	312	1	0	0	0	19	2	0	0	0	0	53	6	0	0	0	0
16:30:00	0	0	2327	356	1	0	0	0	23	4	0	0	0	0	56	3	0	0	0	0
16:45:00	0	0	2671	344	1	0	0	0	23	0	0	0	0	0	60	4	0	0	0	0
17:00:00	0	0	3085	414	1	0	0	0	25	2	0	0	0	0	62	2	0	0	0	0
17:15:00	0	0	3424	339	2	1	0	0	25	0	0	0	0	0	69	7	0	0	0	0
17:30:00	0	0	3854	430	2	0	0	0	27	2	0	0	0	0	72	3	0	0	0	0
17:45:00	0	0	4234	380	3	11	0	0	28	1	0	0	0	0	75	3	0	0	0	0
18:00:00	0	0	4615	381	4	1	0	0	31	3	0	0	0	0	78	3	0	0	0	0
18:15:00	0	0	4615	0	4	0	0	0	31	0	0	0	0	0	78	0	0	0	0	0
18:15:15	0	0	4615	0	4	0	0	0	31	0	0	0	0	0	78	0	0	0	0	0



		Passeng	ger Cars -	South A	pproach			Truc	ks - Sout	h Approa	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	Le	ft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



		Passen	ger Cars	- West Ap	proach			Tru	cks - Wes	t Approa	ch			Не	avys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Ri	ght	West (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	132	132	0	0	0	0	2	2	0	0	0	0	8	8	0	0	0	0
7:30:00	0	0	317	185	0	0	0	0	6	4	0	0	0	0	13	5	0	0	0	0
7:45:00	0	0	557	240	0	0	0	0	6	0	0	0	0	0	17	4	0	0	0	0
8:00:00	0	0	843	286	0	0	0	0	8	2	0	0	0	0	25	8	0	0	0	0
8:15:00	0	0	1117	274	0	0	0	0	10	2	0	0	0	0	31	6	0	0	0	0
8:30:00	0	0	1344	227	0	0	0	0	13	3	0	0	0	0	41	10	0	0	0	0
8:45:00	0	0	1584	240	0	0	0	0	17	4	0	0	0	0	45	4	0	0	0	0
9:00:00	0	0	1791	207	0	0	0	0	19	2	0	0	0	0	54	9	0	0	0	0
9:15:00	0	0	1791	0	0	0	0	0	19	0	0	0	0	0	54	0	0	0	0	0
16:00:00	0	0	1791	0	0	0	0	0	19	0	0	0	0	0	54	0	0	0	0	0
16:15:00	0	0	1984	193	0	0	0	0	21	2	0	0	0	0	58	4	0	0	0	0
16:30:00	0	0	2146	162	0	0	0	0	21	0	0	0	0	0	65	7	0	0	0	0
16:45:00	0	0	2343	197	0	0	0	0	23	2	0	0	0	0	67	2	0	0	0	0
17:00:00	0	0	2562	219	0	0	0	0	23	0	0	0	0	0	71	4	0	0	0	0
17:15:00	0	0	2798	236	0	0	0	0	25	2	0	0	0	0	74	3	0	0	0	0
17:30:00	0	0	3026	228	0	0	0	0	26	1	0	0	0	0	78	4	0	0	0	0
17:45:00	0	0	3252	226	0	0	0	0	28	2	0	0	0	0	81	3	0	0	0	0
18:00:00	0	0	3474	222	0	0	0	0	31	3	0	0	0	0	83	2	0	0	0	0
18:15:00	0	0	3474	0	0	0	0	0	31	0	0	0	0	0	83	0	0	0	0	0
18:15:15	0	0	3474	0	0	0	0	0	31	0	0	0	0	0	83	0	0	0	0	0



Accu-Traffic Inc. **Morning Peak Diagram Specified Period One Hour Peak** From: 7:00:00 From: 8:00:00 To: To: 9:00:00 9:00:00 Weather conditions: Municipality: Mississauga Site #: 1920200006 Intersection: Cherriebell Rd & Access#3 Person counted: TFR File #: Person prepared: Count date: 28-Nov-19 Person checked: ** Non-Signalized Intersection ** Major Road: Cherriebell Rd runs N/S North Leg Total: 38 Heavys 0 1 Heavys 1 0 North Entering: 9 Trucks 0 0 Trucks 0 North Peds: Cars 0 Cars 28 Peds Cross: Totals 0 Totals 29 Cherriebell Rd Heavys Trucks Cars Totals 0 2 2 Access#3 Heavys Trucks Cars Totals 0 0 2 Cherriebell Rd X Cars 2 Peds Cross: Cars 9 28 30 Peds Cross: M 0 West Peds: Trucks 0 Trucks 0 0 South Peds: 2

Comments

Heavys 0

Totals 2

1

29

South Entering: 31

South Leg Total: 42

West Entering: 2

West Leg Total: 4

Heavys

Totals 11



Accu-Traffic Inc. **Specified Period One Hour Peak Afternoon Peak Diagram** From: 16:00:00 From: 16:15:00 To: 18:00:00 To: 17:15:00 Weather conditions: Municipality: Mississauga Site #: 1920200006 Intersection: Cherriebell Rd & Access#3 Person counted: TFR File #: Person prepared: Count date: 28-Nov-19 Person checked: ** Non-Signalized Intersection ** Major Road: Cherriebell Rd runs N/S North Leg Total: 32 Heavys 0 0 0 Heavys 1 0 North Entering: 3 Trucks 0 0 Trucks 0 North Peds: Cars 1 Cars 28 Peds Cross: Totals 1 Totals 29 Cherriebell Rd Heavys Trucks Cars Totals 0 5 5 Access#3 Heavys Trucks Cars Totals 0 0 Cherriebell Rd X Cars 4 Peds Cross: Cars 3 28 32 Peds Cross:

Comments

Trucks 0

Heavys 0

Totals 4

0

1

29

West Peds:

West Entering: 1

West Leg Total: 6

Trucks 0

Heavys 0

Totals 3

0

South Peds:

South Entering: 33

South Leg Total: 36

0



Total Count Diagram

Municipality: Mississauga Weather conditions:

Site #: 1920200006

Intersection: Cherriebell Rd & Access#3

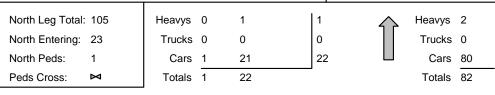
TFR File #: 1

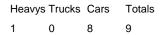
Count date: 28-Nov-19

Person counted: Person prepared: Person checked:

** Non-Signalized Intersection **

Major Road: Cherriebell Rd runs N/S

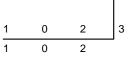






Cherriebell Rd



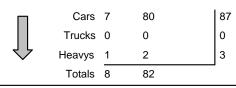






	Peds Cross:	X
	West Peds:	2
	West Entering:	3
	West Leg Total:	12
_		





Peds Cross: South Peds: 2
South Entering: 90
South Leg Total: 115



Accu-Traffic Inc. Traffic Count Summary

Intersection:	Cherrieh	ell Rd &	Access	#3	Count D	Date: 28-Nov-19	g Muni	cipality: Mi	ssissau	na		
			ach Tot				<u> </u>			oach To	tals	
Hour			rucks, & H		Total	North/South Total	Hour			rucks, & F		Total
Ending	Left			Grand	Peds	Approaches	Ending				Grand	Peds
7:00:00	0	Thru O	Right 0	Total 0	0	0	7:00:00	Left O	Thru O	Right 0	Total 0	0
8:00:00	0	6	o o	6	0	15	8:00:00	2	7	Ö	9	0
9:00:00	Ö	9	Ö	9	1	40	9:00:00	2	29	Ö	31	2
16:00:00	0	0	0	0	0	0	16:00:00	0	0	0	0	0
17:00:00	0	2	1	3	0	33	17:00:00	2	28	0	30	0
18:00:00	0	5	0	5	0	25	18:00:00	2	18	0	20	0
Totals:	0	22	1	23	1	113	S Totals:	8	82	0	90	2
Totals.			ach Tota		'		O Totals.			ach Tot		
Hour			rucks, & H	leavys	Total	East/West Total	Hour	Includ	es Cars, T	rucks, & F	leavys	Total
Ending	Left	Thru	Right	Grand Total	Peds	Approaches	Ending	Left	Thru	Right	Grand Total	Peds
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0
8:00:00	0	0	0	0	0	0	8:00:00	0	0	0	0	1
9:00:00	0	0	0	0	0	2	9:00:00	0	0	2	2	0
16:00:00 17:00:00	0 0	0 0	0	0 0	0 0	0 0	16:00:00 17:00:00	0 0	0	0	0	0 0
18:00:00	0	0	0	0	0	1	18:00:00	0	0	1	1	1
70.00.00						,	70.00.00			'	,	,
1	0	0	0	0	0	3	W Totals:	0	0	3	3	2
Totals:			_									
Totals:			Calc	ulated \	vaiues t	Or Traffic Cr	ossina ivi	aior Str	eet			
	ndina:	7:00				or Traffic Cr	_	-		0:00		
Totals: Hours E		7:00 : 0	8:00 0	9:00 3	16:00 0	or Traffic Cr	0551ng W 17:00 0	18:00 0	0:00 0	0:00 0		



Time Cum Incr Cum Incr Cum Incr Cum Incr Cum Incr Cum	Right n Incr	Left	eavys - North Ap Thru		Pedestrians
Time Cum Incr Cum Incr Cum Incr Cum Incr Cum Incr Cum	Ť		Thru		1
Cum Incr Cum Incr Cum Incr Cum Incr Cum Incr Cum	n Incr	l l		Right	North Cross
		Cum Incr	Cum Incr	Cum Incr	Cum Inc
7:00:00 0 0 0 0 0 0 0 0 0	0	0 0	0 0	0 0	0 0
7:15:00 0 0 1 1 0 0 0 0 0 0	0	0 0	0 0	0 0	0 0
7:30:00 0 0 2 1 0 0 0 0 0 0	0	0 0	0 0	0 0	0 0
7:45:00 0 0 2 0 0 0 0 0 0 0	0	0 0	0 0	0 0	0 0
8:00:00 0 0 6 4 0 0 0 0 0 0	0	0 0	0 0	0 0	0 0
8:15:00 0 0 6 0 0 0 0 0 0 0	0	0 0	1 1	0 0	0 0
8:30:00 0 0 9 3 0 0 0 0 0 0	0	0 0	1 0	0 0	0 0
8:45:00 0 0 11 2 0 0 0 0 0 0	0	0 0	1 0	0 0	1 1
9:00:00 0 0 14 3 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
9:15:00 0 0 14 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
16:00:00 0 0 14 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
16:15:00 0 0 15 1 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
16:30:00 0 0 16 1 1 1 0 0 0 0	0	0 0	1 0	0 0	1 0
16:45:00 0 0 16 0 1 0 0 0 0 0 0 17:00:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
17:00:00 0 0 16 0 1 0 0 0 0 0 0 17:15:00 0 0 17 1 1 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
17:30:00 0 0 17 1 1 0 0 0 0 0 0 0 1 1 1 1 1 1	0	0 0	1 0	0 0	1 0
17:45:00 0 0 20 3 1 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
17:40:00 0 0 20 3 1 0 0 0 0 0 0 18:00:00 0 0 0 1 1 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
18:15:00 0 0 21 0 1 0 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
18:15:15 0 0 21 0 1 0 0 0 0 0 0 0	0	0 0	1 0	0 0	1 0
10.15.15		0 0			1 0



Accu-Traffic Inc.

Count Date: 28-Nov-19 Site #: 1920200006

		Passen	ger Cars	- East Ap	proach			Tru	cks - Eas	t Approa	ch			H	eavys - Ea	ast Appro	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Riç	ght	Le	ft	Th	ru	Rig	ght	East (Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:15:15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
			1				[I	



Accu-Traffic Inc.

Count Date: 28-Nov-19 Site #: 1920200006

Count	unt Date: 28-Nov-19															1				
		Passeng	er Cars -	South A	pproach			Truc	ks - Sout	h Appro	ach			He	avys - So	uth Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Rig	ght	Le	eft	Th	ru	Rig	ght	South	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0
7:30:00	0	0	1	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
7:45:00	1	1	2	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
8:00:00	1	0	7	5	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
8:15:00	1	0	12	5	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2	2
8:30:00	1	0	18	6	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2	0
8:45:00	3	2	26	8	0	0	0	0	0	0	0	0	1	0	1	1	0	0	2	0
9:00:00	3	0	35	9	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	0
9:15:00	3	0	35	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	0
16:00:00	3	0	35	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	0
16:15:00	3	0	40	5	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	0
16:30:00 16:45:00	5	1 1	48 56	<u>8</u> 8	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
17:00:00	5	0	62	6	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
17:15:00	7	2	68	6	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
17:30:00	7	0	76	8	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
17:45:00	7	0	78	2	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
18:00:00	7	0	80	2	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
18:15:00	7	0	80	0	0	0	0	0	0	0	0	0	1	0	2	0	0	0	2	0
18:15:15	7	0	80	0	0	0	0	0	ō	0	0	0	1	0	2	0	ō	0	2	0
															_				_	



Accu-Traffic Inc.

Count Date: 28-Nov-19 Site #: 1920200006

		Passen	ger Cars -	- West Ap	proach			Tru	cks - Wes	t Approa	ch			He	avys - W	est Appr	oach		Pedes	trians
Interval	Le	eft	Th	ru	Riç	ght	Le	eft	Th	ru	Riç	jht	Le	ft	Th	ru	Ri	ght	West	Cross
Time	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Inc
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
7:30:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
7:45:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:15:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
8:30:00	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	1	1	1	0
8:45:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
9:00:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
9:15:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
16:00:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
16:15:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
16:30:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
16:45:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
17:00:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0
17:15:00	0	0	0	0	2	1	0	0	0	0	0	0	0	0	0	0	1	0	1	0
17:30:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	2	1
17:45:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	2	0
18:00:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	2	0
18:15:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	2	0
18:15:15	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	2	0



Turning Movement Count Location Name: DIXIE RD & ST JAMES AVE Date: Thu, Mar 08, 2018 Deployment Lead: Theo Daglis

BA Group 45 St. Clair Avenue West, Suite 300 Toronto ON, CANADA, M4V 1K9

Start Time			N App DIXI	roach E RD				S App DIXI	roach E RD				W App			Int. Total (15 min)	Int. Tota (1 hr)
Start Time	Right N:W	Thru N:S	U-Turn N:N	Peds N:	Approach Total	Thru S:N	Left S:W	U-Turn S:S	Peds S:	Approach Total	Right W:S	Left W:N	U-Turn W:W	Peds W:	Approach Total		
07:00:00	6	53	0	0	59	69	0	0	0	69	0	9	0	0	9	137	
07:15:00	1	59	0	0	60	109	0	0	0	109	0	15	0	0	15	184	
07:30:00	3	56	0	0	59	105	0	0	0	105	2	13	0	0	15	179	
07:45:00	4	69	0	0	73	141	0	0	0	141	4	13	0	0	17	231	731
08:00:00	5	87	0	0	92	166	2	0	0	168	0	12	0	0	12	272	866
08:15:00	2	78	0	0	80	137	0	0	0	137	0	15	0	0	15	232	914
08:30:00	6	79	0	0	85	128	2	1	0	131	1	10	0	0	11	227	962
08:45:00 ***BREAK*	6	59	0	0	65	120	1	0	0	121	1	11	0	0	12	198	929
16:00:00	9	99	0	0	108	122	2	0	0	124	1	7	0	0	8	240	
16:15:00	6	81	0	0	87	91	1	0	0	92	1	4	0	1	5	184	
16:30:00	10	105	0	0	115	110	0	0	0	110	0	8	0	0	8	233	
16:45:00	9	120	0	0	129	122	1	0	0	123	2	4	0	0	6	258	915
17:00:00	14	111	0	0	125	113	0	0	0	113	1	5	0	0	6	244	919
17:15:00	11	119	0	0	130	94	0	0	0	94	1	10	0	0	11	235	970
17:30:00	12	108	0	0	120	105	1	0	0	106	0	5	0	1	5	231	968
17:45:00	22	114	0	1	136	99	1	0	0	100	0	8	0	0	8	244	954
Grand Total	126	1397	0	1	1523	1831	11	1	0	1843	14	149	0	2	163	3529	-
Approach%	8.3%	91.7%	0%		-	99.3%	0.6%	0.1%		-	8.6%	91.4%	0%		-	-	-
Totals %	3.6%	39.6%	0%		43.2%	51.9%	0.3%	0%		52.2%	0.4%	4.2%	0%		4.6%	-	-
Heavy	1	48	0		-	41	4	0		-	1	1	0		-	-	-
Heavy %	0.8%	3.4%	0%		-	2.2%	36.4%	0%		-	7.1%	0.7%	0%		-	-	-
Bicycles	-	-	-		-	-	-	-		-	-	-	-		-	-	-
Bicycle %	-	-	-		-	-	-	-			-	-	-				-



Turning Movement Count
Location Name: DIXIE RD & ST JAMES AVE
Date: Thu, Mar 08, 2018 Deployment Lead: Theo Daglis

BA Group 45 St. Clair Avenue West, Suite 300 Toronto ON, CANADA, M4V 1K9

				Pe	eak Hour: 07:4	5 AM -	08:45	AM V	Veath	er: Overcast (-	3.4 °C)	1				
Start Time				roach E RD					roach E RD				W App			Int. Tota (15 min)
	Right	Thru	U-Turn	Peds	Approach Total	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	
07:45:00	4	69	0	0	73	141	0	0	0	141	4	13	0	0	17	231
08:00:00	5	87	0	0	92	166	2	0	0	168	0	12	0	0	12	272
08:15:00	2	78	0	0	80	137	0	0	0	137	0	15	0	0	15	232
08:30:00	6	79	0	0	85	128	2	1	0	131	1	10	0	0	11	227
Grand Total	17	313	0	0	330	572	4	1	0	577	5	50	0	0	55	962
Approach%	5.2%	94.8%	0%		-	99.1%	0.7%	0.2%		-	9.1%	90.9%	0%		-	-
Totals %	1.8%	32.5%	0%		34.3%	59.5%	0.4%	0.1%		60%	0.5%	5.2%	0%		5.7%	-
PHF	0.71	0.9	0		0.9	0.86	0.5	0.25		0.86	0.31	0.83	0		0.81	-
Heavy	0	24	0		24	15	3	0		18	0	1	0		1	
Heavy %	0%	7.7%	0%		7.3%	2.6%	75%	0%		3.1%	0%	2%	0%		1.8%	-
Lights	17	289	0		306	557	1	1		559	5	49	0		54	
Lights %	100%	92.3%	0%		92.7%	97.4%	25%	100%		96.9%	100%	98%	0%		98.2%	-
Single-Unit Trucks	0	19	0		19	7	0	0		7	0	0	0		0	-
Single-Unit Trucks %	0%	6.1%	0%		5.8%	1.2%	0%	0%		1.2%	0%	0%	0%		0%	-
Buses	0	4	0		4	3	3	0		6	0	1	0		1	-
Buses %	0%	1.3%	0%		1.2%	0.5%	75%	0%		1%	0%	2%	0%		1.8%	-
Articulated Trucks	0	1	0		1	5	0	0		5	0	0	0		0	-
Articulated Trucks %	0%	0.3%	0%		0.3%	0.9%	0%	0%		0.9%	0%	0%	0%		0%	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Pedestrians%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-



