PPROSED GAS STATION

1480 DERRY ROAD EAST

MISSISSAUGA, ONTARIO

FUNCTIONAL SERVICING REPORT

FEBRUARY 21, 2020

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1.0 Background

n Architecture Inc. has been retained by Vicky Aulakh, Probh Aulakh Ltd. to prepare a Functional Servicing Report for the 1480 Derry Road East, Mississauga ON, a commercial located at the corner of the south side of Derry Road East and the west of Dixie Road in city of Mississauga.

The purpose of the Functional Servicing Report (FSR) is to evaluate the servicing demands and propose feasible connection and disposal points for municipal services. The report also evaluates the impact of proposed development in terms of stormwater requirements as per city and region criteria.

The subject site is bounded by Derry Road East to the north side, Dixie Road on the east side, existing commercial property on the west side and vacant land on the south side. Refer to location shown below as Figure 1.



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Figure 1 - Site Location Plan

2.0 Existing Site Condition



Figure 2 – Existing Condition

3.0 Land Use & Legal Description

The project site area is 0.42 acres (0.17 hectares). Currently the site has a parking lot covered with gravel and partly covered with grass.

The subject site's legal description is Part of LOT 10, COSESSION 3, EAST OF HURONTARIO STREET, 43R31711, PTS 5,6 (Geographical Township of Toronto), City of Mississauga, Regional Municipality of Peel. (Refer to Plan of Survey dated June 12, 2017 attached in Appendix A).

4.0 Drainage & Topography

Existing topography shows that this is relatively flat land with the north-east corner slopes towards south-west corner of the site. The difference between elevations of north-east to south-west is around 0.08m.

4.1 Existing Services

Existing water and storm services are located on Derry Road East. There are no sanitary services along the property line of Derry Road East and Dixie Road. Closest sanitary sewer is on Derry

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Road East around 200m east of the property line. Record drawings obtained from Region of Peel and City of Mississauga shows drawings show existing services in plan and profile as summarized below in Table 1.

Table 1 – Existing Services Summary

Service	Size &Material	Location
Water	750m CONC.	Dixie Road (50 mm Plug on Site)
Storm	450mm CONC.	Derry Road East, South Side
Sanitary	1050mm CONC.	Derry Road East Approx. 200m East of Property Limit

5.0 Proposed Development

The development proposal is for a 3 pump canopied gas station with two story C-store and office building parking spaces. The site layout is shown on Figure A 1.0 in Appendix C.

5.1 Design Criteria

The summary of design criteria shown in Table 2 has been used for the water demand and sanitary flow calculations expected from the proposed development.

6.0 Servicing Demand Calculations

6.1 Domestic Water Demands

As per Region of Peel – a custom demand calculation method followed considering the nature of the development. To calculate site specific water demand, OBC guideline used as follows:

(1)	Average water demand for Office Space: <i>O.B.C</i> (8.2.1.3.B.12.v) No of Washroom Average Water Demand:	950 L/day per Washroom 1 950 L/day
(3)	Average flow a Store Per m^2 of floor area:	5 L/Day
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	<i>O.B.C</i> (8.2.1.3.B.20) Proposed C-Store Area: Average Water Demand:	192.57 m ² 192.57 x 5 = 962.85 L/ day
(3)	Average flow per Nozzle: <i>O.B.C</i> (8.2, 1.3, <i>B</i> , 19, <i>i</i>)	560 l /nozzle/day
	Number of Nozzle:	6
	Average Water Demand:	560 x 6 L/day = 3360 L/day

Total Average water demand = 950 + 962.85 + 3360 = 5272.3 L/day

As per MOECC standards, a Maximum Day Factor of 2.0 and peak hourly demand Factor of 4.5 will be applied to the average day flows;

Maximum day demand = $5372.3 \times 2.0 = 10545.7 \text{ L/day} = 0.122 \text{ L/sec}$ Maximum hour demand (AM) = $5372.3 \times 4.5 = 23727.8 \text{ L/day} = 0.275 \text{ L/sec}$

WATER SYSTEM DESIGN CRITERIA			
Commercial Average Demand	300 Lpcd ¹		
Max Day Factor	2.0		
Peak Flow	Max Day + Fire Flow		
Fire Flow Calculation Method	FUS Method		
Min System Pressure	20 psi under Max day +		
WASTEWATER SYSTEM DESIGN CRITERIA			
Generation Rate	0.013 m ³ /sec. ²		
Peaking Factor	Included		
Equivalent Population for Commercial	50 person per hectare ³		
Development			