Turning Movement Count Location Name: DIXIE RD & ST JAMES AVE Date: Thu, Mar 08, 2018 Deployment Lead: Theo Daglis

BA Group 45 St. Clair Avenue West, Suite 300 Toronto ON, CANADA, M4V 1K9

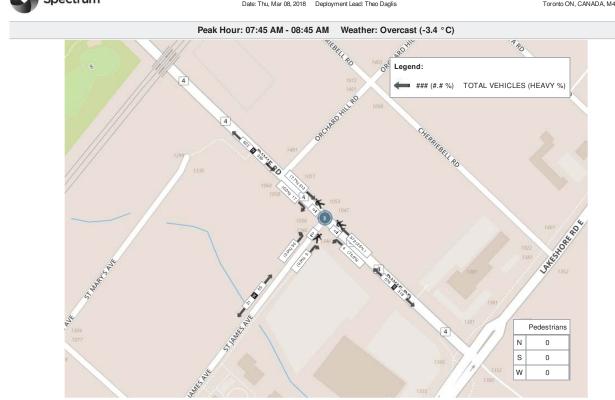
					Peak Hour: 04	30 PM	- 05:3	0 PM	Weat	her: Snow (-0.4	1 °C)					
Start Time				roach E RD				S App	roach E RD				W App			Int. Total (15 min)
	Right	Thru	U-Turn	Peds	Approach Total	Thru	Left	U-Turn	Peds	Approach Total	Right	Left	U-Turn	Peds	Approach Total	
16:30:00	10	105	0	0	115	110	0	0	0	110	0	8	0	0	8	233
16:45:00	9	120	0	0	129	122	1	0	0	123	2	4	0	0	6	258
17:00:00	14	111	0	0	125	113	0	0	0	113	1	5	0	0	6	244
17:15:00	11	119	0	0	130	94	0	0	0	94	1	10	0	0	11	235
Grand Total	44	455	0	0	499	439	1	0	0	440	4	27	0	0	31	970
Approach%	8.8%	91.2%	0%		-	99.8%	0.2%	0%		-	12.9%	87.1%	0%		-	-
Totals %	4.5%	46.9%	0%		51.4%	45.3%	0.1%	0%		45.4%	0.4%	2.8%	0%		3.2%	-
PHF	0.79	0.95	0		0.96	0.9	0.25	0		0.89	0.5	0.68	0		0.7	-
Heavy	0	3	0		3	7	0	0		7	0	0	0		0	-
Heavy %	0%	0.7%	0%		0.6%	1.6%	0%	0%		1.6%	0%	0%	0%		0%	-
Lights	44	452	0		496	432	1	0		433	4	27	0		31	-
Lights %	100%	99.3%	0%		99.4%	98.4%	100%	0%		98.4%	100%	100%	0%		100%	-
Single-Unit Trucks	0	2	0		2	3	0	0		3	0	0	0		0	-
Single-Unit Trucks %	0%	0.4%	0%		0.4%	0.7%	0%	0%		0.7%	0%	0%	0%		0%	-
Buses	0	1	0		1	1	0	0		1	0	0	0		0	-
Buses %	0%	0.2%	0%		0.2%	0.2%	0%	0%		0.2%	0%	0%	0%		0%	-
Articulated Trucks	0	0	0		0	3	0	0		3	0	0	0		0	-
Articulated Trucks %	0%	0%	0%		0%	0.7%	0%	0%		0.7%	0%	0%	0%		0%	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
Pedestrians%	-	-	-	0%		-	-	-	0%		-	-	-	0%		-

 Turning Movement Count
 Page 3 of 6
 BAC18L5R



Turning Movement Count
Location Name: DIXIE RD & ST JAMES AVE
Date: Thu, Mar 08, 2018 Deployment Lead: Theo Daglis

BA Group 45 St. Clair Avenue West, Suite 300 Toronto ON, CANADA, M4V 1K9





Turning Movement Count
Location Name: DIXIE RD & ST JAMES AVE
Date: Thu, Mar 08, 2018 Deployment Lead: Theo Daglis

BA Group 45 St. Clair Avenue West, Suite 300 Toronto ON, CANADA, M4V 1K9





Turning Movement Count
Location Name: FERGUS AVE & LAKESHORE RD E
Date: Thu, Mar 08, 2018 Deployment Lead: Theo Daglis

BA Group 45 St. Clair Avenue West, Suite 300 Toronto ON, CANADA, M4V 1K9

-																										
					Tu	irning Mov	emen	ıt Cou	int (4	. FER	GUS	AVE & LA	KESH	ORE	RD E	E) Cu	stID:	: SPEC_INS	TRUC	Mic	oID:	501039)			
Start Time				N Approa			_			E Approac			_			S Approa			_			W Approa			Int. Total (15 min)	Int. Tota (1 hr)
tart III	Right N:W	Thru N:S	Left N:E	U-Turn N:N	Peds N:	Approach Total	Right E:N	Thru E:W	Left E:S	U-Turn E:E	Peds E:	Approach Total	Right S:E	Thru S:N	Left S:W	U-Turn S:S	Peds S:	Approach Total	Right W:S	Thru W:E	Left W:N	U-Turn W:W	Peds W:	Approach Total		
07:00:00	7	0	2	0	0	9	3	101	6	0	0	110	0	0	3	0	0	3	4	177	2	0	0	183	305	
07:15:00	3	0	3	0	2	6	0	114	6	0	0	120	1	0	2	0	0	3	5	243	3	0	0	251	380	
07:30:00	9	0	2	0	2	11	1	109	3	0	0	113	1	0	2	0	0	3	13	264	2	0	0	279	406	
07:45:00	12	0	0	0	3	12	1	140	12	0	0	153	4	0	3	0	0	7	9	314	4	0	0	327	499	1590
08:00:00	17	0	2	0	2	19	1	146	14	0	0	161	3	0	3	0	0	6	9	288	3	0	0	300	486	1771
08:15:00	10	0	3	0	0	13	1	197	9	0	0	207	5	0	6	0	0	11	7	280	4	0	1	291	522	1913
08:30:00	12	0	1	0	1	13	4	150	11	0	0	165	5	0	7	0	0	12	5	248	6	0	0	259	449	1956
08:45:00	19	0	4	0	0	23	1	136	8	0	0	145	8	0	8	0	1	16	10	241	5	0	0	256	440	189
***BREAK	···																									
16:00:00	3	0	0	0	1	3	3	264	0	0	1	267	9	0	9	0	0	18	6	182	11	0	1	199	487	
16:15:00	1	0	0	0	4	1	6	298	4	0	0	308	9	0	5	0	0	14	2	129	5	0	0	136	459	
16:30:00	11	0	2	0	3	13	6	310	0	0	0	316	10	0	6	0	0	16	2	132	11	0	0	145	490	
16:45:00	6	0	1	0	1	7	2	321	0	1	0	324	8	0	1	0	0	9	4	176	4	0	1	184	524	196
17:00:00	10	0	3	0	0	13	4	353	1	0	0	358	4	0	8	0	0	12	0	165	9	0	0	174	557	203
17:15:00	10	0	3	0	3	13	4	314	1	0	0	319	4	0	3	0	0	7	1	164	6	0	1	171	510	208
17:30:00	6	0	1	0	2	7	4	328	1	0	0	333	0	0	2	0	0	2	2	161	7	0	0	170	512	210
17:45:00	8	0	3	0	3	11	3	252	1	0	0	256	1	0	2	0	0	3	0	148	6	0	0	154	424	200
rand Total	144	0	30	0	27	174	44	3533	77	1	1	3655	72	0	70	0	1	142	79	3312	88	0	4	3479	7450	-
Approach%	82.8%	0%	17.2%	0%		-	1.2%	96.7%	2.1%	0%		-	50.7%	0%	49.3%	0%		-	2.3%	95.2%	2.5%	0%		-	-	-
Totals %	1.9%	0%	0.4%	0%		2.3%	0.6%	47.4%	1%	0%		49.1%	1%	0%	0.9%	0%		1.9%	1.1%	44.5%	1.2%	0%		46.7%	-	-
Heavy	8	0	0	0		-	1	103	31	0		-	27	0	26	0		-	28	100	3	0		-	-	-
Heavy %	5.6%	0%	0%	0%		-	2.3%	2.9%	40.3%	0%		-	37.5%	0%	37.1%	0%		-	35.4%	3%	3.4%	0%		-	-	-
Bicycles	0	0	0	0		-	0	0	0	0		-	0	0	0	0		-	0	1	0	0		-	-	-
Bicycle %	0%	0%	0%	0%		-	0%	0%	0%	0%		-	0%	0%	0%	0%		-	0%	0%	0%	0%		-	-	-

APPENDIX D-1 Synchro HCM and Queuing Reports Existing (2020) Traffic Conditions

		•	1	<i>></i>	-		
	23.74	i i	MA	100	1948	(V)	_
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	N.		1>		ħ	↑	
Traffic Volume (veh/h)	3	35	763	2	15	509	
Future Volume (Veh/h)	3	35	763	2	15	509	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly flow rate (vph)	3	36	787	2	15	525	
Pedestrians	1						
Lane Width (m)	3.6						
Walking Speed (m/s)	1.2						
Percent Blockage	0						
Right turn flare (veh)							
Median type			TWLTL			None	
Median storage veh)			2				
Upstream signal (m)			206				
pX, platoon unblocked	0.91	0.91			0.91		
vC, conflicting volume	1344	789			790		
vC1, stage 1 conf vol	789						
vC2, stage 2 conf vol	555						
vCu, unblocked vol	1328	715			716		
tC, single (s)	6.4	6.2			4.4		
tC, 2 stage (s)	5.4						
tF (s)	3.5	3.3			2.4		
p0 queue free %	99	91			98		
cM capacity (veh/h)	368	393			706		
			CD 1	CD 2			
Direction, Lane #	WB 1	NB 1	SB 1	SB 2			
Volume Total	39	789	15	525			
Volume Left	3	0	15	0			
Volume Right	36	2	0	0			
cSH	391	1700	706	1700			
Volume to Capacity	0.10	0.46	0.02	0.31			
Queue Length 95th (m)	2.6	0.0	0.5	0.0			
Control Delay (s)	15.2	0.0	10.2	0.0			
Lane LOS	С		В				
Approach Delay (s)	15.2	0.0	0.3				
Approach LOS	С						
Intersection Summary							
Average Delay			0.5				
Intersection Capacity Utiliza	ation		50.3%	IC	Ulevel	of Service	
Analysis Period (min)	20011		15	10	5 L5 VOI (J. 001 VI00	
Analysis i Gilou (IIIII)			10				

	٠	7	1	1	1	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W		7	^	1>		
Traffic Volume (veh/h)	50	5	4	703	499	17	
Future Volume (Veh/h)	50	5	4	703	499	17	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.68	0.50	0.25	0.90	0.95	0.79	
Hourly flow rate (vph)	74	10	16	781	525	22	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				TWI TI	TWLTL		
Median storage veh)				2	2		
Upstream signal (m)				156	_		
pX, platoon unblocked	0.89			100			
vC, conflicting volume	1349	536	547				
vC1, stage 1 conf vol	536	000	041				
vC2, stage 2 conf vol	813						
vCu, unblocked vol	1331	536	547				
tC, single (s)	6.4	6.2	4.8				
tC, 2 stage (s)	5.4	0.2	٦.0				
tF (s)	3.5	3.3	2.9				
p0 queue free %	79	98	98				
cM capacity (veh/h)	357	549	740				
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
Volume Total	84	16	781	547			
Volume Left	74	16	0	0			
Volume Right	10	0	0	22			
cSH	373	740	1700	1700			
Volume to Capacity	0.23	0.02	0.46	0.32			
Queue Length 95th (m)	6.8	0.5	0.0	0.0			
Control Delay (s)	17.4	10.0	0.0	0.0			
Lane LOS	С	Α					
Approach Delay (s)	17.4	0.2		0.0			
Approach LOS	С						
Intersection Summary							
Average Delay			1.1				
Intersection Capacity Utiliza	ation		47.0%	I	CU Level o	of Service	Α
Analysis Period (min)			15				

	•	-	1	4	†	1	↓
Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	468	960	1	1191	2	219	315
v/c Ratio	1.05	0.39	0.00	0.74	0.01	0.81	0.56
Control Delay	84.3	8.4	18.0	27.0	30.0	66.0	6.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	84.3	8.4	18.0	27.0	30.0	66.0	6.7
Queue Length 50th (m)	~90.5	45.5	0.1	111.8	0.2	49.2	0.0
Queue Length 95th (m)	#170.4	67.6	1.2	153.8	2.5	77.7	16.1
Internal Link Dist (m)		142.5		28.0	29.3		38.7
Turn Bay Length (m)	15.0		20.0			27.0	
Base Capacity (vph)	445	2435	270	1601	446	354	623
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	1.05	0.39	0.00	0.74	0.00	0.62	0.51

Intersection Summary

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	•		•	1	4	•	1	†	1	1	ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1		7	1			4		7	13	
Traffic Volume (vph)	426	872	2	1	814	269	0	1	1	199	0	287
Future Volume (vph)	426	872	2	1	814	269	0	1	1	199	0	287
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00		1.00	0.97			0.99		1.00	0.96	
Flpb, ped/bikes	1.00	1.00		0.99	1.00			1.00		0.99	1.00	
Frt	1.00	1.00		1.00	0.96			0.93		1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00			1.00		0.95	1.00	
Satd. Flow (prot)	1752	3504		1786	3267			1758		1758	1438	
Flt Permitted	0.12	1.00		0.30	1.00			1.00		0.76	1.00	
Satd. Flow (perm)	226	3504		561	3267			1758		1400	1438	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	468	958	2	1	895	296	0	1	1	219	0	315
RTOR Reduction (vph)	0	0	0	0	25	0	0	1	0	0	254	0
Lane Group Flow (vph)	468	960	0	1	1166	0	0	1	0	219	61	0
Confl. Peds. (#/hr)	40		18	18		40	18		2	2		18
Heavy Vehicles (%)	3%	3%	0%	0%	4%	2%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA			NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	78.9	78.9		54.9	54.9			22.0		22.0	22.0	
Effective Green, g (s)	78.9	78.9		54.9	54.9			22.0		22.0	22.0	
Actuated g/C Ratio	0.70	0.70		0.48	0.48			0.19		0.19	0.19	
Clearance Time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Lane Grp Cap (vph)	439	2435		271	1580			340		271	278	
v/s Ratio Prot	c0.20	0.27			0.36			0.00			0.04	
v/s Ratio Perm	c0.54			0.00						c0.16		
v/c Ratio	1.07	0.39		0.00	0.74			0.00		0.81	0.22	
Uniform Delay, d1	30.5	7.3		15.2	23.5			36.9		43.7	38.5	
Progression Factor	1.00	1.00		1.00	1.00			1.00		1.00	1.00	
Incremental Delay, d2	61.6	0.5		0.0	3.1			0.0		16.0	0.4	
Delay (s)	92.1	7.7		15.2	26.6			36.9		59.8	38.9	
Level of Service	F	Α		В	С			D		Е	D	
Approach Delay (s)		35.4			26.6			36.9			47.5	
Approach LOS		D			С			D			D	
Intersection Summary												
HCM 2000 Control Delay			34.1	H	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capa	acity ratio		1.03									
Actuated Cycle Length (s)			113.5		um of lost				15.6			
Intersection Capacity Utiliza	ation		88.9%	IC	U Level o	of Service			E			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	*	1		M		
Traffic Volume (veh/h)	4	1066	1042	12	5	4	
Future Volume (Veh/h)	4	1066	1042	12	5	4	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Hourly flow rate (vph)	5	1211	1184	14	6	5	
Pedestrians					12		
Lane Width (m)					3.6		
Walking Speed (m/s)					1.2		
Percent Blockage					1		
Right turn flare (veh)							
Median type		TWLTL	TWLTL				
Median storage veh)		2	2				
Upstream signal (m)		96					
pX, platoon unblocked					0.88		
vC, conflicting volume	1210				1818	611	
vC1, stage 1 conf vol					1203		
vC2, stage 2 conf vol					616		
vCu, unblocked vol	1210				1664	611	
tC, single (s)	4.1				7.2	7.4	
tC, 2 stage (s)					6.2		
tF (s)	2.2				3.7	3.5	
p0 queue free %	99				97	99	
cM capacity (veh/h)	578				198	381	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	SB 1	
Volume Total	5	606	606	789	409	11	
Volume Left	5	0	0	0	0	6	
Volume Right	0	0	0	0	14	5	
cSH	578	1700	1700	1700	1700	253	
Volume to Capacity	0.01	0.36	0.36	0.46	0.24	0.04	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.0	1.1	
Control Delay (s)	11.3	0.0	0.0	0.0	0.0	19.9	
Lane LOS	В					С	
Approach Delay (s)	0.0			0.0		19.9	
Approach LOS						С	
Intersection Summary							
Average Delay			0.1				
Intersection Capacity Utilization	on		39.5%	IC	U Level o	of Service	A
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		1		ħ	†	
Traffic Volume (veh/h)	0	2	725	0	2	510	
Future Volume (Veh/h)	0	2	725	0	2	510	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	0	2	763	0	2	537	
Pedestrians	1						
Lane Width (m)	3.6						
Walking Speed (m/s)	1.2						
Percent Blockage	0						
Right turn flare (veh)							
Median type			TWLTL			TWLTL	
Median storage veh)			2			2	
Upstream signal (m)			63			_ _	
pX, platoon unblocked	0.89	0.89	00		0.89		
vC, conflicting volume	1305	764			764		
vC1, stage 1 conf vol	764	701			701		
vC2, stage 2 conf vol	541						
vCu, unblocked vol	1281	672			672		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4	0.2			7.1		
tF (s)	3.5	3.3			2.2		
p0 queue free %	100	100			100		
cM capacity (veh/h)	381	408			824		
					024		
Direction, Lane #	WB 1	NB 1	SB 1	SB 2			
Volume Total	2	763	2	537			
Volume Left	0	0	2	0			
Volume Right	2	0	0	0			
cSH	408	1700	824	1700			
Volume to Capacity	0.00	0.45	0.00	0.32			
Queue Length 95th (m)	0.1	0.0	0.1	0.0			
Control Delay (s)	13.9	0.0	9.4	0.0			
Lane LOS	В		Α				
Approach Delay (s)	13.9	0.0	0.0				
Approach LOS	В						
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utiliza	ation		48.2%	IC	U Level	of Service	Α
Analysis Period (min)			15				