Table 2 – Water & Sanitary Design Criteria

³ Section 2.1, page3, Region of Peel Design Criteria Manual – Sanitary Sewer

¹ Section 2.3, page 4, Region of Peel watermain design criteria ² Region of Peel STD DWG 2-9-2 (Sewage Flows (excluding Infiltration, Region of Peel Design Criteria Manual – Sanitary Sewer

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Extraneous Flows (I/I)	0.0002 m ³ /sec/ha ⁴
Allowance for Maintenance Hole	0.000028 m ³ /sec/mh

6.1.1 Fire Flow

A fire flow demand for the proposed building has been calculated as per guidelines of Fire Underwriter Survey. Based on the type of construction, total floor areas and other fire suppression related information, fire flow demands have been calculated as 2000 L/min (33 l/sec). Refer to Table B1, Appendix B for detailed analysis.

6.1.2 Peak Flow

The peak flow calculated by adding Max Day Flow + Fire Flow = 0.122 + 33.0 = 33.122 L/sec.

7.0 Waste Water Generation

Based on the Region of Peel's criteria, waste water flow calculated as follows:

(1) Commercial waste water generation rate:	$0.013 \text{ m}^3/\text{sec.}$
(2) Extraneous Flow:	0.0002 m ³ /sec/ha
(3) Lot Area:	0.17 m^2
(4) Site's extraneous flow:	$0.0002 \text{ x } 0.17 = 0.000034 \text{ m}^3/\text{sec}$
(5) Maintenance Hole allowance:	0.000028 m ³ /sec/MH
(6) No. of MH:	4
(7) Total Maintenance Hole allowance:	$0.000028 \text{ x } 4 = 0.000192 \text{ m}^3/\text{sec}$
(8) Waste water flow from the site including infiltrat	tion allowance is calculated $(1) + (4) +$
(7) as 1142.72 L/day (0.0132 L/sec).	

⁴ Section 2.3, page 4, Region of Peel Design Criteria Manual – Sanitary Sewer **n Architecture Inc.** Architects and Civil Engineers

8.0 Proposed Servicing

The following service connections are proposed from the existing municipal infrastructure on Derry Road East. The services will be installed according to city/Region standards.

8.1 New Sanitary Connection

A new sanitary connection will be provided from the existing 1050mm diameter sanitary main on Derry Road. Two manholes along with 300 diameter sewer on Derry Road proposed to install to connect the site. The location of proposed sanitary service connection is shown in Figure C2 and Figure C2a (Appendix C). Sanitary Design Sheet attached in Appendix C.

8.2 New Domestic / Fire Water Connection

New 50mm service connections are proposed to be connecting to existing water connection plug on Derry Street East. The location of proposed water service connections are shown on Figure C2, Appendix C.

8.0 Stormwater Management

The preliminary storm water development is based on the requirements of the City of Waterloo Engineering Design Manual.

STORM MANAGEMENT DESIGN CRITERIA			
Quantity Control	Post development storm discharge is to be controlled to pre-development levels of for year through 100 years		
Quality Control	MOE Level 1 Enhanced Protection		
Water Balance	Retain first 5mm from each rainfall through on-site infiltration, filtration, evapo-transpiration and/or rainwater reuse		

Table 3 – Storm Design Criteria

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The following storm water management approach is proposed for the development.

- Post Development Flows to be controlled to pre-development levels with the help of onsite detention.
- Quality improvement to be achieved using oil/grit separator.
- Water Balance to be achieved through landscape areas and infiltration chambers.

8.1 **Proposed Quantity Control**

The pre-development land use breakdown and runoff coefficient calculation is as per Table 4 below. Pre-development landuse are shown in figure DR101, Appendix C.

Table 4 – Pre-Development Runoff Coefficient Calculations

AREA TYPE	ARE	ZA (M ²)	RUNOFF COEFFICIENT "C"	AREA x C
ASPHALT/CONC.			0.90	0.00
BUILDING ROOF			0.90	0.00
LANDSCAPED AREA	19	2.570	0.25	48.14
GRAVEL	157	70.130	0.50	785.07
		ΣAREA X C		833.21
		WEIGHTED AVERAGE "C"		0.47
		AREA "A" (Hectares)		0.1763

Post development land use breakdown and runoff coefficient calculation is as per Table 5 below. Post-development landuse are shown in figure DR102, Appendix C.

 Table 5 – Post Development Runoff Coefficient Calculations

AREA TYPE	AREA (M	²) RUNOFF COEFFICIENT "C"	AREA x C
ASPHALT/CONC.	1268.680	0.90	1141.81
LANDSCAPED AREA	301.450	0.25	75.36
BUILDING	192.570	0.90	173.31
		ΣAREA X C	1390.49
		WEIGHTED AVERAGE "C"	0.79
		AREA "A" (Hectares)	0.1763

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Quantity control through onsite detention (parking lot ponding) will limit the post-development runoff release to the allowable pre-development level. To provide quantity control of up to 100-year design storm, the controlled release rate shall be less than or equal to post development flow rate.

Table 6 – Comparison	of Existing and	Proposed Release	Rates
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Return Period (Years)	2	5	10	25	50	100
Pre-Development Allowable Flow (L/sec)	13.97	18.78	23.14	26.57	29.66	32.82
Post-Development Peak Flow (L/sec)	23.32	31.35	38.61	48.78	59.40	68.47
Orifice Controlled Flow (L/sec)	13.36	14.90	16.29	22.95	26.06	26.88

8.2 Required Onsite Detention

Require detention storage caused by flow restriction calculated for 2, 5, 10, 25, 50 and 100 years rainfall events and presented Table 6. Maximum depth of 0.15m will create total storage on paved surface of 29.22 m³ (Refer: Drawing C1, Ponding Storage Table) and additional storage available in pipes, catch basin and manholes will be 3.86m³. (Refer; Stormwater Management Report, Appendix C, Table 3)

Return Period (Years)	2	5	10	25	50	100
Detention Storage Required (m ³)	8.99	14.84	20.13	19.31	21.16	25.18
Storage Used in Pipe (m ³)	1.74	1.74	1.74	1.74	1.74	1.74
Storage Used in MH (m ³)	2.36	2.36	2.36	2.36	2.36	2.36
Storage Used in Ponding (m ³)	4.89	10.74	16.03	15.21	17.06	21.08
Total Available Storage	30.78	30.78	30.78	30.78	30.78	30.78

Table 6 – Detention Storage Summary

9.0 Water Balance

According to City's SWM Guidelines retain storm water on-site, to the extent practicable, to achieve the same level of annual volume of overland runoff allowable from the development site under pre-development conditions. Site volume requirements for water balance are calculated at 5mm rainfall depth for catchment areas. Initial abstraction for the site calculated and presented in Table 4 below:

Catchment	Area (m ²)	IA (mm)	Retention(m ³)
Rooftop	192.57	1	0.19
Asphalt/Concrete Surface	1,268.68	1	1.27
Landscaped Surface	301.45	5	1.51
Total	2.97 m ³		

Table 7 – Detention Storage Summary

According to City's guideline, required quantity for water balance was calculated as follows:

Post Development Water Balance Quantity = Site Area x $5mm = 1762.7 \text{ x} (5/1000) = 8.81 \text{ m}^3$

Stormtech storage tank (DC-780) recommended as shown in Drawing C2. Bed size of the chamber is $12m^2$ (5.55 m x 2.09m) with a capacity of 6.0 m³. Water Balance calculations are summarized below in Table 8:

Table 8: Water Balance Quantity

Required Water Balance Quantity (m ³)	8.81
Water Balance Available:	
1) Initial Abstraction (m ³)	2.97
2) Storm chamber (MC3500) (m^3)	6.0

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 Total Water Balance Quantity Available (m³)
 8

8.97

Water balance deficit will be met through the infiltration chambers proposed on the north-east driveway as shown in Drawing C2, Appendix C.

10.0 Summary & Conclusions

The report presents analysis of existing infrastructure, proposed development concept and servicing demands. The surrounding streets contain water and sanitary services at adequate depths. A stormwater management strategy has been proposed which provides the required quality and quantity controls.

Conceptual servicing and stormwater management plan have been included in support of the proposed development.