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	**	1		NA.		
Traffic Volume (veh/h)	0	1037	1046	1	1	1	
Future Volume (Veh/h)	0	1037	1046	1	1	1	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Hourly flow rate (vph)	0	1178	1189	1	1	1	
Pedestrians					8		
Lane Width (m)					3.6		
Walking Speed (m/s)					1.2		
Percent Blockage					1		
Right turn flare (veh)							
Median type		None	TWLTL				
Median storage veh)			2				
Upstream signal (m)		52					
pX, platoon unblocked					0.88		
vC, conflicting volume	1198				1786	603	
vC1, stage 1 conf vol					1198		
vC2, stage 2 conf vol					589		
vCu, unblocked vol	1198				1628	603	
tC, single (s)	4.1				6.8	6.9	
tC, 2 stage (s)					5.8		
tF (s)	2.2				3.5	3.3	
p0 queue free %	100				100	100	
cM capacity (veh/h)	586				235	444	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	SB 1	
Volume Total	0	589	589	793	397	2	
Volume Left	0	0	0	0	0	1	
Volume Right	0	0	0	0	1	1	
cSH	1700	1700	1700	1700	1700	308	
Volume to Capacity	0.00	0.35	0.35	0.47	0.23	0.01	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.2	
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	16.8	
Lane LOS						С	
Approach Delay (s)	0.0			0.0		16.8	
Approach LOS						С	
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilizat	tion		38.9%	IC	U Level c	of Service	A
Analysis Period (min)			15				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	A.			ની	13		
Traffic Volume (veh/h)	0	2	2	28	9	0	
Future Volume (Veh/h)	0	2	2	28	9	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	
Hourly flow rate (vph)	0	2	2	35	11	0	
Pedestrians				2	1		
Lane Width (m)				3.6	3.6		
Walking Speed (m/s)				1.2	1.2		
Percent Blockage				0	0		
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	51	13	11				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	51	13	11				
tC, single (s)	6.4	6.7	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.8	2.2				
p0 queue free %	100	100	100				
cM capacity (veh/h)	961	942	1621				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	2	37	11				
Volume Left	0	2	0				
Volume Right	2	0	0				
cSH	942	1621	1700				
Volume to Capacity	0.00	0.00	0.01				
Queue Length 95th (m)	0.1	0.0	0.0				
Control Delay (s)	8.8	0.4	0.0				
Lane LOS	Α	Α					
Approach Delay (s)	8.8	0.4	0.0				
Approach LOS	Α						
Intersection Summary							
Average Delay			0.6				
Intersection Capacity Utilizati	ion		14.0%	IC	CU Level c	of Service	A
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	NA.		1		7	†	
Traffic Volume (veh/h)	5	34	665	5	23	598	
Future Volume (Veh/h)	5	34	665	5	23	598	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	5	36	707	5	24	636	
Pedestrians			2				
Lane Width (m)			3.6				
Walking Speed (m/s)			1.2				
Percent Blockage			0				
Right turn flare (veh)							
Median type			TWLTL			None	
Median storage veh)			2				
Upstream signal (m)			206				
pX, platoon unblocked	0.95	0.95			0.95		
vC, conflicting volume	1396	710			712		
vC1, stage 1 conf vol	710						
vC2, stage 2 conf vol	686						
vCu, unblocked vol	1390	671			673		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4	V. <u> </u>					
tF (s)	3.5	3.3			2.2		
p0 queue free %	99	92			97		
cM capacity (veh/h)	366	439			884		
	WB 1	NB 1	SB 1	SB 2	001		
Direction, Lane # Volume Total	41	712	24	636			
Volume Left	5	0	24	030			
	36	5	0	0			
Volume Right cSH	428		884	1700			
		1700 0.42		0.37			
Volume to Capacity	0.10 2.5		0.03				
Queue Length 95th (m)		0.0		0.0			
Control Delay (s)	14.3	0.0	9.2	0.0			
Lane LOS	B	0.0	A				
Approach Delay (s)	14.3	0.0	0.3				
Approach LOS	В						
Intersection Summary							
Average Delay			0.6				
Intersection Capacity Utiliza	ation		45.3%	IC	U Level o	of Service	Α
Analysis Period (min)			15				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	A		7	†	1		
Traffic Volume (veh/h)	27	4	1	628	544	44	
Future Volume (Veh/h)	27	4	1	628	544	44	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.50	0.68	0.25	0.90	0.95	0.79	
Hourly flow rate (vph)	54	6	4	698	573	56	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				TWLTL	TWLTL		
Median storage veh)				2	2		
Upstream signal (m)				156			
pX, platoon unblocked	0.94						
vC, conflicting volume	1307	601	629				
vC1, stage 1 conf vol	601						
vC2, stage 2 conf vol	706						
vCu, unblocked vol	1295	601	629				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	5.4						
tF (s)	3.5	3.3	2.2				
p0 queue free %	86	99	100				
cM capacity (veh/h)	391	504	963				
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
Volume Total	60	4	698	629			
Volume Left	54	4	0	0			
Volume Right	6	0	0	56			
cSH	400	963	1700	1700			
Volume to Capacity	0.15	0.00	0.41	0.37			
Queue Length 95th (m)	4.2	0.1	0.0	0.0			
Control Delay (s)	15.6	8.8	0.0	0.0			
Lane LOS	С	Α					
Approach Delay (s)	15.6	0.0		0.0			
Approach LOS	С						
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utilizati	on		43.1%	I	CU Level c	of Service	A
Analysis Period (min)			15				

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Lane Group	EBL	EBT	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	289	697	1695	4	284	297
v/c Ratio	0.97	0.30	0.98	0.01	0.85	0.55
Control Delay	79.5	9.3	47.0	31.8	67.0	12.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	79.5	9.3	47.0	31.8	67.0	12.0
Queue Length 50th (m)	54.7	35.8	207.2	0.6	67.6	8.8
Queue Length 95th (m)	#111.9	45.7	#267.9	3.7	#115.8	36.0
Internal Link Dist (m)		142.5	28.0	29.3		38.7
Turn Bay Length (m)	15.0				27.0	
Base Capacity (vph)	298	2298	1727	412	335	541
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.97	0.30	0.98	0.01	0.85	0.55
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	•		7	1	#	•	1	Ť	1	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1		7	1			4		7	1	
Traffic Volume (vph)	275	661	1	0	1271	339	1	2	1	270	0	282
Future Volume (vph)	275	661	1	0	1271	339	1	2	1	270	0	282
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	6.3			6.3			6.3		6.3	6.3	
Lane Util. Factor	1.00	0.95			0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00			0.99			1.00		1.00	0.98	
Flpb, ped/bikes	1.00	1.00			1.00			1.00		1.00	1.00	
Frt	1.00	1.00			0.97			0.97		1.00	0.85	
Flt Protected	0.95	1.00			1.00			0.99		0.95	1.00	
Satd. Flow (prot)	1752	3504			3433			1805		1763	1464	
Flt Permitted	0.06	1.00			1.00			0.94		0.76	1.00	
Satd. Flow (perm)	118	3504			3433			1724		1402	1464	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	289	696	1	0	1338	357	1	2	1	284	0	297
RTOR Reduction (vph)	0	0	0	0	20	0	0	1	0	0	192	0
Lane Group Flow (vph)	289	697	0	0	1675	0	0	3	0	284	105	0
Confl. Peds. (#/hr)	8					8	6		1	1		6
Heavy Vehicles (%)	3%	3%	0%	0%	1%	1%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	78.7	78.7			59.7			28.7		28.7	28.7	
Effective Green, g (s)	78.7	78.7			59.7			28.7		28.7	28.7	
Actuated g/C Ratio	0.66	0.66			0.50			0.24		0.24	0.24	
Clearance Time (s)	3.0	6.3			6.3			6.3		6.3	6.3	
Lane Grp Cap (vph)	295	2298			1707			412		335	350	
v/s Ratio Prot	c0.13	0.20			0.49						0.07	
v/s Ratio Perm	c0.51							0.00		c0.20		
v/c Ratio	0.98	0.30			0.98			0.01		0.85	0.30	
Uniform Delay, d1	40.6	8.9			29.6			34.8		43.6	37.4	
Progression Factor	1.00	1.00			1.00			1.00		1.00	1.00	
Incremental Delay, d2	47.5	0.3			17.8			0.0		22.5	2.2	
Delay (s)	88.1	9.2			47.4			34.8		66.1	39.6	
Level of Service	F	Α			D			С		E	D	
Approach Delay (s)		32.3			47.4			34.8			52.6	
Approach LOS		С			D			С			D	
Intersection Summary												
HCM 2000 Control Delay			43.7	H	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.97									
Actuated Cycle Length (s)			120.0		ım of lost				15.6			
Intersection Capacity Utiliza	ation		96.8%	IC	U Level o	of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

	٨	-		•	1	1
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	**	†		Y	
Traffic Volume (veh/h)	11	928	1602	15	4	4
Future Volume (Veh/h)	11	928	1602	15	4	4
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	12	987	1704	16	4	4
Pedestrians					8	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)						
Median type		TWLTL	TWLTL			
Median storage veh)		2	2			
Upstream signal (m)		96				
pX, platoon unblocked					0.92	
vC, conflicting volume	1728				2238	868
vC1, stage 1 conf vol	20				1720	
vC2, stage 2 conf vol					518	
vCu, unblocked vol	1728				2168	868
tC, single (s)	4.1				6.8	6.9
tC, 2 stage (s)					5.8	
tF (s)	2.2				3.5	3.3
p0 queue free %	97				97	99
cM capacity (veh/h)	368				127	298
	EB 1	ED 0	ED 2	WB 1	WB 2	SB 1
Direction, Lane #		EB 2	EB 3			
Volume Total	12	494	494	1136	584	8
Volume Left	12	0	0	0	0	4
Volume Right	0	0	0	0	16	4
cSH	368	1700	1700	1700	1700	178
Volume to Capacity	0.03	0.29	0.29	0.67	0.34	0.05
Queue Length 95th (m)	0.8	0.0	0.0	0.0	0.0	1.1
Control Delay (s)	15.1	0.0	0.0	0.0	0.0	26.2
Lane LOS	С					D
Approach Delay (s)	0.2			0.0		26.2
Approach LOS						D
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utiliz	ation		54.8%	IC	U Level	of Service
Analysis Period (min)			15			
			.,			

	1	•	1	~	1	↓	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		1		7	†	
Traffic Volume (veh/h)	2	4	631	2	3	565	
Future Volume (Veh/h)	2	4	631	2	3	565	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly flow rate (vph)	2	4	651	2	3	582	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			TWLTL			TWLTL	
Median storage veh)			2			2	
Upstream signal (m)			63				
pX, platoon unblocked	0.93	0.93			0.93		
vC, conflicting volume	1240	652			653		
vC1, stage 1 conf vol	652						
vC2, stage 2 conf vol	588						
vCu, unblocked vol	1222	592			593		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4						
tF (s)	3.5	3.3			2.2		
p0 queue free %	100	99			100		
cM capacity (veh/h)	410	476			927		
Direction, Lane #	WB 1	NB 1	SB 1	SB 2			
Volume Total	6	653	3	582			
Volume Left	2	0	3	0			
Volume Right	4	2	0	0			
cSH	452	1700	927	1700			
Volume to Capacity	0.01	0.38	0.00	0.34			
Queue Length 95th (m)	0.3	0.0	0.1	0.0			
Control Delay (s)	13.1	0.0	8.9	0.0			
Lane LOS	В		Α				
Approach Delay (s)	13.1	0.0	0.0				
Approach LOS	В						
Intersection Summary							
Average Delay			0.1				
Intersection Capacity Utiliza	ition		43.3%	IC	U Level	of Service	A
Analysis Period (min)			15				

	١	-	+	•	1	4	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	**	1		Y		
Traffic Volume (veh/h)	0	937	1599	2	0	0	
Future Volume (Veh/h)	0	937	1599	2	0	0	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	0	997	1701	2	0	0	
Pedestrians					4		
Lane Width (m)					3.6		
Walking Speed (m/s)					1.2		
Percent Blockage					0		
Right turn flare (veh)							
Median type		None	TWLTL				
Median storage veh)			2				
Upstream signal (m)		52	_				
pX, platoon unblocked					0.92		
vC, conflicting volume	1707				2204	856	
vC1, stage 1 conf vol					1706		
vC2, stage 2 conf vol					498		
vCu, unblocked vol	1707				2131	856	
tC, single (s)	4.1				6.8	6.9	
tC, 2 stage (s)					5.8	0.0	
tF (s)	2.2				3.5	3.3	
p0 queue free %	100				100	100	
cM capacity (veh/h)	376				130	304	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	SB 1	
Volume Total	0	498	498	1134	569	0	
Volume Left	0	0	0	0	0	0	
Volume Right	0	0	0	0	2	0	
cSH	1700	1700	1700	1700	1700	1700	
Volume to Capacity	0.00	0.29	0.29	0.67	0.33	0.00	
Queue Length 95th (m)	0.00	0.29	0.29	0.07	0.0	0.00	
	0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay (s) Lane LOS	0.0	0.0	0.0	0.0	0.0	0.0 A	
	0.0			0.0		0.0	
Approach Delay (s) Approach LOS	0.0			0.0		0.0 A	
Intersection Summary						7,	
			0.0				
Average Delay	tion		0.0	10	lll ovel s	of Comiles	Λ
Intersection Capacity Utiliza	uOH		47.6%	IC	o Level (of Service	A
Analysis Period (min)			15				

	٨	*	4	†	ļ	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			र्स	1>		
Traffic Volume (veh/h)	0	1	4	29	2	1	
Future Volume (Veh/h)	0	1	4	29	2	1	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	1	4	32	2	1	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				110110	110110		
Jpstream signal (m)							
oX, platoon unblocked							
C, conflicting volume	42	2	3				
C1, stage 1 conf vol	72		J				
C2, stage 2 conf vol							
Cu, unblocked vol	42	2	3				
C, single (s)	6.4	6.2	4.1				
C, 2 stage (s)	0.4	0.2	7.1				
F (s)	3.5	3.3	2.2				
o0 queue free %	100	100	100				
cM capacity (veh/h)	971	1087	1632				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	1	36	3				
/olume Left	0	4	0				
/olume Right	1	0	1				
SH	1087	1632	1700				
Volume to Capacity	0.00	0.00	0.00				
Queue Length 95th (m)	0.0	0.1	0.0				
Control Delay (s)	8.3	0.8	0.0				
Lane LOS	Α	Α					
Approach Delay (s)	8.3	0.8	0.0				
Approach LOS	Α						
ntersection Summary							
Average Delay			0.9				
ntersection Capacity Utiliza	ation		14.9%	IC	CU Level o	of Service	Α
Analysis Period (min)			15				

APPENDIX D-2
Synchro HCM and Queuing Reports
Existing (2020) Traffic Conditions with Mitigation

	•	-	1		1	1	↓
Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	468	960	1	1191	2	219	315
v/c Ratio	0.91	0.39	0.00	0.83	0.01	0.83	0.49
Control Delay	51.8	8.0	23.0	36.2	31.5	71.3	2.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.8	8.0	23.0	36.2	31.5	71.3	2.6
Queue Length 50th (m)	86.3	47.4	0.2	138.2	0.2	50.9	0.0
Queue Length 95th (m)	#148.1	61.8	1.4	#187.4	2.6	#86.2	0.0
Internal Link Dist (m)		142.5		28.0	29.3		38.7
Turn Bay Length (m)	15.0		20.0			27.0	
Base Capacity (vph)	561	2465	240	1427	389	309	680
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.39	0.00	0.83	0.01	0.71	0.46
Intersection Summary							

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	٨	-	7	1		1	1	Ť	1	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† 1>		7	†			4		*	Þ	
Traffic Volume (vph)	426	872	2	1	814	269	0	1	1	199	0	287
Future Volume (vph)	426	872	2	1	814	269	0	1	1	199	0	287
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00		1.00	0.97			0.99		1.00	0.96	
Flpb, ped/bikes	1.00	1.00		0.99	1.00			1.00		0.99	1.00	
Frt	1.00	1.00		1.00	0.96			0.93		1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00			1.00		0.95	1.00	
Satd. Flow (prot)	1752	3504		1785	3265			1758		1757	1437	
Flt Permitted	0.09	1.00		0.30	1.00			1.00		0.76	1.00	
Satd. Flow (perm)	172	3504		560	3265			1758		1399	1437	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	468	958	2	1	895	296	0	1	1	219	0	315
RTOR Reduction (vph)	0	0	0	0	25	0	0	1	0	0	256	0
Lane Group Flow (vph)	468	960	0	1	1166	0	0	1	0	219	59	0
Confl. Peds. (#/hr)	40		18	18		40	18		2	2		18
Heavy Vehicles (%)	3%	3%	0%	0%	4%	2%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA			NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	81.8	81.8		50.0	50.0			21.9		21.9	21.9	
Effective Green, g (s)	81.8	81.8		50.0	50.0			21.9		21.9	21.9	
Actuated g/C Ratio	0.70	0.70		0.43	0.43			0.19		0.19	0.19	
Clearance Time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Lane Grp Cap (vph)	512	2464		240	1403			331		263	270	
v/s Ratio Prot	c0.23	0.27			0.36			0.00			0.04	
v/s Ratio Perm	c0.42			0.00						c0.16		
v/c Ratio	0.91	0.39		0.00	0.83			0.00		0.83	0.22	
Uniform Delay, d1	32.6	7.0		18.9	29.4			38.3		45.4	40.0	
Progression Factor	1.00	1.00		1.00	1.00			1.00		1.00	1.00	
Incremental Delay, d2	20.8	0.5		0.0	5.9			0.0		19.7	0.4	
Delay (s)	53.4	7.5		19.0	35.3			38.3		65.1	40.4	
Level of Service	D	Α		В	D			D		Е	D	
Approach Delay (s)		22.6			35.2			38.3			50.5	
Approach LOS		С			D			D			D	
Intersection Summary			00.1		014600							
HCM 2000 Control Delay	.,		32.1	H	SM 2000	Level of S	Service		С			
HCM 2000 Volume to Capa	acity ratio		0.92						45.5			
Actuated Cycle Length (s)			116.3		ım of lost				15.6			
Intersection Capacity Utiliza	ation		88.9%	IC	U Level o	of Service			E			
Analysis Period (min)			15									

APPENDIX E Correspondence with the City Background Traffic Growth Rate

Dumitru Liubeznii

From: Tyler Xuereb <Tyler.Xuereb@mississauga.ca>
Sent: Tuesday, November 26, 2019 10:32 AM

To: Dumitru Liubeznii

Subject: RE: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry

Hi Dumitru,

Using the City's Travel Demand Model and supporting Traffic count data, the City's Transportation Planning section has determined the projected growth on Lakeshore Road to be used as part of your study. The recommended projected growth is shown below:

Lakeshore Road

	Compounded Annual Growth from Existing to 2026					
	EB	WB				
Time						
AM Peak						
Hour	0.0%	2.0%				
PM Peak						
Hour	2.0%	0.0%				

-Note that this analysis assumes the Lakeview Development

Regards,

Tyler

From: Dumitru Liubeznii [mailto:dliubeznii@coleengineering.ca]

Sent: 2019/11/26 9:38 AM

To: Tyler Xuereb

Subject: RE: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry

Hi,

I have also attached the a copy of TOR circulated to the city for your reference. The traffic analysis will be conducted for following horizons:

- Existing (2019) traffic conditions; and,
- Future (2026) traffic conditions, which corresponds to 5-year after development full build out in year 2021.

Let me know if you have any further questions.

Kind regards,

Dumitru Liubeznii, EIT

Traffic Analyst

Urban Development (ICI&T)

Cole Engineering Group Ltd.

70 Valleywood Dr., Markham, ON L3R 4T5

T. 905-940-6161 Ext 295

F: 905-940-2064

Email: dliubeznii@coleengineering.ca
Website: www.ColeEngineering.ca

CONFIDENTIALITY NOTE

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Please consider the environment before printing this email

From: Tyler Xuereb < Tyler. Xuereb@mississauga.ca>

Sent: Tuesday, November 26, 2019 9:31 AM

To: Dumitru Liubeznii <dliubeznii@coleengineering.ca>

Subject: RE: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry

Thanks for this Dumitru!

What is the horizon years for your development?

Thanks,

Tyler

From: Dumitru Liubeznii [mailto:dliubeznii@coleengineering.ca]

Sent: 2019/11/26 9:21 AM

To: Tyler Xuereb **Cc:** Sevim Coskun

Subject: RE: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry

Hi Tyler,

I have attached the email correspondence in support of TOR circulation. It was suggested by Gregory Borys, C.E.T., to confirm the growth rates with you. As a reference, we used a study submitted in May 11, 2018 by BA Group for the proposed development at 1345 Lakeshore Road located just across the street from our site.

Please confirm if you are in agreement with the traffic growth rates we plan to use in our study, as described in email dated November 22, 2019 in beginning of this email chain.

Kind regards,

Dumitru Liubeznii, EIT

Traffic Analyst
Urban Development (ICI&T)

Cole Engineering Group Ltd.

70 Valleywood Dr., Markham, ON L3R 4T5

T. 905-940-6161 Ext 295

F: 905-940-2064

Email: <u>dliubeznii@coleengineering.ca</u>
Website: <u>www.ColeEngineering.ca</u>

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Please consider the environment before printing this email

From: Tyler Xuereb < <u>Tyler.Xuereb@mississauga.ca</u>>

Sent: Tuesday, November 26, 2019 9:03 AM

To: Dumitru Liubeznii < dliubeznii@coleengineering.ca>

Subject: RE: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry

Good Morning Dumitru,

Sorry for the late response as I was out of the office yesterday. Can you just confirm that a TOR has been submitted and that you have received comments back from city staff?

Thanks,

Tyler

From: Dumitru Liubeznii [mailto:dliubeznii@coleengineering.ca]

Sent: 2019/11/26 8:54 AM

To: Tyler Xuereb **Cc:** Sevim Coskun

Subject: RE: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry

Good morning Tyler,

Following the email we sent you earlier last week, we will proceed under the assumption that you are in agreement with proposed traffic growth rates that will be used in our study.

Kind regards,

Dumitru Liubeznii, EIT

Traffic Analyst Urban Development (ICI&T)

Cole Engineering Group Ltd.

70 Valleywood Dr., Markham, ON L3R 4T5

T. 905-940-6161 Ext 295

F: 905-940-2064

Email: dliubeznii@coleengineering.ca
Website: www.ColeEngineering.ca

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Please consider the environment before printing this email

From: Dumitru Liubeznii

Sent: Friday, November 22, 2019 2:17 PM

To: 'tyler.xuereb@mississauga.ca' < tyler.xuereb@mississauga.ca >

Cc: Sevim Coskun <SCoskun@coleengineering.ca>

Subject: [COLE#2019-0263] 1381 Lakeshore Road East - Future Traffic Growth Inquiry [Filed 22 Nov 2019 14:17]

Good afternoon Tyler,

We are conducting TIS for proposed residential development at 1381 Lakeshore Road with following major intersections being considered in the analysis:

- Dixie Road and Orchard Hill Road;
- Lakeshore Road East and Dixie Road; and,
- Lakeshore Road East and Cherriebell Road.

Based on preliminary research for similar development applications submitted in the area, we plan to use annual corridor traffic growth rate of 0.5% for the weekday morning and 2.0% for the afternoon peak hour in the <u>eastbound</u> <u>direction along Lakeshore Road East</u>. Similarly, a growth rate of 4.0% for the weekday morning and 0.0% for the afternoon peak hour per annum in the <u>westbound direction along Lakeshore Road</u> East morning. No growth rate will be applied to Dixie Road and other local roads.

Please confirm if you are in agreement with the aforementioned statement so we can proceed accordingly.

Kind regards,

Dumitru Liubeznii, EIT

Traffic Analyst, Urban Development Industrial, Commercial, Institutional & Traffic

Cole Engineering Group Ltd.

70 Valleywood Dr., Markham, ON L3R 4T5 T. 905-940-6161 Ext 295

F: 905-940-2064

Email: dliubeznii@coleengineering.ca
Website: www.ColeEngineering.ca

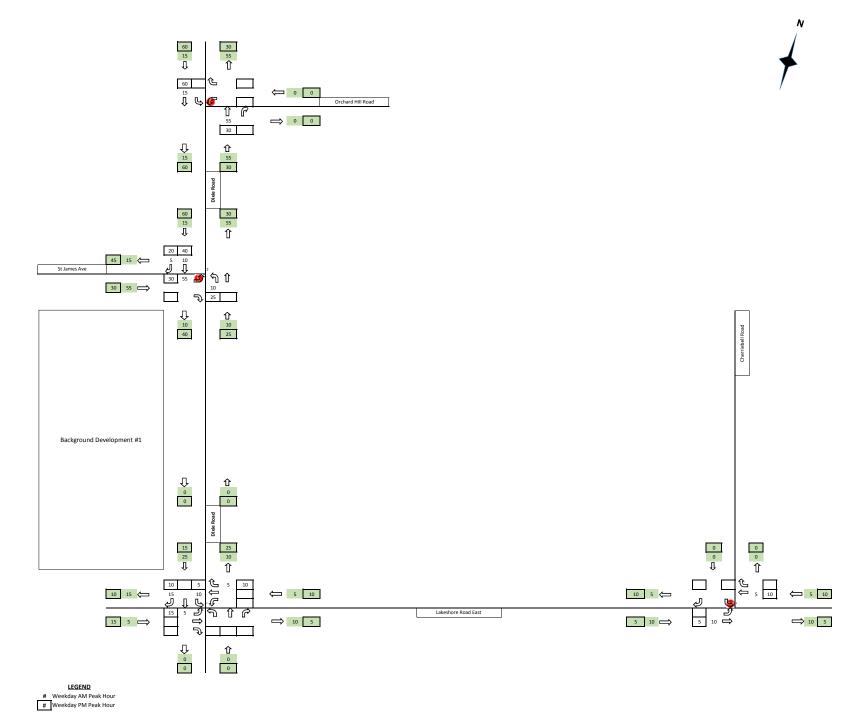
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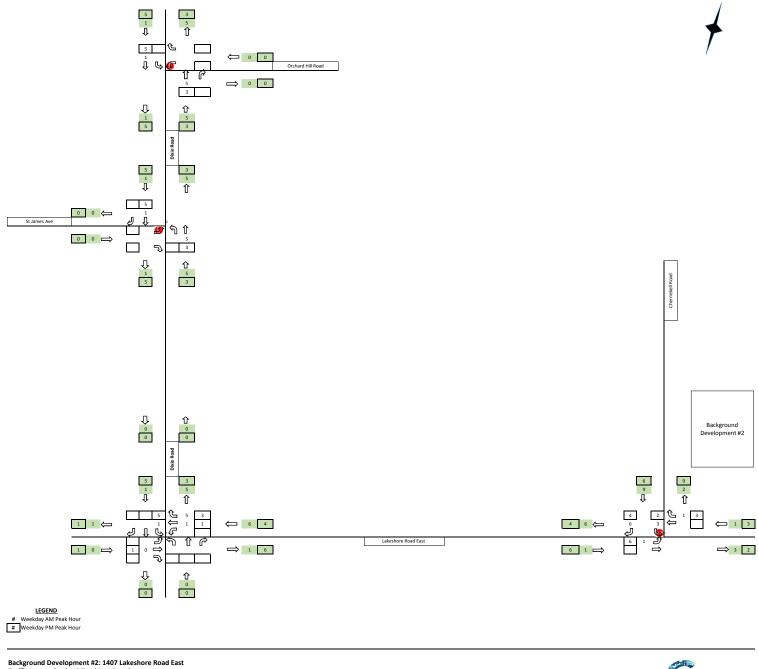
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APPENDIX F Background Development Traffic Volumes



Background Development #1: 1345 Lakeshore Road East Traffic Impact Study - Mixed-Use Development 1381 Lakeshore Road East, City of Mississauga 2019-0263







APPENDIX G Future Planned Roadway Improvements Supporting Documents

Lakeshore Road Transportation Master Plan and Implementation Strategy

DRAFT Final Report

May 2019





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Acknowledgements

City of Mississauga Steering Committee

Andy Harvey Geoff Marinoff Helen Noehammer Jason Bevan Jodi Robillos Joe Perrotta Lesley Pavan Mickey Frost

City of Mississauga Core Team

CITY OT MISSISSAUGA
Mark VanderSluis
Susan Tanabe
Norbert Orzel
Pauline Craig
Fred Sandoval
Jane Darragh
Yang Huang
Ben Philips
Romas Juknevicius

HDR

Tyrone Gan
Tara Erwin
Nico Malfara
Michelle Mascarenhas
Juan Rodriguez
Rhys Wolff
Kareem Kobeissi
Charlotte Yuen
Carl Wong
Liming Sun
Norman Walker
Soheil Kashi

DTAH

Brent Raymond Tanya Brown

Cumming+Company

Sue Cumming

Golder

Lynnette Dagenais Heather Melcher Henry Cary Hugh Daechsel

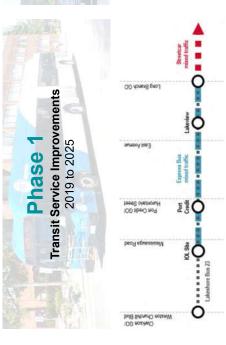
MSH

Elizabeth Howson Dave Yauk

4

Implementation and Phasing

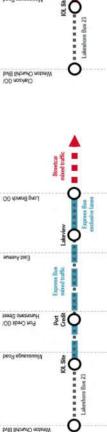
detailed design and construction may vary. The implementation strategy of the interim recommendation and ultimate recommendation follows a phased approach. For all phases of implementation the existing local service (Route 23) will be maintained to complement express bus service between Clarkson GO Station and Long Branch GO Station, via Port Credit GO Station. Changes to transit service concepts are at the discretion implementation of improvements is dependent on administrative prioritization. Depending on available funding and municipal priorities, the timing for this project to proceed with environmental assessment approvals, of MiWay.



Multi-Modal Road Work and Further Transit Improvements Phase 2A: 2025 to 2030 Phase 2B: 2031 to 2041 Phase 2



OD young fuo



Port Credit

Phase 3 (i.e. the final phase of implementation and ultimate Mississauga Road and East Avenue, and in exclusive lanes transit configuration) involves the conversion of the express bus based transit service to an extension of the Toronto between East Avenue and the Etobicoke Creek to Long streetcar service operating in mixed traffic between

Branch GO Station.

streetcar into Mississauga from the Long Branch GO Station is transit travel between Toronto and Mississauga by eliminating In the fullness of time (i.e. beyond 2041), the Study Corridor protected for, subject to discussions with the City of Toronto. The extension of the TTC streetcar will allow for seamless has been designed such that the extension of the TTC a forced transfer and additional fare at the border.

Phase 2 of the implementation strategy builds on Phase 1 and includes multimodal road work improvements and further transit service improvements. Phase 2 will be realized in two sub-phases as follows:

2025 with minimal infrastructure requirements. Phase 1 will be Phase 1 of the implementation strategy makes transit service

realized in three sub-phases as follows:

improvements along the Study Corridor between 2019 and

Increase local bus service by doubling the peak frequency

Upgrade local bus service from 40 ft to 60 ft buses to

increase capacity of the local bus

bus service

Introduce express bus service layered on top of the local

New transit stop infrastructure (i.e. bus shelters) would be required to implement this phase; however, no new major

transportation infrastructure would be required (i.e. road

widening or re-construction)

more frequent express bus service (70 Mississauga Road to Long Branch express rail (RER) service on the Lakeshore West GO Line. In addition to oriented development and facilitate direct, fast, and reliable transit trips to development of the Lakeview Village development site to support transit phase. Transit signal priority at intersections along the route can also be implemented to provide travel time reliability in the mixed traffic section Multi-modal road work (Shawnmarr Road to the Etobicoke Creek) and Avenue and the Etobicoke Creek. This should be completed with the Shawnmarr Road and East Avenue are also implemented during this GO Station) to be implemented between 2025 and 2030. This phase the exclusive transit lanes, multi-modal road work improvements (as shown in the preferred corridor design for Segments 4 to 7) between and from the site to the Long Branch GO station and future regional involves constructing exclusive median transit lanes between East Ä

Multi-modal road work (Winston Churchill Boulevard to Shawnmarr Road) and 2041. This phase includes multi-modal road work improvements (as to be implemented following the completion of Phase 2A between 2031 shown in the preferred corridor design for Segments 1-3) between

Winston Churchill Boulevard and Shawnmarr Road.

City of Mississauga | DRAFT Lakeshore Connecting Communities Final Report Transit, Right of Way and Credit River Crossing Alternatives

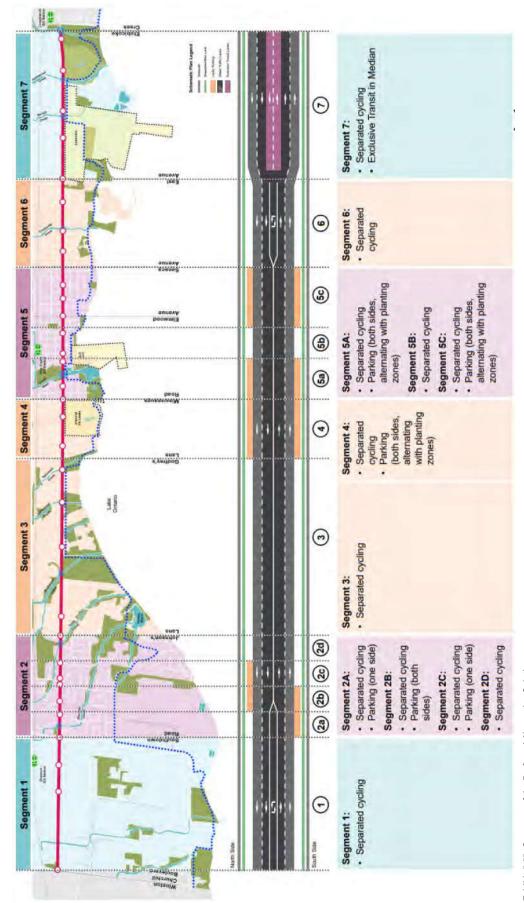
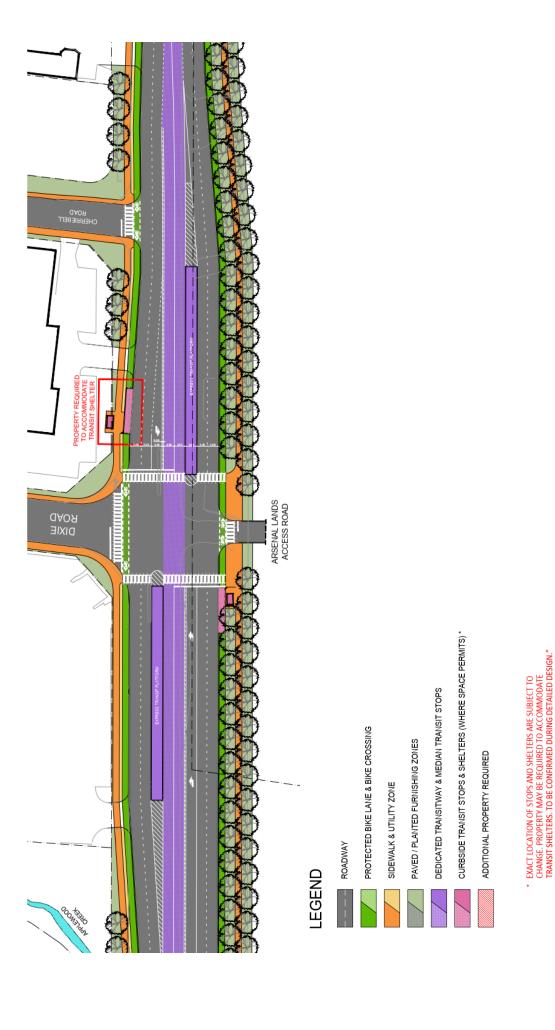


Exhibit 5-55: Summary of the Preferred Alternative Solution



SEGMENT 7: LAKEVIEW EMPLOYMENT AREA

Segment 7 has a 44.5 m right-of-way; therefore, dedicated transit options were considered. Option 3 was selected as the preferred alternative as shown in Exhibit 5-54. This option has six (6) lanes including two (2) exclusive transit lanes with separated cycling facilities on both sides. This option provides exclusive transit lanes in the median.

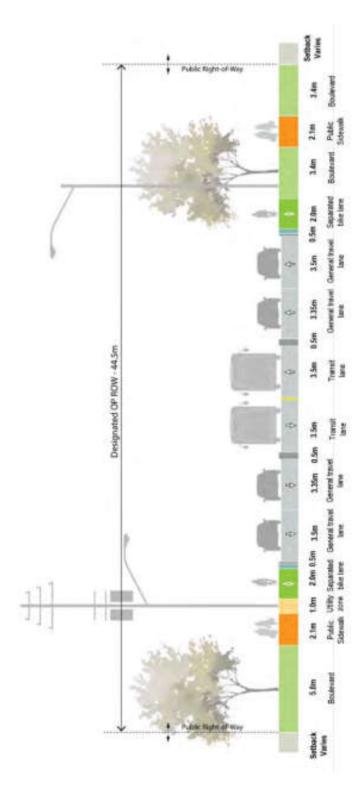


Exhibit 5-54 Preferred ROW Alternative (Segment 7)

APPENDIX H-1
Synchro HCM and Queuing Reports
Future (2025) Background Traffic Conditions

	1	•	1	~	1	1		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	M		1		7	†		
Traffic Volume (veh/h)	3	35	823	2	15	525		
Future Volume (Veh/h)	3	35	823	2	15	525		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97		
Hourly flow rate (vph)	3	36	848	2	15	541		
Pedestrians	1							
Lane Width (m)	3.6							
Walking Speed (m/s)	1.2							
Percent Blockage	0							
Right turn flare (veh)								
Median type			TWLTL			None		
Median storage veh)			2					
Upstream signal (m)			206					
pX, platoon unblocked	0.91	0.91			0.91			
vC, conflicting volume	1421	850			851			
vC1, stage 1 conf vol	850							
vC2, stage 2 conf vol	571							
vCu, unblocked vol	1414	789			790			
tC, single (s)	6.4	6.2			4.4			
tC, 2 stage (s)	5.4	0.2						
tF (s)	3.5	3.3			2.4			
p0 queue free %	99	90			98			
cM capacity (veh/h)	347	360			666			
	WB 1	NB 1	SB 1	SB 2				
Direction, Lane # Volume Total	39	850	15	541				
Volume Left	3	000	15	0				
	36	2	0	0				
Volume Right cSH			666	1700				
	359	1700						
Volume to Capacity	0.11	0.50	0.02	0.32				
Queue Length 95th (m)	2.9	0.0	0.6	0.0				
Control Delay (s)	16.3	0.0	10.5	0.0				
Lane LOS	C	0.0	В					
Approach Delay (s)	16.3	0.0	0.3					
Approach LOS	С							
Intersection Summary								
Average Delay			0.5					
Intersection Capacity Utiliza	ation		53.4%	IC	U Level o	of Service	A	
Analysis Period (min)			15					