Respectfully submitted,



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Appendix A









Appendix B





Appendix C

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		PROVIDED 70SQM. (0.43Acre)	55.57M	67.25 SQM. D Ard	2:30 SQM. PRINCIPAL: 260.15 SQM. 0430 -	51 SQM.(16.31%)	.15 SQM.(14.75%) E: info	IUKEY (5.9M) WWW.	0.14 < 0.45	.25 ON WEST, 55M ON EAST	-1.5M	1.5M	1.5M	PROVIDED	28.60 M.	1.50 M. 10.25 M	1.74 M.		PROVIDED 7	0	13	400X5200MM)	
	I, MISSISSAUGA, UNIARIO NE C5 STANDARDS	REQUIRED-MIN. 1762.7	48.0M			N/A 287.5	260. 6M	1 215.	<u>OOR AREA</u> = 260.15 AREA = 1762.70 = C	4.5M 10.	4.5M	4.5M	3.0M	RFQUIRFD-MIN.	4.5 M.	4.5 M.	4.5 M.		REQUIRED-MIN. a) (Q	15	2 (34	(24
PROJECT STATISTICS	ZONING - D, TABLE BASED ON ZON	MIN. LOT AREA	MIN. LOT FRONTAGE	PROPOSED C-STORE	MAX. GROSS FLOOR AREA	(G.F.A.) Landscaped area	BUILDING COVERAGE	PAVED AND ASPHALT AREA	FLOOR SPACE INDEX : GROSS FL	MAX INT SIDE YARD	MAX REAR YARD	MIN. BUFFER STRIF NEW UNE MIN. BUFFER STRIP REQUIRED	FROM ADJACENT EMPLOYMENT USE	BUILDING SEIBACK	FRONT YARD (NORTH)	REAR YARD (SOUTH)	SIDE YARD (REST)	PARKING CALCULATIONS		TAKE-OUT RESTAURANT	TOTAL	BARRIER FREE PARKING	



PROJECT CONSULTANT	Gas Statior n Architectu	re Inc.					CI	'Y OF A	NISSIS	SAU	ΒA					
PROJECT NO.	n 1690					DESIGN CI	HART S	s1: SAN	JITARY	SEWE	R DESI	SN SH	IEET			
			FLC	W		DESIGN FL	MC	SUM. FLOW			Πd	ЪЕ				
AREA (I and Use Tvpe)	UPSTREAM MH	DOWNSTREAM MH	Area	AVG.	TOTAL	INFILTRATION	TOTAL	DESIGN					(DES.)CA	EXTR.	DES.	EXTR.
			(ha.)	FLOW	AREA	FLOW	FLOW	(L/s)	LENGIH	SIZE	GKADE		.ч (%)	. (%) (%)	VEL.F	VEL. FULL
				(r/s)	(ha.)	(T/S)	(L/s)									
1480 (Commercial)	BUILDING PLUG	SAN MH1A	0.170	13.00	0.170	0.034	13.03	13.03	0.0	300	2.50	152.9	%6	%0	0.98	2.19
	SAN MH1A	SAN MH2A						13.03	18.5	300	0.50	68.4	19%			
	SAN MH2A	SAN MH3A						13.03	14.5	300	1.50	118.4	11%			
	SAN MH 3A	SAN MH4A						13.03	119.0	300	2.50	152.9	6%			
	SAN MH4A	Ex Pipe						13.03	100.0	300	2.50	152.9	9%			

TABLE B2: Fire Flow (FIRE FLOW CALCULATION as per FIRE UNDERWRITERS SURVEY (1999)

PROJECT: 1480 Derry Road East City of Mississauga, ON

1. Fire Flow Equation

$$F = 220 C \sqrt{A}$$

where F is the required fire flow [LPM] C is the coefficient determined by type of construction [unitless]

A is the total protection area [sq.m]

2. Architecture Information

Type of Construction	Fire-resistive	
Fire Rating, Vertical Separation	Inadequate	
Sprinkler Provided (Y/N)	No	
		-
Total Floor Area [sq.m]	179	
Coefficient, C [1]	0.8	
Fire Flow, F [LPM]	2352	
Fire Flow, F [LPM]	2000	Round to nearest 1000

3. Occupancy Reduction

		_
Occupancy Adjustment	0.85	Limited Combustible
Fire Flow, F [LPM]	1700	

4. Sprinkler Reduction

Sprinkler Reduction	0.00
Sprinkler Reduction [LPM]	0

5. Exposure Adjustment

North		0%
East		0%
South		0%
West		15%
	Total	15%
Exposure Adjustment [LPM]		255

6. Required Fire Flow, Duration & Volume

Fire Flow, F [LPM]	1700	
Sprinkler Reduction [LPM]	0	
Exposure Adjustment [LPM]	255	
Required Fire Flow [LPM]	1955	
Required Fire Flow [LPM]	2000	Round to nearest 1000
Required Fire Flow [LPS]	33	

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