Z. Dixie Road & St Jail	IES AVE						00/10/2020
	٠	*	1	†	1	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y		7	†	1		
Traffic Volume (veh/h)	105	5	14	708	510	22	
Future Volume (Veh/h)	105	5	14	708	510	22	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.68	0.50	0.25	0.90	0.95	0.79	
Hourly flow rate (vph)	154	10	56	787	537	28	
Pedestrians	10.				001		
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				T\A/I TI	TWLTL		
Median storage veh)				2	2		
,				156	2		
Upstream signal (m)	0.00			100			
pX, platoon unblocked	0.90	<i></i> 1	FCF				
vC, conflicting volume	1450	551	565				
vC1, stage 1 conf vol	551						
vC2, stage 2 conf vol	899	== 1	-0-				
vCu, unblocked vol	1444	551	565				
tC, single (s)	6.4	6.2	4.8				
tC, 2 stage (s)	5.4						
tF (s)	3.5	3.3	2.9				
p0 queue free %	51	98	92				
cM capacity (veh/h)	312	538	727				
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
Volume Total	164	56	787	565			
Volume Left	154	56	0	0			
Volume Right	10	0	0	28			
cSH	320	727	1700	1700			
Volume to Capacity	0.51	0.08	0.46	0.33			
Queue Length 95th (m)	22.1	2.0	0.0	0.0			
Control Delay (s)	27.5	10.4	0.0	0.0			
Lane LOS	D	В					
Approach Delay (s)	27.5	0.7		0.0			
Approach LOS	D						
Intersection Summary							
Average Delay			3.2				
Intersection Capacity Utiliza	ation		50.1%	I	CU Level o	of Service	A
Analysis Period (min)			15		23 20.010	. 300	
			.0				

	•	-	1		1	1	Ţ
Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	474	960	1	1316	2	231	332
v/c Ratio	0.94	0.39	0.00	0.94	0.01	0.85	0.51
Control Delay	60.8	8.2	23.0	45.8	31.5	73.4	2.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.8	8.2	23.0	45.8	31.5	73.4	2.9
Queue Length 50th (m)	95.6	49.8	0.2	~171.3	0.2	54.3	0.0
Queue Length 95th (m)	#158.7	61.8	1.4	#222.7	2.6	#93.4	0.0
Internal Link Dist (m)		142.5		28.0	29.3		38.7
Turn Bay Length (m)	15.0		20.0			27.0	
Base Capacity (vph)	542	2447	236	1407	387	307	672
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.87	0.39	0.00	0.94	0.01	0.75	0.49

Intersection Summary

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	٨	-	7	1		•	1	Ť	~	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† 1>		ሻ	1			4		7	₽	
Traffic Volume (vph)	431	872	2	1	918	279	0	1	1	210	0	302
Future Volume (vph)	431	872	2	1	918	279	0	1	1	210	0	302
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00		1.00	0.97			0.99		1.00	0.96	
Flpb, ped/bikes	1.00	1.00		0.99	1.00			1.00		0.99	1.00	
Frt	1.00	1.00		1.00	0.97			0.93		1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00			1.00		0.95	1.00	
Satd. Flow (prot)	1752	3504		1785	3277			1758		1757	1437	
Flt Permitted	0.08	1.00		0.30	1.00			1.00		0.76	1.00	
Satd. Flow (perm)	141	3504		560	3277			1758		1399	1437	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	474	958	2	1	1009	307	0	1	1	231	0	332
RTOR Reduction (vph)	0	0	0	0	23	0	0	1	0	0	268	0
Lane Group Flow (vph)	474	960	0	1	1293	0	0	1	0	231	64	0
Confl. Peds. (#/hr)	40		18	18		40	18		2	2		18
Heavy Vehicles (%)	3%	3%	0%	0%	4%	2%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA			NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	81.8	81.8		49.5	49.5			22.7		22.7	22.7	
Effective Green, g (s)	81.8	81.8		49.5	49.5			22.7		22.7	22.7	
Actuated g/C Ratio	0.70	0.70		0.42	0.42			0.19		0.19	0.19	
Clearance Time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Lane Grp Cap (vph)	501	2447		236	1385			340		271	278	
v/s Ratio Prot	c0.24	0.27			0.39			0.00			0.04	
v/s Ratio Perm	c0.43			0.00						c0.17		
v/c Ratio	0.95	0.39		0.00	0.93			0.00		0.85	0.23	
Uniform Delay, d1	35.6	7.3		19.5	32.2			38.1		45.6	39.8	
Progression Factor	1.00	1.00		1.00	1.00			1.00		1.00	1.00	
Incremental Delay, d2	27.0	0.5		0.0	12.8			0.0		21.9	0.4	
Delay (s)	62.6	7.8		19.6	45.1			38.1		67.5	40.3	
Level of Service	Е	Α		В	D			D		Е	D	
Approach Delay (s)		25.9			45.1			38.1			51.4	
Approach LOS		С			D			D			D	
Intersection Summary			0= 0		014600							
HCM 2000 Control Delay	.,		37.9	H	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.95						45.0			
Actuated Cycle Length (s)			117.1		um of lost				15.6			
Intersection Capacity Utiliza	ation		93.2%	IC	U Level o	of Service			F			
Analysis Period (min)			15									

	۶	→	+	•	1	1
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሽ	*	†		W	
Traffic Volume (veh/h)	5	1076	1178	13	8	10
Future Volume (Veh/h)	5	1076	1178	13	8	10
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	1223	1339	15	9	11
Pedestrians					12	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					1	
Right turn flare (veh)						
Median type		TWLTL	TWLTL			
Median storage veh)		2	2			
Upstream signal (m)		96				
pX, platoon unblocked					0.89	
vC, conflicting volume	1366				1982	689
vC1, stage 1 conf vol					1358	
vC2, stage 2 conf vol					624	
vCu, unblocked vol	1366				1851	689
tC, single (s)	4.1				7.2	7.4
tC, 2 stage (s)					6.2	
tF (s)	2.2				3.7	3.5
p0 queue free %	99				94	97
cM capacity (veh/h)	504				163	336
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	SB 1
Volume Total	6	612	612	893	461	20
Volume Left	6	0	0	0	0	9
Volume Right	0	0	0	0	15	11
cSH	504	1700	1700	1700	1700	227
Volume to Capacity	0.01	0.36	0.36	0.53	0.27	0.09
Queue Length 95th (m)	0.3	0.0	0.0	0.0	0.0	2.3
Control Delay (s)	12.2	0.0	0.0	0.0	0.0	22.4
Lane LOS	В					С
Approach Delay (s)	0.1			0.0		22.4
Approach LOS						С
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utiliza	ation		43.0%	IC	U Level o	of Service
Analysis Period (min)	-		15			

	1	•	1	~	1	Ţ		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Y		1		7	†		
Traffic Volume (veh/h)	5	34	698	5	23	663		
Future Volume (Veh/h)	5	34	698	5	23	663		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Hourly flow rate (vph)	5	36	743	5	24	705		
Pedestrians			2					
Lane Width (m)			3.6					
Walking Speed (m/s)			1.2					
Percent Blockage			0					
Right turn flare (veh)								
Median type			TWLTL			None		
Median storage veh)			2					
Upstream signal (m)			206					
pX, platoon unblocked	0.97	0.97			0.97			
vC, conflicting volume	1500	746			748			
vC1, stage 1 conf vol	746							
vC2, stage 2 conf vol	755							
vCu, unblocked vol	1501	721			724			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)	5.4							
tF (s)	3.5	3.3			2.2			
p0 queue free %	99	91			97			
cM capacity (veh/h)	342	417			860			
Direction, Lane #	WB 1	NB 1	SB 1	SB 2				
Volume Total	41	748	24	705				
Volume Left	5	0	24	0				
Volume Right	36	5	0	0				
cSH	406	1700	860	1700				
Volume to Capacity	0.10	0.44	0.03	0.41				
Queue Length 95th (m)	2.7	0.0	0.7	0.0				
Control Delay (s)	14.9	0.0	9.3	0.0				
Lane LOS	В		Α					
Approach Delay (s)	14.9	0.0	0.3					
Approach LOS	В							
Intersection Summary								
Average Delay			0.5					
Intersection Capacity Utiliza	tion		47.0%	IC	U Level o	of Service	A	
Analysis Period (min)			15					

	٠	*	1	1	1	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y		7	↑	13		
Traffic Volume (veh/h)	57	4	26	631	589	64	
Future Volume (Veh/h)	57	4	26	631	589	64	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.50	0.68	0.25	0.90	0.95	0.79	
Hourly flow rate (vph)	114	6	104	701	620	81	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				TWI TI	TWLTL		
Median storage veh)				2	2		
Upstream signal (m)				156	_		
pX, platoon unblocked	0.95			100			
vC, conflicting volume	1570	660	701				
vC1, stage 1 conf vol	660	000	701				
vC2, stage 2 conf vol	909						
vCu, unblocked vol	1573	660	701				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	5.4	0.2	7.1				
tF (s)	3.5	3.3	2.2				
p0 queue free %	61	99	89				
cM capacity (veh/h)	295	466	905				
				OD 4			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
Volume Total	120	104	701	701			
Volume Left	114	104	0	0			
Volume Right	6	0	0	81			
cSH	300	905	1700	1700			
Volume to Capacity	0.40	0.11	0.41	0.41			
Queue Length 95th (m)	14.8	3.1	0.0	0.0			
Control Delay (s)	24.8	9.5	0.0	0.0			
Lane LOS	С	Α					
Approach Delay (s)	24.8	1.2		0.0			
Approach LOS	С						
Intersection Summary							
Average Delay			2.4				
Intersection Capacity Utiliza	ation		44.9%		CU Level of	of Service	Α
Analysis Period (min)			15				

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Lane Group	EBL	EBT	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	305	785	1710	4	295	307
v/c Ratio	1.02	0.34	0.99	0.01	0.88	0.57
Control Delay	93.2	9.7	49.1	31.8	71.4	13.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	93.2	9.7	49.1	31.8	71.4	13.0
Queue Length 50th (m)	~63.1	41.8	210.9	0.6	70.8	10.8
Queue Length 95th (m)	#121.3	52.4	#272.4	3.7	#122.5	39.6
Internal Link Dist (m)		142.5	28.0	29.3		38.7
Turn Bay Length (m)	15.0				27.0	
Base Capacity (vph)	298	2298	1726	412	335	541
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.02	0.34	0.99	0.01	0.88	0.57

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	Þ		*	1		1	1	1	1	1	1	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	†		7	1			4		7	T _p	
Traffic Volume (vph)	290	745	1	0	1272	352	1	2	1	280	0	292
Future Volume (vph)	290	745	1	0	1272	352	1	2	1	280	0	292
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	6.3			6.3			6.3		6.3	6.3	
Lane Util. Factor	1.00	0.95			0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00			0.99			1.00		1.00	0.98	
Flpb, ped/bikes	1.00	1.00			1.00			1.00		1.00	1.00	
Frt	1.00	1.00			0.97			0.97		1.00	0.85	
Flt Protected	0.95	1.00			1.00			0.99		0.95	1.00	
Satd. Flow (prot)	1752	3504			3429			1805		1763	1464	
FIt Permitted	0.06	1.00			1.00			0.94		0.76	1.00	
Satd. Flow (perm)	118	3504			3429			1722		1402	1464	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	305	784	1	0	1339	371	1	2	1	295	0	307
RTOR Reduction (vph)	0	0	0	0	21	0	0	1	0	0	192	0
Lane Group Flow (vph)	305	785	0	0	1689	0	0	3	0	295	115	0
Confl. Peds. (#/hr)	8					8	6		1	1		6
Heavy Vehicles (%)	3%	3%	0%	0%	1%	1%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	78.7	78.7			59.7			28.7		28.7	28.7	
Effective Green, g (s)	78.7	78.7			59.7			28.7		28.7	28.7	
Actuated g/C Ratio	0.66	0.66			0.50			0.24		0.24	0.24	
Clearance Time (s)	3.0	6.3			6.3			6.3		6.3	6.3	
Lane Grp Cap (vph)	295	2298			1705			411		335	350	
v/s Ratio Prot	c0.14	0.22			0.49						0.08	
v/s Ratio Perm	c0.54							0.00		c0.21		
v/c Ratio	1.03	0.34			0.99			0.01		0.88	0.33	
Uniform Delay, d1	41.1	9.2			29.9			34.8		44.0	37.7	
Progression Factor	1.00	1.00			1.00			1.00		1.00	1.00	
Incremental Delay, d2	61.5	0.4			19.7			0.0		26.5	2.5	
Delay (s)	102.6	9.6			49.5			34.8		70.5	40.2	
Level of Service	F	Α			D			С		Е	D	
Approach Delay (s)		35.6			49.5			34.8			55.1	
Approach LOS		D			D			С			E	
Intersection Summary												
HCM 2000 Control Delay			46.0	H	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		1.02									
Actuated Cycle Length (s)			120.0		um of lost				15.6			
Intersection Capacity Utiliza	ation		98.7%	IC	U Level o	of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

	۶	-		•	1	1	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	*	**	1		1		
Traffic Volume (veh/h)	17	1050	1612	18	6	8	
Future Volume (Veh/h)	17	1050	1612	18	6	8	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	18	1117	1715	19	6	9	
Pedestrians					8		
Lane Width (m)					3.6		
Walking Speed (m/s)					1.2		
Percent Blockage					1		
Right turn flare (veh)							
Median type		TWLTL	TWLTL				
Median storage veh)		2	2				
Upstream signal (m)		96					
pX, platoon unblocked					0.90		
vC, conflicting volume	1742				2327	875	
vC1, stage 1 conf vol					1732	0.0	
vC2, stage 2 conf vol					594		
vCu, unblocked vol	1742				2254	875	
tC, single (s)	4.1				6.8	6.9	
tC, 2 stage (s)					5.8	0.0	
tF (s)	2.2				3.5	3.3	
p0 queue free %	95				95	97	
cM capacity (veh/h)	363				124	295	
		ED 0	ED 2	WD 4			
Direction, Lane # Volume Total	EB 1	EB 2 558	EB 3 558	WB 1 1143	WB 2 591	SB 1 15	
Volume Left	18	0	0	0	0	6	
			0	0	19	9	
Volume Right	0	1700					
cSH Valume to Canacity	363	1700	1700	1700	1700	190	
Volume to Capacity	0.05	0.33	0.33	0.67	0.35	0.08	
Queue Length 95th (m)	1.2	0.0	0.0	0.0	0.0	2.0	
Control Delay (s)	15.4	0.0	0.0	0.0	0.0	25.5	
Lane LOS	C			0.0		D	
Approach Delay (s)	0.2			0.0		25.5	
Approach LOS						D	
Intersection Summary							
Average Delay			0.2				
Intersection Capacity Utilizati	ion		55.1%	IC	U Level o	of Service	В
Analysis Period (min)			15				

APPENDIX H-2 Synchro HCM and Queuing Reports Future (2025) Background Traffic Conditions with Mitigation

	•	→	*	Ť	1	1
Lane Group	EBL	EBT	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	305	785	1710	4	295	307
v/c Ratio	0.93	0.33	0.96	0.01	0.91	0.58
Control Delay	69.8	8.3	41.4	33.2	78.0	12.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	69.8	8.3	41.4	33.2	78.0	12.6
Queue Length 50th (m)	57.9	38.2	202.9	0.6	71.7	8.8
Queue Length 95th (m)	#113.6	47.9	#264.4	3.7	#125.5	37.5
Internal Link Dist (m)		142.5	28.0	29.3		38.7
Turn Bay Length (m)	15.0				27.0	
Base Capacity (vph)	327	2386	1782	382	323	530
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.93	0.33	0.96	0.01	0.91	0.58
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

2019-0263_1381_Lakeshore_Rd - TIS COLE

Queue shown is maximum after two cycles.

3: SAIB Private Access/Dixie Road & Lakeshore Road East	5		1			
	3: SAIB Private A	ccess/Dixie	Road &	Lakeshore	Road	Eas

	٠	-	7	1		•	1	1	~	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	1		7	1			4		7	1	
Traffic Volume (vph)	290	745	1	0	1272	352	1	2	1	280	0	292
Future Volume (vph)	290	745	1	0	1272	352	1	2	1	280	0	292
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	2.0	5.3			5.3			6.3		5.3	6.3	
Lane Util. Factor	1.00	0.95			0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00			0.99			1.00		1.00	0.98	
Flpb, ped/bikes	1.00	1.00			1.00			1.00		1.00	1.00	
Frt	1.00	1.00			0.97			0.97		1.00	0.85	
Flt Protected	0.95	1.00			1.00			0.99		0.95	1.00	
Satd. Flow (prot)	1752	3504			3429			1805		1763	1464	
Flt Permitted	0.06	1.00			1.00			0.94		0.76	1.00	
Satd. Flow (perm)	116	3504			3429			1717		1402	1464	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	305	784	1	0.00	1339	371	1	2	1	295	0.00	307
RTOR Reduction (vph)	0	0	0	0	21	0	0	1	0	0	204	0
Lane Group Flow (vph)	305	785	0	0	1689	0	0	3	0	295	103	0
Confl. Peds. (#/hr)	8	700	U	U	1003	8	6	J	1	1	100	6
Heavy Vehicles (%)	3%	3%	0%	0%	1%	1%	0%	0%	0%	2%	0%	8%
			0 /0		NA	1 /0			0 70		NA	0 70
Turn Type Protected Phases	pm+pt	NA		Perm	NA 6		Perm	NA 8		Perm	4	
Permitted Phases	5 2	2		6	U		8	0		4	4	
	80.7	80.7		Ü	60.7		0	26.7		26.7	26.7	
Actuated Green, G (s) Effective Green, g (s)	81.7	81.7			61.7			26.7		27.7	26.7	
		0.68						0.22		0.23	0.22	
Actuated g/C Ratio	0.68				0.51 6.3			6.3		6.3		
Clearance Time (s)	3.0	6.3									6.3	
Lane Grp Cap (vph)	324	2385			1763			382		323	325	
v/s Ratio Prot	c0.14	0.22			c0.49			2.22		0.04	0.07	
v/s Ratio Perm	0.50							0.00		c0.21		
v/c Ratio	0.94	0.33			0.96			0.01		0.91	0.32	
Uniform Delay, d1	40.1	7.9			27.9			36.3		45.0	39.0	
Progression Factor	1.00	1.00			1.00			1.00		1.00	1.00	
Incremental Delay, d2	37.1	0.4			13.6			0.0		32.2	2.5	
Delay (s)	77.1	8.2			41.5			36.4		77.2	41.5	
Level of Service	E	Α			D			D		E	D	
Approach Delay (s)		27.5			41.5			36.4			59.0	
Approach LOS		С			D			D			Е	
Intersection Summary									_			
HCM 2000 Control Delay			40.1	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.95									
Actuated Cycle Length (s)			120.0		um of lost				13.6			
Intersection Capacity Utiliza	ation		97.8%	IC	U Level of	of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

APPENDIX I Synchro HCM and Queuing Reports Future (2025) Total Traffic Conditions

	1	•	1	1	1	ļ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1→		7	†
Traffic Volume (veh/h)	3	35	850	2	15	534
Future Volume (Veh/h)	3	35	850	2	15	534
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	3	36	876	2	15	551
Pedestrians	1		J. J	_		
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type			TWLTL			None
Median storage veh)			2			140110
Upstream signal (m)			206			
pX, platoon unblocked	0.91	0.91	200		0.91	
vC, conflicting volume	1459	878			879	
vC1, stage 1 conf vol	878	070			013	
vC2, stage 2 conf vol	581					
vCu, unblocked vol	1455	818			819	
tC, single (s)	6.4	6.2			4.4	
tC, 2 stage (s)	5.4	0.2			4.4	
tF (s)	3.5	3.3			2.4	
p0 queue free %	99	90			98	
	336	345			648	
cM capacity (veh/h)					040	
Direction, Lane #	WB 1	NB 1	SB 1	SB 2		
Volume Total	39	878	15	551		
Volume Left	3	0	15	0		
Volume Right	36	2	0	0		
cSH	345	1700	648	1700		
Volume to Capacity	0.11	0.52	0.02	0.32		
Queue Length 95th (m)	3.0	0.0	0.6	0.0		
Control Delay (s)	16.8	0.0	10.7	0.0		
Lane LOS	С		В			
Approach Delay (s)	16.8	0.0	0.3			
Approach LOS	С					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliza	ation		54.9%	IC	U Level	of Service
Analysis Period (min)			15	.0	2 23707	55. 1100
Analysis i ellou (IIIII)			IJ			

Z. Dixie Road & St Jail	IIES AVE						00/10/2020
	Þ	•	1	1	ļ	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y		7	†	1		
Traffic Volume (veh/h)	105	5	14	735	519	22	
Future Volume (Veh/h)	105	5	14	735	519	22	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.68	0.50	0.25	0.90	0.95	0.79	
Hourly flow rate (vph)	154	10	56	817	546	28	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				TWLTL	TWLTL		
Median storage veh)				2	2		
Upstream signal (m)				156	_		
pX, platoon unblocked	0.89						
vC, conflicting volume	1489	560	574				
vC1, stage 1 conf vol	560		• • •				
vC2, stage 2 conf vol	929						
vCu, unblocked vol	1488	560	574				
tC, single (s)	6.4	6.2	4.8				
tC, 2 stage (s)	5.4	·					
tF (s)	3.5	3.3	2.9				
p0 queue free %	49	98	92				
cM capacity (veh/h)	301	532	720				
				CD 4			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
Volume Total	164	56	817	574			
Volume Left	154	56	0	0			
Volume Right	10	0	0	28			
cSH	310	720	1700	1700			
Volume to Capacity	0.53	0.08	0.48	0.34			
Queue Length 95th (m)	23.3	2.0	0.0	0.0			
Control Delay (s)	29.0	10.4	0.0	0.0			
Lane LOS	D	В					
Approach Delay (s)	29.0	0.7		0.0			
Approach LOS	D						
Intersection Summary							
Average Delay			3.3				
Intersection Capacity Utiliza	ation		51.5%	I	CU Level of	of Service	Α
Analysis Period (min)			15				

	•	-	1	•	†	1	Ţ
Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
			VVDL				
Lane Group Flow (vph)	476	960	1	1325	2	241	332
v/c Ratio	0.94	0.39	0.00	0.95	0.01	0.88	0.51
Control Delay	61.2	8.4	23.0	48.1	31.5	76.3	2.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.2	8.4	23.0	48.1	31.5	76.3	2.9
Queue Length 50th (m)	96.6	50.0	0.2	~180.2	0.2	57.2	0.0
Queue Length 95th (m)	#160.3	61.8	1.4	#225.2	2.6	#99.6	0.0
Internal Link Dist (m)		142.5		28.0	29.3		38.7
Turn Bay Length (m)	15.0		20.0			27.0	
Base Capacity (vph)	540	2438	234	1396	385	306	672
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.88	0.39	0.00	0.95	0.01	0.79	0.49

Intersection Summary

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	٨	-	*	1		•	1	1	~	/	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1		7	1			4		7	1	
Traffic Volume (vph)	433	872	2	1	926	279	0	1	1	219	0	302
Future Volume (vph)	433	872	2	1	926	279	0	1	1	219	0	302
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Lane Util. Factor	1.00	0.95		1.00	0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00		1.00	0.97			0.99		1.00	0.96	
Flpb, ped/bikes	1.00	1.00		0.99	1.00			1.00		0.99	1.00	
Frt	1.00	1.00		1.00	0.97			0.93		1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00			1.00		0.95	1.00	
Satd. Flow (prot)	1752	3504		1785	3278			1758		1757	1437	
Flt Permitted	0.08	1.00		0.30	1.00			1.00		0.76	1.00	
Satd. Flow (perm)	141	3504		560	3278			1758		1399	1437	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	476	958	2	1	1018	307	0	1	1	241	0	332
RTOR Reduction (vph)	0	0	0	0	23	0	0	1	0	0	267	0
Lane Group Flow (vph)	476	960	0	1	1302	0	0	1	0	241	65	0
Confl. Peds. (#/hr)	40		18	18		40	18		2	2		18
Heavy Vehicles (%)	3%	3%	0%	0%	4%	2%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA			NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	81.8	81.8		49.3	49.3			23.2		23.2	23.2	
Effective Green, g (s)	81.8	81.8		49.3	49.3			23.2		23.2	23.2	
Actuated g/C Ratio	0.70	0.70		0.42	0.42			0.20		0.20	0.20	
Clearance Time (s)	3.0	6.3		6.3	6.3			6.3		6.3	6.3	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Lane Grp Cap (vph)	502	2437		234	1374			346		275	283	
v/s Ratio Prot	c0.24	0.27			0.40			0.00			0.05	
v/s Ratio Perm	c0.42			0.00						c0.17		
v/c Ratio	0.95	0.39		0.00	0.95			0.00		0.88	0.23	
Uniform Delay, d1	35.8	7.5		19.9	32.9			37.9		45.8	39.7	
Progression Factor	1.00	1.00		1.00	1.00			1.00		1.00	1.00	
Incremental Delay, d2	27.3	0.5		0.0	14.7			0.0		25.3	0.4	
Delay (s)	63.1	8.0		19.9	47.6			37.9		71.1	40.1	
Level of Service	Е	Α		В	D			D		Е	D	
Approach Delay (s)		26.3			47.5			37.9			53.1	
Approach LOS		С			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			39.3	H	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.95									
Actuated Cycle Length (s)			117.6		um of lost				15.6			
Intersection Capacity Utiliza	ation		93.6%	IC	U Level o	of Service			F			
Analysis Period (min)			15									

c Critical Lane Group

Movement
Traffic Volume (veh/h) 14 1076 1178 18 23 18 Future Volume (Veh/h) 14 1076 1178 18 23 18 Sign Control Free Free Stop Grade 0% 0% 0% 0% Peak Hour Factor 0.88 0.88 0.88 0.88 0.88 0.88 0.88 Hourly flow rate (vph) 16 1223 1339 20 26 20 Pedestrians 12 Lane Width (m) 3.6 Walking Speed (m/s) 1.2 Percent Blockage 1 1 Right turn flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 2 Upstream signal (m) 96 pX, platoon unblocked vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol vC2, stage 2 conf vol vC4, stage 2 conf vol vC4, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 50 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 0 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 0.0
Traffic Volume (veh/h) 14 1076 1178 18 23 18 Future Volume (Veh/h) 14 1076 1178 18 23 18 Sign Control Free Free Free Stop 7 Grade 0% 0% 0% 0% Peadestrians 12 2 20 Pedestrians 12 3.6 2 Lane Width (m) 3.6 3.6 3.6 Walking Speed (m/s) 1.2 2 Percent Blockage 1 1 Right turn flare (veh) 3.6 3.6 Median type TWLTL TWLTL Wedian storage veh) 2 2 Upstream signal (m) 96 96 pX, platoon unblocked 0.89 vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 644 44 vC2, stage 2 conf vol 644 44 vC3, stage (s) 1371 1875
Future Volume (Veh/h) 14 1076 1178 18 23 18 Sign Control Free Free Stop Grade 0% 0% 0% Peak Hour Factor 0.88 0.88 0.88 0.88 0.88 0.88 Hourly flow rate (vph) 16 1223 1339 20 26 20 Pedestrians 12 13 13 12 12 12 12 12 12 12 12 12
Grade 0% 0% 0% Peak Hour Factor 0.88 0.89
Grade 0% 0% 0% Peak Hour Factor 0.88 0.89
Peak Hour Factor 0.88 0.80 20<
Hourly flow rate (vph) 16 1223 1339 20 26 20 Pedestrians 12 Lane Width (m) 3.6 Walking Speed (m/s) 1.2 Percent Blockage 1 Right turn flare (veh) Median type TWLTL TWLTL Median storage veh) 2 2 Upstream signal (m) 96 pX, platoon unblocked vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol vC2, stage 2 conf vol vC4, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 183 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 20 cSH 502 T700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 0.0
Pedestrians
Lane Width (m) 3.6 Walking Speed (m/s) 1.2 Percent Blockage 1 Right turn flare (veh) TWLTL Median type TWLTL Median storage veh) 2 Upstream signal (m) 96 pX, platoon unblocked 0.89 vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 1361 004 004 004 vC2, stage 2 conf vol 644 004 004 004 004 004 004 004 004 004 004 004
Walking Speed (m/s) 1.2 Percent Blockage 1 Right turn flare (veh) TWLTL Median storage veh) 2 Upstream signal (m) 96 pX, platoon unblocked 0.89 vC, conflicting volume 1371 vC1, stage 1 conf vol 1361 vC2, stage 2 conf vol 644 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 15 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 20 volume Right 0 0 0 0 0 20 volume to Capacity 0.03 0.36 0.36 0.53 </td
Percent Blockage
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Median type TWLTL TWLTL TWLTL Median storage veh) 2 2 Upstream signal (m) 96 0.89 pX, platoon unblocked 0.89 0.89 vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 1361 0.89 0.89 0.89 vC1, stage 1 conf vol 1361 0.89 0.99 0.89 0.89 0.99 0.89 0.99 0.89 0.99 0.89 0.99 0.89 0.99 0.89 0.99 0.99 0.89 0.99 0.99<
Median storage veh) 2 2 Upstream signal (m) 96 pX, platoon unblocked 0.89 vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 1361 vC2, stage 2 conf vol 644 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 157 157 157 tF (s) 2.2 3.7 3.5 157 158 15
Upstream signal (m) 96 pX, platoon unblocked 0.89 vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 1361 644 vC2, stage 2 conf vol 644 692 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 6.2 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 0.0
pX, platoon unblocked 0.89 vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 1361 1361 vC2, stage 2 conf vol 644 644 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 6.2 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 0.0
vC, conflicting volume 1371 2004 692 vC1, stage 1 conf vol 1361 1361 vC2, stage 2 conf vol 644 644 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 6.2 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 0.0 <t< td=""></t<>
vC1, stage 1 conf vol 1361 vC2, stage 2 conf vol 644 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 5.2
vC2, stage 2 conf vol 644 vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 5.2 3.7 3.5 3.5 5.2 3.7 3.5 5.2 3.7 3.5 5.2 5.2 3.4 5.2 5.2 3.4 5.2
vCu, unblocked vol 1371 1875 692 tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 5.2 3.7 3.5 p0 queue free % 97 84 94 94 94 94 94 94 94 94 95 95 97 96 97 96 97 96 97 96 97
tC, single (s) 4.1 7.2 7.4 tC, 2 stage (s) 6.2 tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 0 26 Volume Right 0 0 0 0 0 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
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tF (s) 2.2 3.7 3.5 p0 queue free % 97 84 94 cM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
p0 queue free % cM capacity (veh/h) 97 birection, Lane # EB 1 birection EB 2 birection EB 3 birection WB 1 birection WB 2 birection SB 1 birection Volume Total 16 birection 16 birectio
CM capacity (veh/h) 502 162 334 Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
Direction, Lane # EB 1 EB 2 EB 3 WB 1 WB 2 SB 1 Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
Volume Total 16 612 612 893 466 46 Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
Volume Left 16 0 0 0 0 26 Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
Volume Right 0 0 0 0 20 20 cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
cSH 502 1700 1700 1700 1700 208 Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
Volume to Capacity 0.03 0.36 0.36 0.53 0.27 0.22 Queue Length 95th (m) 0.8 0.0 0.0 0.0 0.0 6.5
Queue Length 95th (m) 0.8 0.0 0.0 0.0 6.5
Control Delay (s) 12.4 0.0 0.0 0.0 0.0 27.1
, ()
Lane LOS B D
Approach Delay (s) 0.2 0.0 27.1
Approach LOS D
Intersection Summary
Average Delay 0.5
Intersection Capacity Utilization 43.2% ICU Level of Service
Analysis Period (min) 15

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations		7	1			†	
Traffic Volume (veh/h)	0	27	740	2	0	545	
Future Volume (Veh/h)	0	27	740	2	0	545	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	0	28	779	2	0	574	
Pedestrians	1						
Lane Width (m)	3.6						
Walking Speed (m/s)	1.2						
Percent Blockage	0						
Right turn flare (veh)							
Median type			TWLTL			TWLTL	
Median storage veh)			2			2	
Upstream signal (m)			63				
pX, platoon unblocked	0.89	0.89			0.89		
vC, conflicting volume	1355	781			780		
vC1, stage 1 conf vol	781						
vC2, stage 2 conf vol	574						
vCu, unblocked vol	1336	688			687		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4	<u> </u>					
tF(s)	3.5	3.3			2.2		
p0 queue free %	100	93			100		
cM capacity (veh/h)	369	398			811		
Direction, Lane #	WB 1	NB 1	SB 1		• • • • • • • • • • • • • • • • • • • •		
Volume Total	28	781	574				
Volume Left	0	0	0				
Volume Right	28	2	0				
cSH	398	1700	1700				
Volume to Capacity	0.07	0.46	0.34				
Queue Length 95th (m)	1.8	0.0	0.0				
Control Delay (s)	14.7	0.0	0.0				
Lane LOS	B	0.0	0.0				
Approach Delay (s)	14.7	0.0	0.0				
Approach LOS	В						
Intersection Summary							
Average Delay			0.3				
Intersection Capacity Utiliza	ation		49.1%	IC	U Level	of Service	Α
Analysis Period (min)			15				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			ર્લ	7		
Traffic Volume (veh/h)	0	8	14	28	9	0	
Future Volume (Veh/h)	0	8	14	28	9	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	
Hourly flow rate (vph)	0	10	17	35	11	0	
Pedestrians				2	1		
Lane Width (m)				3.6	3.6		
Walking Speed (m/s)				1.2	1.2		
Percent Blockage				0	0		
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				110110	110110		
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	81	13	11				
vC1, stage 1 conf vol	01	10					
vC2, stage 2 conf vol							
vCu, unblocked vol	81	13	11				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	٠.٦	0.2	7.1				
tF (s)	3.5	3.3	2.2				
p0 queue free %	100	99	99				
cM capacity (veh/h)	916	1065	1621				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	10	52	11				
Volume Left	0	17	0				
Volume Right	10	0	0				
cSH	1065	1621	1700				
Volume to Capacity	0.01	0.01	0.01				
Queue Length 95th (m)	0.2	0.3	0.0				
Control Delay (s)	8.4	2.4	0.0				
Lane LOS	Α	Α					
Approach Delay (s)	8.4	2.4	0.0				
Approach LOS	Α						
Intersection Summary							
Average Delay			2.9				
Intersection Capacity Utiliza	ation		19.6%	IC	CU Level o	of Service	Α
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	N.		1		7	†	
Traffic Volume (veh/h)	5	34	708	5	23	678	
Future Volume (Veh/h)	5	34	708	5	23	678	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	5	36	753	5	24	721	
Pedestrians			2				
Lane Width (m)			3.6				
Walking Speed (m/s)			1.2				
Percent Blockage			0				
Right turn flare (veh)							
Median type			TWLTL			None	
Median storage veh)			2				
Upstream signal (m)			206				
pX, platoon unblocked	0.97	0.97			0.97		
vC, conflicting volume	1526	756			758		
vC1, stage 1 conf vol	756						
vC2, stage 2 conf vol	771						
vCu, unblocked vol	1527	735			738		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4						
tF(s)	3.5	3.3			2.2		
p0 queue free %	99	91			97		
cM capacity (veh/h)	337	411			854		
Direction, Lane #	WB 1	NB 1	SB 1	SB 2			
Volume Total	41	758	24	721			
Volume Left	5	0	24	0			
Volume Right	36	5	0	0			
cSH	401	1700	854	1700			
Volume to Capacity	0.10	0.45	0.03	0.42			
Queue Length 95th (m)	2.7	0.0	0.7	0.0			
Control Delay (s)	15.0	0.0	9.3	0.0			
Lane LOS	С		Α				
Approach Delay (s)	15.0	0.0	0.3				
Approach LOS	С						
Intersection Summary							
Average Delay			0.5				
Intersection Capacity Utiliza	ation		47.6%	IC	U Level o	of Service	
Analysis Period (min)			15				

	٨	7	4	1	Ţ	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W		ሻ	↑	1,	52. (
Traffic Volume (veh/h)	57	4	26	641	604	64	
Future Volume (Veh/h)	57	4	26	641	604	64	
Sign Control	Stop	•		Free	Free	<u> </u>	
Grade	0%			0%	0%		
Peak Hour Factor	0.50	0.68	0.25	0.90	0.95	0.79	
Hourly flow rate (vph)	114	6	104	712	636	81	
Pedestrians			101	, , _	000	01	
_ane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				TWLTL	TW/LTI		
Median storage veh)				2	2		
Upstream signal (m)				156	2		
oX, platoon unblocked	0.95			150			
/C, conflicting volume	1596	676	717				
/C1, stage 1 conf vol	676	070	7 1 7				
/C2, stage 2 conf vol	920						
/Cu, unblocked vol		676	717				
•	1601	676					
C, single (s)	6.4	6.2	4.1				
C, 2 stage (s)	5.4	2.2	0.0				
F (s)	3.5	3.3	2.2				
o0 queue free %	61	99	88				
cM capacity (veh/h)	290	457	893				
Direction, Lane #	EB 1	NB 1	NB 2	SB 1			
/olume Total	120	104	712	717			
/olume Left	114	104	0	0			
Volume Right	6	0	0	81			
SH	295	893	1700	1700			
/olume to Capacity	0.41	0.12	0.42	0.42			
Queue Length 95th (m)	15.1	3.2	0.0	0.0			
Control Delay (s)	25.3	9.6	0.0	0.0			
ane LOS	D	Α					
Approach Delay (s)	25.3	1.2		0.0			
Approach LOS	D						
ntersection Summary							
Average Delay			2.4				
Intersection Capacity Utiliza	tion		45.7%	I	CU Level o	of Service	A
Analysis Period (min)	•		15	•			

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Lane Group	EBL	EBT	WBT	NBT	SBL	SBT
Lane Group Flow (vph)	311	785	1713	4	311	307
v/c Ratio	0.95	0.33	0.96	0.01	0.96	0.58
Control Delay	73.7	8.3	41.5	33.2	87.8	12.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.7	8.3	41.5	33.2	87.8	12.6
Queue Length 50th (m)	59.7	38.2	203.5	0.6	76.7	8.8
Queue Length 95th (m)	#117.3	47.9	#264.7	3.7	#134.8	37.5
Internal Link Dist (m)		142.5	28.0	29.3		38.7
Turn Bay Length (m)	15.0				27.0	
Base Capacity (vph)	327	2386	1784	382	323	530
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.95	0.33	0.96	0.01	0.96	0.58
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1		7	1			4		7	1	
Traffic Volume (vph)	295	745	1	0	1275	352	1	2	1	295	0	292
Future Volume (vph)	295	745	1	0	1275	352	1	2	1	295	0	292
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	2.0	5.3			5.3			6.3		5.3	6.3	
Lane Util. Factor	1.00	0.95			0.95			1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00			0.99			1.00		1.00	0.98	
Flpb, ped/bikes	1.00	1.00			1.00			1.00		1.00	1.00	
Frt	1.00	1.00			0.97			0.97		1.00	0.85	
Flt Protected	0.95	1.00			1.00			0.99		0.95	1.00	
Satd. Flow (prot)	1752	3504			3429			1805		1763	1464	
Flt Permitted	0.06	1.00			1.00			0.94		0.76	1.00	
Satd. Flow (perm)	116	3504			3429			1717		1402	1464	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	311	784	1	0	1342	371	1	2	1	311	0	307
RTOR Reduction (vph)	0	0	0	0	21	0	0	1	0	0	204	0
Lane Group Flow (vph)	311	785	0	0	1692	0	0	3	0	311	103	0
Confl. Peds. (#/hr)	8					8	6		1	1		6
Heavy Vehicles (%)	3%	3%	0%	0%	1%	1%	0%	0%	0%	2%	0%	8%
Turn Type	pm+pt	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases	5	2			6			8			4	
Permitted Phases	2			6			8			4		
Actuated Green, G (s)	80.7	80.7			60.7			26.7		26.7	26.7	
Effective Green, g (s)	81.7	81.7			61.7			26.7		27.7	26.7	
Actuated g/C Ratio	0.68	0.68			0.51			0.22		0.23	0.22	
Clearance Time (s)	3.0	6.3			6.3			6.3		6.3	6.3	
Lane Grp Cap (vph)	324	2385			1763			382		323	325	
v/s Ratio Prot	c0.14	0.22			c0.49						0.07	
v/s Ratio Perm	0.51							0.00		c0.22		
v/c Ratio	0.96	0.33			0.96			0.01		0.96	0.32	
Uniform Delay, d1	40.4	7.9			28.0			36.3		45.6	39.0	
Progression Factor	1.00	1.00			1.00			1.00		1.00	1.00	
Incremental Delay, d2	40.8	0.4			13.8			0.0		41.5	2.5	
Delay (s)	81.2	8.2			41.8			36.4		87.1	41.5	
Level of Service	F	Α			D			D		F	D	
Approach Delay (s)		29.0			41.8			36.4			64.5	
Approach LOS		С			D			D			E	
Intersection Summary												
HCM 2000 Control Delay			41.8	H	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	acity ratio		0.96									
Actuated Cycle Length (s)			120.0		ım of lost				13.6			
Intersection Capacity Utiliza	ation		99.0%	IC	U Level o	of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	^	1		Y		
Traffic Volume (veh/h)	32	1050	1612	26	11	11	
Future Volume (Veh/h)	32	1050	1612	26	11	11	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	34	1117	1715	28	12	12	
Pedestrians					8		
Lane Width (m)					3.6		
Walking Speed (m/s)					1.2		
Percent Blockage					1		
Right turn flare (veh)							
Median type		TWLTL	TWLTL				
Median storage veh)		2	2				
Upstream signal (m)		96					
pX, platoon unblocked					0.91		
vC, conflicting volume	1751				2364	880	
vC1, stage 1 conf vol					1737		
vC2, stage 2 conf vol					626		
vCu, unblocked vol	1751				2301	880	
tC, single (s)	4.1				6.8	6.9	
tC, 2 stage (s)					5.8		
tF (s)	2.2				3.5	3.3	
p0 queue free %	91				90	96	
cM capacity (veh/h)	360				123	293	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	SB 1	
Volume Total	34	558	558	1143	600	24	
Volume Left	34	0	0	0	0	12	
Volume Right	0	0	0	0	28	12	
cSH	360	1700	1700	1700	1700	173	
Volume to Capacity	0.09	0.33	0.33	0.67	0.35	0.14	
Queue Length 95th (m)	2.5	0.0	0.0	0.0	0.0	3.8	
Control Delay (s)	16.0	0.0	0.0	0.0	0.0	29.2	
Lane LOS	С					D	
Approach Delay (s)	0.5			0.0		29.2	
Approach LOS						D	
Intersection Summary							
Average Delay			0.4				
Intersection Capacity Utilizati	ion		55.4%	IC	U Level o	of Service	В
Analysis Period (min)			15				

	•	•	1	~	/	ļ	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations		7	1>			↑	
Traffic Volume (veh/h)	0	10	659	5	0	600	
Future Volume (Veh/h)	0	10	659	5	0	600	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly flow rate (vph)	0	10	679	5	0	619	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			TWLTL			TWLTL	
Median storage veh)			2			2	
Upstream signal (m)			63				
pX, platoon unblocked	0.93	0.93			0.93		
vC, conflicting volume	1300	682			679		
vC1, stage 1 conf vol	682						
vC2, stage 2 conf vol	619						
vCu, unblocked vol	1286	623			620		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	5.4						
tF (s)	3.5	3.3			2.2		
p0 queue free %	100	98			100		
cM capacity (veh/h)	394	457			905		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	10	684	619				
Volume Left	0	0	0				
Volume Right	10	5	0				
cSH	457	1700	1700				
Volume to Capacity	0.02	0.40	0.36				
Queue Length 95th (m)	0.5	0.0	0.0				
Control Delay (s)	13.1	0.0	0.0				
Lane LOS	В	0.0	3.0				
Approach Delay (s)	13.1	0.0	0.0				
Approach LOS	В	0.0	3.0				
Intersection Summary							
			0.1				
Average Delay	41		0.1	10	III avel	of Comile	
Intersection Capacity Utiliza	IUON		45.0%	IC	U Level	of Service	
Analysis Period (min)			15				

	٨	•	1	1	1	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			ર્લ	ĵ.		
Traffic Volume (veh/h)	0	23	23	29	2	0	
Future Volume (Veh/h)	0	23	23	29	2	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	25	25	32	2	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				110110	110110		
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	84	2	2				
vC1, stage 1 conf vol	0-1						
vC2, stage 2 conf vol							
vCu, unblocked vol	84	2	2				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.4	0.2	7.1				
tF (s)	3.5	3.3	2.2				
p0 queue free %	100	98	98				
cM capacity (veh/h)	908	1088	1634				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	25	57	2				
Volume Left	0	25	0				
Volume Right	25	0	0				
cSH	1088	1634	1700				
Volume to Capacity	0.02	0.02	0.00				
Queue Length 95th (m)	0.6	0.4	0.0				
Control Delay (s)	8.4	3.2	0.0				
Lane LOS	Α	Α					
Approach Delay (s)	8.4	3.2	0.0				
Approach LOS	Α						
Intersection Summary							
Average Delay			4.7				
Intersection Capacity Utiliza	ation		19.5%	IC	CU Level o	of Service	Α
Analysis Period (min)			15				

APPENDIX J Parking Justification Supporting Documents

ONTARIO TRAFFIC INC - PARKING SURVEY

<u>Location:</u> 25 Agnes Street, Mississauga

	Underground	Tenant	100	Available	35	33	30	56	23	21	21	20	20	19	19	19
	Under	Ter	1	Parked	9	29	20	74	77	79	62	80	80	81	81	81
		Disables		Available	1	1	1	2	1	1	1	1	1	1	1	1
mber 18, 201		Reserved Disables	2	Parked	1	1	1	0	1	1	1	1	1	1	1	1
Monday, November 18, 2019	Surface	ant	9	Available	4	4	2	2	4	4	4	4	4	3	3	3
N	Sur	Tenant		Parked	2	2	1	1	2	2	2	2	2	3	3	3
		Visitor	17	Available	4	3	2	3	4	2	0	1	1	2	2	4
				Parked	13	14	15	14	13	15	17	16	16	15	15	13
	Underground	Tenant	100	Available	43	41	32	34	33	21	24	11	13	15	17	17
				Parked	22	29	9	99	29	79	92	68	87	85	83	83
	ace	Disables	2	Available	2	2	2	1	1	1	1	1	0	1	1	0
nber 17, 2019		Reserved Disabl		Parked	0	0	0	1	1	1	1	1	2	1	1	2
Sunday, Novem		Visitor Tenant Tenant	9	Available	2	2	2	4	4	5	2	4	3	3	3	4
;	Surface		9	Parked	1	1	1	2	2	1	1	2	3	3	3	7
			17	Available	2	2	1	3	3	4	2	2	1	1	2	3
		Vis	1	Parked	15	15	16	14	14	13	12	15	16	16	15	14
					19:30	20:00	20:30	21:00	21:30	22:00	22:30	23:00	23:30	00:00	00:30	01:00
Time				to	to	to	to	to	to	to	to	to	to	to	to	
					19:00	19:30	20:00	20:30	21:00	21:30	22:00	22:30	23:00	23:30	00:00	00:30

Wednesday, November 27, 2019	Underground	Tenant	100	Available	33	24	22	12	11	13	12	11	14	16	17	18
	Under	Ten		Parked	29	92	78	88	68	87	88	68	98	84	83	82
	Surface	Disables	2	Available	2	2	1	2	1	1	2	2	1	1	0	0
		Reserved Disables		Parked	0	0	1	0	1	1	0	0	1	1	2	2
ednesday, Nov		Tenant	9	Available	2	2	2	2	4	2	2	2	2	3	3	2
We				Parked	1	1	1	1	2	1	1	1	1	3	3	1
		Visitor	17	Available	3	4	3	4	2	3	0	0	3	4	4	3
		Visi		Parked	14	13	14	13	12	14	17	17	14	13	13	14
	Underground	Tenant	100	Available	33	53	23	54	22	70	17	17	14	17	18	17
				Parked	29	71	77	92	78	80	83	83	98	83	82	83
6	Surface	Reserved Disables	2	Available	7	7	1	7	1	1	1	7	1	1	1	7
mber 25, 201		Reserve		Parked	0	0	1	0	1	1	1	0	1	1	1	0
Monday, November 25, 2019		Tenant	17 6	Available	2	2	2	4	8	8	8	8	8	7	7	2
J				Parked	1	1	1	7	3	3	3	3	3	4	4	1
		Visitor		Available	2	2	0	2	4	3	2	3	4	2	4	2
		Nis		Parked	15	15	17	12	13	14	15	14	13	12	13	12
					19:30	20:00	20:30	21:00	21:30	22:00	22:30	23:00	23:30	00:00	00:30	01:00
Time					to											
						19:30	20:00	20:30	21:00	21:30	22:00	22:30	23:00	23:30	00:00	00:30

ONTARIO TRAFFIC INC - PARKING SURVEY

<u>Location:</u> 2929 Aquitaine Avenue, Mississauga

	Underground	R.W.I.		Parked Available Parked	4 3 133	5 2 139	6 1 144	7 0 147	5 2 149	5 2 150	5 2 153	6 1 154	6 1 155	6 1 155	6 1 156	6 1 156
Monday, November 18, 2019		Commercial	8	Parked Available	5 3	5 3	4 4	4 4	4 4	4 4	4 4	5 3	5 3	5 3	5 3	2
Mo	Surface	Tenant	11	Parked Available	7 4	8	9 2	9 2	10 1	10 1	10 1	10 1	10 1	10 1	10 1	10 1
		A.C.C.	2	Available	2	1	1	1	0	0	0	0	0	0	0	U
		Tenant	207	Available Parked	89	69 4	70 4	68 4	99	9 29	62 5	9 09	5 65	58 5	2 29	5.
	Underground	R.W.I.		Available Parked	1 139	1 138	2 137	2 139	1 141	1 142	0 145	1 147	1 148	1 149	0 151	151
17, 2019				ailable Parked	2 6	2 6	3 2	3 2	2 6	2 6	1 7	2 6	2 6	2 6	1 7	1 7
Sunday, November 1		Commercial	8	le Parked Avail	9	9	2	2	9	9	7	9	9	9	7	7
	Surface	Tenant	11	Parked Available	10 1	10 1	10 1	10 1	11 0	11 0	11 0	9 2	9 2	10 1	11 0	11 0
		A.C.C.	5	Parked Available	0 9	0 9	0	0	0	0 9	0 9	0 9	0 9	0 9	0	0
				Time Park	to 19:30 5	to 20:00 5	to 20:30 5	to 21:00 5	to 21:30 5	to 22:00 5	to 22:30 5	to 23:00 5	to 23:30 5	to 00:00 5	to 00:30 5	700100
				ŢĬ	19:00 to	19:30 to	20:00 to	20:30 to	21:00 to	21:30 to	22:00 to	22:30 to	23:00 to	23:30 to	00:00 tc	08:00

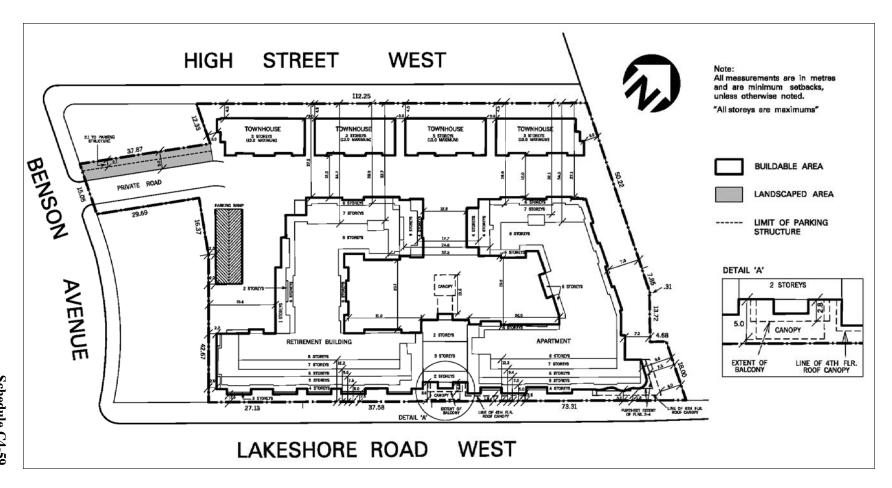
Columbia	_		
Monday, November 25, 2019 Underground Tenant Surface Acron and a surface Acron and a surface Acron and a surface Acron and a surface Acro and a surface <th< th=""><th rowspan="3">Underground</th><th>52 52 52 51 52 51 58 58</th><th>54 54</th></th<>	Underground	52 52 52 51 52 51 58 58	54 54
Act		155 156 155 155 156 149 149	152 153 153
Act		4 4 4 2 2 2 2 2 0 0 0 0	0 0 0
Accordany, November 25, 2019		x x x x x x x x x x x x x x x x x x x	7 7
Accordany, November 25, 2019		2 2 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	3 2 3
Accorday, November 25, 2019 Accorday, November 25, 2019 Accorday, November 25, 2019 Accorday, November 25, 2019 Accordance	or (Amoonin	0 7 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 9 2
Monday, November 25, 2019 Monday, November 25, 2019 Independence of the color of		7 1 1 0 0 0 3 3	2 2 2
ACC Tenant Tena	J	8 8 11 11 11 10 10	6 6
ACC Surface Nonday, November 25, 2019 Nonderground Tenart Tenar		. 1 1 1 2 3 2 3 3	0 1
Surface Monday, November 25, 2019 Monday, November 2		2 2 2 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	5 4
ACC		53 40 41 42 42 51 52 42 52	59 60 62
ACC Tenant Commercial R.W.I.		154 167 166 165 156 155 165	148 147 145
AC.C. Surface Monday, November 25, 2019		3 7 7 7 3 3 3	2 2 3
ACC. Tenant Commercia		4 4 4 5 5 5 5 5	5 5
ACC Surface Monday		4 4 8 4 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	3 4
ACC Tenant	San (Amana)	4 4 5 4 5 9	5 4
AC.C. State AC.C. State Stat		0 0 0 0 1 1 1 3 3	2 2
Parted 5 Par		8 8 10 10 11 11 11	9 9
19:30 4 20:30 4 20:30 4 4 22:30 4 4 22:30 4 4 22:30 4 4 22:30 4 4 22:30 3 3 23:30 4 4 23:30 4 4 00:00 4 4		1 1 1 0 0 1	1 2 2
		4 7 4 4 4 6 4 4	4 8 8
Time to		20:00 20:30 21:00 21:30 22:00 22:30 23:00 23:30	00:30
		0 0 0 0 0 0	t to
19:00 19:30 20:00 20:30 21:30 21:30 22:30 23:30 23:30		19:30 20:00 20:30 21:00 21:30 22:30 23:00	23:30

6.2.5.59	Exception: C4-59	By-law: 0281-2015, 0174-2017, 0130-2018, 0181-2018/LPAT Order 2019 February 15				
In a C4-59 zon uses/regulation		shall be as specified for a C4	zone except that the following			
Permitted Use	es					
6.2.5.59.1	Lands zoned C4-59 shall or (1) Apartment (2) Retirement Build (3) Assisted Living D (4) Townhouse (5) Financial Institut (6) Office (7) Medical Office (8) Personal Service (9) Repair Establish (10) Retail Store (11) Restaurant (12) Take-out Restaur	welling Unit ion Establishment nent				
	(13) Outdoor patio acce	essory to a restaurant or				
	take-out restaura	nt				
Regulations						
6.2.5.59.2		a Subsections 2.1.2, 2.1.25 an 2.4, 14.0 and 16.0 contained shall not apply				
6.2.5.59.3	Maximum number of dwell	ling units	325			
6.2.5.59.4	Maximum number of townl	nouse dwelling units	16			
6.2.5.59.5	For the purposes of this By- considered one lot	-law, all lands zoned C4-59 si	hall be			
6.2.5.59.6	Maximum floor space inde	ex - apartment zone	2.4			
6.2.5.59.7	Maximum gross floor area - non- residential used for an office, medical office, financial institution, personal service establishment, repair establishment, restaurant, take-out restaurant or retail store, or any combination thereof					
6.2.5.59.8	service establishment, rep take-out restaurant or reta	inancial institution, personalie establishment, restaurale ail store, or any combination irst storey of a retirement backeshore Road West	nt, thereof,			
6.2.5.59.9		losures providing direct access included in the number of st				
6.2.5.59.10	Minimum landscaped area	1	35%			
6.2.5.59.11	The lot line abutting Lakeshore Road West shall be deemed to be the front lot line					
6.2.5.59.12	Minimum number of resident parking spaces per one-bedroom or two-bedroom apartment dwelling unit					
6.2.5.59.13	Minimum number of visitor parking spaces per apartment dwelling unit 0.19					
6.2.5.59.14	Minimum number of parki retirement dwelling unit	ng spaces per	0.40			
6.2.5.59.15	Minimum number of parking spaces per assisted living dwelling unit 0.33					

Exception C4-59 continued on next page

6.2.5.59	Excep	tion: C4-59	Map # 08		, 0130-2018, LPAT Order
Exception C4	1-59 cont	tinued from previous	page		
6.2.5.59.16	area - institu	non-residential for a ation, personal service	g spaces per 100 m ² gross and office, medical office, fing e establishment, repair taurant or retail store		1.0
6.2.5.59.17		num number of parkin non-residential for a	g spaces per 100 m² gross t restaurant	floor	7.65
6.2.5.59.18	Minim	num number of loadin	g spaces		1
6.2.5.59.19	Minimum setback from a parking structure completely below or above finished grade to a lot line unless otherwise identified on Schedule C4-59 of this Exception				0.0 m
6.2.5.59.20		aximum projections poe in compliance with	ermitted beyond the buildab the following:	ole area	
	(1)	porch			1.5 m
	(2)	awnings			1.5 m
	(3)		and other architectural elem- undation, such as but not lin and corbel		1.0 m
	(4)	balcony			1.8 m
6.2.5.59.21	Stairs, walkways, planters and ventilation shafts are permitted to encroach into a required landscaped area and are permitted outside the buildable area identified on Schedule C4-59 of this Exception				
6.2.5.59.22	"Assisted Living Dwelling Unit" means a retirement dwelling unit within a retirement building occupied by persons with memory and/or cognitive impairment where a higher level of supervised support and care is provided compared to the rest of the retirement building , but whose residents do not require the services and support provided in a long-term care building				
6.2.5.59.23		e development plans s sception	hall comply with Schedule	C4-59 of	

Page 6.2.5 ~ 43





A by-law to amend By-law Number 0225-2007, as amended.

WHEREAS pursuant to sections 34 and 36 of the *Planning Act*, R.S.O. 1990, c.P.13, as amended, the council of a local municipality may, respectively, pass a zoning by-law and enact a by-law to impose a holding provision;

NOW THEREFORE the Council of The Corporation of the City of Mississauga ENACTS as follows:

1. By-law Number 0225-2007, as amended, being a City of Mississauga Zoning By-law, is amended by adding the following Exception Table:

4.15.6.53	Exception: RA5-53	Map # 08	By-law:			
In a RA5-53 zone the permitted uses and applicable regulations shall be as specified for a RA5 zone except that the following uses /regulations shall apply:						
Additional Pe	ermitted Uses					
4.15.6.53.1	(1) Uses permitted i Table 6.2.1 of th	n a C4 zone as contained in is By-law	n			
Regulations						
4.15.6.53.2 The provisions of Article 2.1.30.1 contained in Subsection 2.1.30 and Lines 11.1, 11.2, 13.3, 15.5 contained in Table 4.15.1 of this By-law shall not apply						
4.15.6.53.3	The uses contained in Sentence 4.15.6.53.1 of this Exception shall only be permitted on the first storey of an apartment					
4.15.6.53.4	Maximum floor space	Maximum floor space index - apartment zone 9.2				
4.15.6.53.5	Minimum gross floor a	area - non-residential	250 m^2			
4.15.6.53.6	Maximum gross floor area - apartment zone for each of the 13 th and 14 th storeys					
4.15.6.53.7		f all balconies located aborrom the outermost faces of				
4.15.6.53.8	Notwithstanding Sentence 4.15.6.53.7 of this Exception, maximum projection of a rooftop balcony on the second storey measured from the outermost face of the building					
4.15.6.53.9	Notwithstanding Senter maximum projection of the seventh storey mean the building	lon				

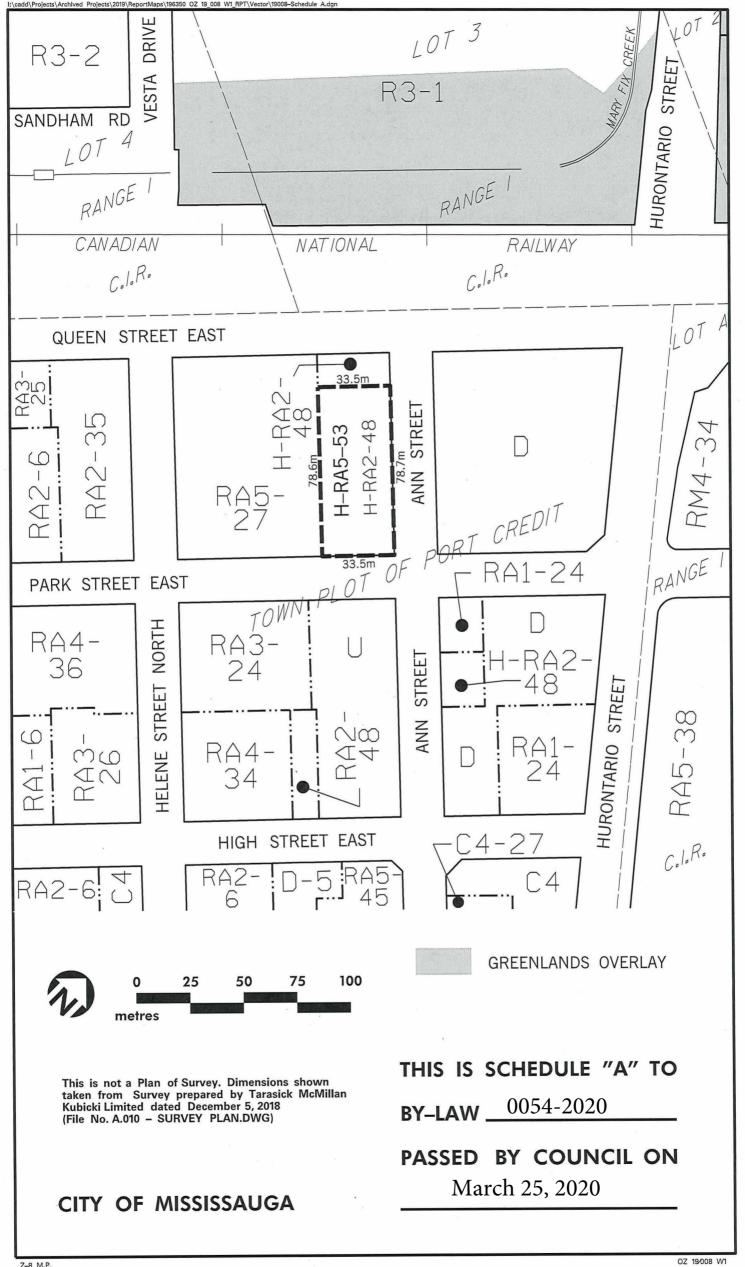
4.15.6.53	Exception: RA5-53 Ma	up # 08	By-law:			
4.15.6.53.10	Notwithstanding Sentence 4.15.6.53.7 of this Exception, maximum projection of a rooftop balcony located on the ninth storey measured from the outermost face of the building					
4.15.6.53.11	maximum projection of a roo	Notwithstanding Sentence 4.15.6.53.7 of this Exception, maximum projection of a rooftop balcony located on the 15 th storey measured from the outermost face of the building				
4.15.6.53.12	Maximum projection of an architectural feature located above the sixth storey measured from the outermost face of the building					
4.15.6.53.13	External access stairwell and permitted to extend beyond below grade parking struct					
4.15.6.53.14	Notwithstanding Sentence 4.15.6.53.24 of this Exception, maximum projection of a canopy facing Park Street East from the building face			2.2 m		
4.15.6.53.15	Notwithstanding Sentence 4.15.6.53.24 of this Exception, maximum projection of a canopy facing Ann Street from the building faces			2.2 m		
4.15.6.53.16	Notwithstanding Sentence 4 stairs and ramps shall be per buildable area and encroach	the				
4.15.6.53.17	Minimum number of resider one-bedroom apartment dwe	per	0.75			
4.15.6.53.18	Minimum number of resider two-bedroom apartment dw	per	0.90			
4.15.6.53.19	Minimum number of resident parking spaces per three-bedroom apartment dwelling unit			1.10		
4.15.6.53.20	Minimum number of shared visitor and non-residential parking spaces per dwelling unit			0.10		
4.15.6.53.21	Minimum aisle width			6.6 m		
4.15.6.53.22	Minimum landscaped area	380 m^2				
4.15.6.53.23	Minimum amenity area	1 300 m ²				
4.15.6.53.24	All site development plans s Schedule RA5-53 of this Ex					

4.15.6.53	Excep	tion: RA5-53 Map # 08 By-law:						
Holding Pr	ovision							
	or any amend Part 13	The holding symbol H is to be removed from the whole or any part of the lands zoned H-RA5-53 by further amendment to Map 08 of Schedule B contained in Part 13 of this By-law, as amended, upon satisfaction of the following requirements:						
	the for	the following requirements.						
	(1)	delivery of an executed Development Agreement in a form and on terms satisfactory						
	(2)	to the City of Mississauga (the City); submission of grading and servicing drawings to City standards and specifications satisfactory						
	(3)	to the City; submission of an updated Functional Servicing Report and Traffic Impact Study satisfactory to						
	(4)	the City; submission of a Phase II Environmental Site Assessment Reports and all supporting documents, including a Letter of Reliance,						
	(5)	satisfactory to the City; submission of Final Remediation Report, Site Remediation Securities and a Dewatering Plan, including a Letter of Reliance, satisfactory to						
	(6)	the Transportation and Works Department; Record of Site Condition for lands to be dedicated to the City and all supporting						
	(7)	documents, including Letter of Reliance satisfactory arrangements with the Region of Peel for Waste Collection subject to the most						
	recent Waste Collection Design Standards; (8) confirmation by the Region of Peel that satisfactory arrangements have been made for							
	(9)	water and waste water services to the site; a letter from the Planning and Building Department indicating satisfactory arrangements						
	(10)	have been made with respect to addressing the City's Housing Strategy; and, delivery of an executed agreement for community benefits pursuant to section 37 of the <i>Planning Act</i> , as amended, in a form and on terms satisfactory to the City.						

2. Map Number 08 of Schedule "B" to By-law Number 0225-2007, as amended, being a City of Mississauga Zoning By-law, is amended by changing thereon from "H-RA2-48" to "H-RA5-53", the zoning of Part of the Town Plot of Port Credit, in the City of Mississauga, PROVIDED HOWEVER THAT the "H-RA5-53" zoning shall only apply to the lands which are shown on the attached Schedule "A", which is deemed to be an integral part of this By-law, outlined in the heaviest broken line with the "H-RA5-53" zoning indicated thereon.

ENACTED and PASSED this 25	day of
	Bonie Chombre
ADDROVED	MAYOR
APPROVED AS TO FORM City Solicitor	CLERK

MISSISSAUGA



APPENDIX "A" TO BY-LAW NUMBER 0054-2020

Explanation of the Purpose and Effect of the By-law

The purpose of this By-law is to permit a 22 storey apartment building with ground floor commercial uses and a FSI of 9.2.

This By-law amends the zoning of the property outlined on the attached Schedule "A" from "H-RA2-48" (Apartment – Exception with a Holding Provision) to "H-RA5-53" (Apartment – Exception with a Holding Provision).

"H-RA2-48" permits an 8 storey apartment building with an FSI of 1.0.

Upon removal of the "H" provision, the "RA5-53" zone will permit a 22 storey apartment building with ground floor commercial uses and a FSI of 9.2

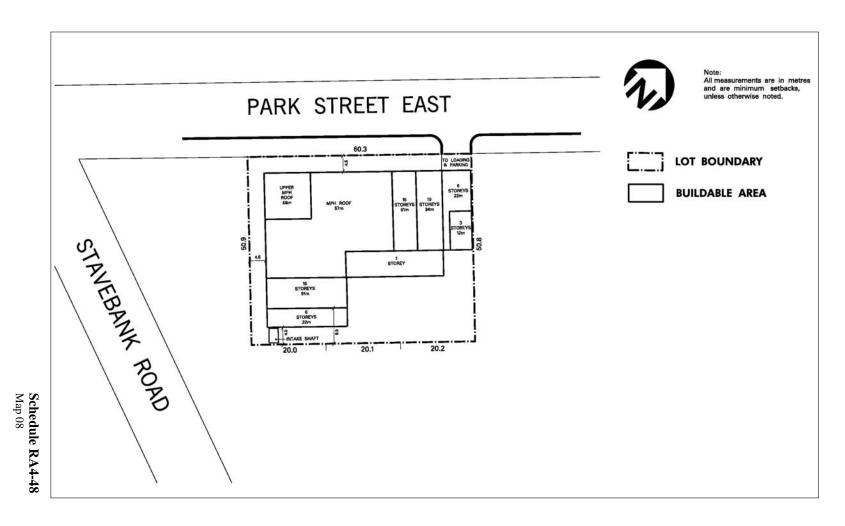
Location of Lands Affected

Northwest corner of Ann Street and Park Street East, in the City of Mississauga, as shown on the attached Map designated as Schedule "A".

Further information regarding this By-law may be obtained from David Ferro of the City Planning and Building Department at 905-615-3200 ext. 4554.

http://teamsites.mississauga.ca/sites/18/bylaws/oz 19 008 w1.by-law.df.fs.docx

4.15.5.48	Exception: RA4-48	Map # 08	By-law: 0174 0142-2019	-2018,		
In a RA4-48 zo except that the	one the permitted uses and ap following uses /regulations sh	plicable regulations shall be all apply:	as specified for	a RA4 zone		
Regulations						
4.15.5.48.1	The provisions of Article 4. Lines 11.2, 13.5, 15.1, 15.2, Table 4.15.1 of this By-law					
4.15.5.48.2	Maximum floor space inde	X		6.3		
4.15.5.48.3	O	Maximum gross floor area - apartment zone per storey for a 200 m ² each storey above 12 storeys				
4.15.5.48.4	Minimum front yard			4.5 m		
4.15.5.48.5	Stairs, ramps, planters, cano encroach into a required fro	opies and patios shall be pern	nitted to			
4.15.5.48.6	Minimum number of reside apartment dwelling unit	nt parking spaces per one-bo	edroom	0.8		
4.15.5.48.7	Minimum number of reside apartment dwelling unit	nt parking spaces per two-b	edroom	1.0		
4.15.5.48.8	Minimum number of reside apartment dwelling unit	nt parking spaces per three-	bedroom	1.3		
4.15.5.48.9	Minimum number of parking dwelling unit	ng spaces per grade related a	partment	1.3		
4.15.5.48.10	Minimum number of visitor dwelling unit	parking spaces per apartme	ent	0.1		
4.15.5.48.11		arking structure completely external access stairwells, to		4.5 m		
4.15.5.48.12	Minimum landscaped area	1		780 m^2		
4.15.5.48.13	Minimum depth of a landso	caped buffer abutting an OS	1 zone	4.5 m		
4.15.5.48.14	Minimum central amenity	area		930 m^2		
4.15.5.48.15	All site development plans of this Exception	shall comply with Schedule I	RA4-48			



4.15.6.45	Exception: RA5-45	Map # 08	By-law: 0103-2017, 0174-2017
	one the permitted uses and app following uses /regulations sh		s specified for a RA5 zone
Additional Pe	rmitted Use		
4.15.6.45.1	(1) Semi-Detached		
Regulations			
4.15.6.45.2	Minimum lot frontage		25.0 m
4.15.6.45.3	Maximum floor space inde	x - apartment zone	4.3
4.15.6.45.4	Maximum height		15 storeys
4.15.6.45.5	Maximum number of apartn	nent dwelling units	69
4.15.6.45.6	Maximum number of semi-	letached dwelling units	2
4.15.6.45.7	Minimum front yard		4.0 m
4.15.6.45.8	Minimum exterior side yar	d	2.9 m
4.15.6.45.9	Minimum interior side yar	d to a semi-detached	2.9 m
4.15.6.45.10	Minimum interior side yar	d to an apartment	3.2 m
4.15.6.45.11	Minimum rear yard		6.9 m
4.15.6.45.12		alcony located above the first st face or faces of the building	
4.15.6.45.13		alcony located above the first st face or faces of the building into a required rear yard	
4.15.6.45.14		nopies, ventilation shafts and ncroach into a required yard a	
4.15.6.45.15	Minimum separation between	en buildings	5.9 m
4.15.6.45.16	Minimum number of resider	nt parking spaces per dwellin	g unit 1.0
4.15.6.45.17	Minimum number of visitor	parking spaces per dwelling	unit 0.15
4.15.6.45.18	Minimum setback from a pa finished grade to any lot lin	arking structure completely be	pelow 0.2 m
4.15.6.45.19	Minimum landscaped area		30%
4.15.6.45.20	Minimum depth of a landsc High Street East	aped buffer abutting	4.0 m
4.15.6.45.21	Minimum depth of a landsc	aped buffer abutting Ann Str	eet 2.9 m
4.15.6.45.22	Minimum depth of a landsc lot line	aped buffer along an interior	• side 0.0 m
4.15.6.45.23	Minimum depth of a landsc	aped buffer along a rear lot l	line 0.5 m
4.15.6.45.24	Minimum amenity area		340 m^2

Exception RA5-45 continued on next page

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4.15.6.45	Excepti	on: RA5-45	Map # 08	By-law: 0103-2017, 0174-2017				
Exception RA	5-45 con	tinued from previo	us page					
Section 37 Pul	Section 37 Public Benefits Contribution							
	Pursuar as amer this Exc	the owner of the lar agreement with The Mississauga (the Cfacilities, services cheight and density of the agreement is regRA5-45;	nds zoned RA5-45 enters into e Corporation of the City of ity) for the provision of certain or matters in return for the inco of the development; gistered on title to the lands z	ded by an reased oned				
	(3)	toward the redevelor Park and/or renovati and/or the installati	City the sum of \$300,000 to opment of the Port Credit Certions to the West Bank Toten on of a commemorative statury of Port Credit at J.J. Plaus	otaph n Pole e				

4.15.6.46	Exception: RA5-46	Map # 22	By-law: 0155-2016/ OMB Order 2017 January 11, 0174-2017, 0179-2018		
	one the permitted uses and following uses /regulation		shall be as specified for a RA5 zone		
Additional Pe	ermitted Uses				
4.15.6.46.1	(1) Day Care (2) Retail Store				
Regulations					
4.15.6.46.2		d in Subsection 2.1.14, 2, 15.2 and 15.3 in Table			
4.15.6.46.3	For the purposes of this considered one lot	By-law, all lands zoned	RA5-46 shall be		
4.15.6.46.4	within a building , struc	entence 4.15.6.46.1 shale ture or part thereof, use care building, retirement	d for an		
4.15.6.46.5		of dwelling units in Bu RA5-46 of this Exception			
4.15.6.46.6	Maximum total number of dwelling units in Buildable Area 'B' identified on Schedule RA5-46 of this Exception				
4.15.6.46.7	Maximum total number of dwelling units in Buildable Areas 'C1' 522 and 'C2' identified on Schedule RA5-46 of this Exception				
4.15.6.46.8	Maximum floor space index - apartment zone , measured over the lot area prior to road widening(s) 8.2				
4.15.6.46.9	Maximum total gross floor area - apartment zone in Buildable 24 450 m ² Area 'A' identified on Schedule RA5-46 of this Exception				
4.15.6.46.10	Maximum total gross floor area - apartment zone in Buildable Area 'B' identified on Schedule RA5-46 of this Exception				

Exception RA5-46 continued on next page

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Dumitru Liubeznii

From: Jim Levac <jiml@gsai.ca>

Sent: Tuesday, May 12, 2020 10:51 AM

To: Rao Marthi; chriszeppa; Bruce McCall-Richmond; Berardo Graziani; Rob Lincoln

Subject: RE: Lakeshore

Rao:

I don't have the plan in front of me but I think we were pretty much using the same standard that is being proposed on the Plazacorp condo at 420 Lakeshore. See below.

	PARKING REQUESTED (1)(2)(3)	
	RATIO	SPACE
RESIDENTIAL	1 / UNIT	172
COMMERCIAL		3
VISITOR(CONDO APARTMENT)	0.15/UNIT	26
TOTAL		201

PARKING # PROVIDED

SURFACE	7
U//G1	97
U/G 2	97
	201

NOTE

1)ASSUMING RETAIL PARKING RATE REQUEST =1/100M2 2)ASSUMING RESIDENTIAL PARKING RATE=1/UNIT 3)ASSUMING VISITOR PARKING =8 SPACES

FSI: 5.17

UNITS: 172

TOTAL GFA: 19,382m²